**CITY OF BARRIE** 

## ALLANDALE MOBILITY HUB STUDY FINAL REPORT











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CITY OF BARRIE

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## CITY OF BARRIE

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## **1 INTRODUCTION**

## 1.1 PURPOSE

The City of Barrie retained WSP to study the feasibility of shifting the main Barrie Transit Terminal from its current location on Maple Avenue in Downtown to an expanded facility immediately adjacent to the Allandale Waterfront GO Station. This study examined several factors to assess the potential impacts of relocating the bus terminal and established a functional design to demonstrate how the site can meet the program requirements safely, efficiently and in line with City policies. A comprehensive review of relevant transportation and land use planning policies was compiled to understand the policy context of the site and the long-term vision for the area. To inform decisions around the design a best practice report was compiled based on a literature review and study of comparable transit facilities recently constructed around the world. A traffic impact study was also completed to understand the potential impacts of the proposed site reconfigurations on the area's road network. Each piece provided an individual input to the overall functional site plan which can serve as the basis for further consultation and design work.

Throughout the project, WSP worked closely with staff from the City of Barrie to coordinate assumptions and requirements. This report provides an overview of the overall study, including the key findings of each piece of analysis, major recommendations and assumptions developed to conduct each part of the study. The report documents the decision-making process used to evaluate different potential options. Individual components of the study such as the policy review, best practice report and traffic impact study are all included as appendices.

## 1.2 BACKGROUND

Situated on a natural crossroads at the head of Kempenfelt Bay, Allandale has been an important transportation hub for over 160 years. The return of rail service to Allandale Station in 2012 represented the beginning of the rejuvenation of area's historic role in Ontario's transportation network. The start of electrified two-way, all-day service in 2024 will continue this progress by providing a fast, frequent and reliable rapid transit connection to the rest of the Greater Golden Horseshoe. The site is ideally situated to host travel connections between the GTA, Simcoe County, Muskoka and cities along the Georgian Bay coast. With it's proximity to Barrie's rapidly growing south and designation by the Province as an Urban Growth Centre, Allandale is also well suited to serve as a central transportation hub for the city of Barrie. An expanded central bus terminal at Allandale Station builds on previous and planned strategic investments to advance the community's evolution into a modern intermodal and inter-regional mobility hub.

The relocation of the transit facility creates space in the downtown for a new farmers market and event space while consolidating the transit hub function at the GO Rail station to coincide with the arrival of allday rail service. Reducing the footprint of the transit facility in the downtown while maintaining high service levels furthers the City's objectives for downtown revitalization. Investing in transportation and public realm amenities in the Allandale area advances the City's preparation for intensification and development in the area in the future. The new terminal further advances transportation plans to coordinate local and regional transit services with seamless connections.

## **1.3 FUNCTIONAL DESIGN SUMMARY**

The image below shows an overview rendering of the proposed functional design for the site. The bus terminal has been reconfigured into a loop with a central landscaped island, converting the existing western station access road into a transit-only driveway. A new, two-storey station services building is situated in an expanded public plaza located on the Essa Road frontage of the site. Continuous pedestrian canopies and additional shelters are extended from the station building to ensure comfortable waiting areas for transit passengers. Pedestrian crossing of the bus driveway is focused to one location at the top of the loop allowing for improved operational efficiency and safety. A multi-use path has been extended from the existing Lakeshore trail facility directly to the bus terminal and rail platforms. Linear bus stops are included on Essa Road, preserving Barrie Transit's ability to run express service along the corridor without routing into the terminal. Many of the existing features of the current terminal are preserved to save construction costs and minimize the removal of any valuable infrastructure. Ample landscaping is provided to ensure that the site is attractive to users and minimize any potential impacts on surrounding properties.



**Overview of the Functional Design** 

A more detailed explanation of the proposed functional design can be found in section 4.3. Larger versions of the functional design drawings can be found in Appendix C.

## 2 PLANNING CONTEXT

## 2.1 POLICY REVIEW

To inform the mobility hub study, WSP prepared a comprehensive review of policies and plans relevant to the site. This included both provincial and municipal planning policies speaking to transportation infrastructure generally and the Allandale community specifically. The review included relevant guidance from the 2017 review of the Province's Growth Plan for the Greater Golden Horseshoe which includes new policies for Major Transit Station Areas. It also studied relevant sections of the City's Official Plan and Zoning By-law as well as the City's specific design guidelines for transit station areas. A key finding of this review was the planned potential for significant redevelopment and intensification in the Allandale area which will likely see the district transform significantly during the service life of the new transit facility. Another key finding was the limited potential for increased parking supply at the station site, necessitating that increased ridership be accommodated through alternative access means.

## Key Findings of the Policy Review:

- While the Allandale area is currently relatively low density, the City of Barrie, in accordance with provincial planning policies, has established a planning framework allowing for significant intensification of the area in the future
- It is likely the community will grow significantly over the lifespan of any new transit facility
- Parking is highly constrained at the station meaning that ridership growth will have to come from other alternative means of station access
- Increasing GO Station access mode share by Barrie Transit is an efficient way of addressing the station's parking constraints as expanding commuter parking at the station is difficult and disruptive
- The long-term role of the station as the terminus of the GO Rail line means that regional services may use the hub as a connection to destinations beyond the City of Barrie

A comprehensive review of municipal and provincial planning policies relevant to the site can be found in the full Policy Review contained in Appendix A.

## 2.2 CURRENT AND FUTURE TRANSIT SERVICES

## CURRENT BARRIE TRANSIT SERVICES

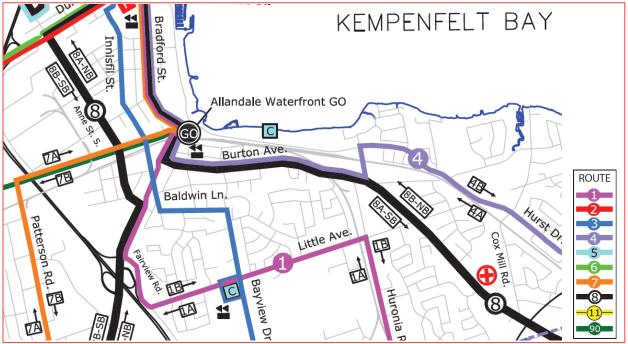
Barrie Transit currently serves Allandale with five routes connecting from Essa Rd, Tiffin St and Bradford Street. The terminal does not have a station services building and is not staffed. There are four heated, fully enclosed shelters at the site to provide full weather protection for transit patrons. Bus services to the site may be used to access GO Rail during the morning and evening peak periods. Few transfers are timed to take place at the terminal and buses typically do not layover for long periods of times.

The current Barrie Transit routes serving Allandale terminal include:

- Route 1A (Georgian Mall-northbound) and Route 1B (Welham-southbound) operate along Bradford Street/Essa Road with a 30-min headway during Monday to Saturday daytime, and a 45-min headway during Monday to Saturday evening and on Sunday.
- Route 4A (East Bayfield-northbound) and Route 4B (South GO-southbound) operate along Bradford Street/Essa Road with a 35-min headway during Monday to Saturday daytime, and a 65-min headway

during Monday to Saturday evening and on Sunday. During Monday to Friday morning hour (5:45 to 6:45), service headways is reduced to 30-min for both routes.

- Route 7A (Bell Farm-northbound) and Route 7B (Bear Creek-southbound) operate along Bradford Street/Tiffin Street with a 30-min headway during Monday to Saturday daytime, and a 60-min headway during Monday to Saturday evening and on Sunday.
- Route 8A (Yonge-southbound) and Route 8B (Crosstown-northbound) operate along Bradford Street/Essa Road with a 30-min headway during Monday to Saturday daytime, and a 60-min headway during Monday to Saturday evening and on Sunday.
- Route 90A (Angus Borden-clockwise) and Route 90B (Peacekeepers Way-counter-clockwise) operate along Tiffin Street with five departures daily from Monday to Friday.



**Current Barrie Transit Map Surrounding Allandale Station** 

Intercity coach services operated by Greyhound, Ontario Northland and Hammond Transportation currently serve the existing downtown terminal. Greyhound operates four trips daily between Toronto and Barrie. Ontario Northland offers seven daily trips in each direction with Barrie as a stopover between Toronto, North Bay and Sudbury. Hammond Transportation offers one trip daily between Barrie and Midland and 3-4 trips between Barrie and Orillia.

#### CURRENT GO TRANSIT SERVICES

Allandale Waterfront GO rail station is currently served on weekdays by seven southbound departures in the morning and seven northbound arrivals in the evening and on weekends by three morning departures and evening arrivals on weekends. Trains on the Barrie corridor serve several stations in York Region and connect directly to Line 1 of the TTC Subway at Downsview Park Station. Additional connections to new rapid transit lines are planned. GO buses currently service the terminal with the route 68 which operates approximately hourly throughout the day. The route terminates at the current Barrie Transit terminal just north of the Allandale site. Due to the variable nature of GO bus travel times, buses typically layover for longer periods in Barrie. Route 68 is a local-stop service, using Yonge Street to provide an

intercity connection between Barrie, Innisfil, Bradford, Newmarket and Aurora. Two variants of the route serve either East Gwillimbury GO Station or Aurora GO Station where mid-day train connections are available. GO bus service is replaced by rail service during the peak period in the peak direction.

#### **EXISTING TRANSIT FACILITIES**

The existing Barrie Transit terminal is located on Maple Street in the city's downtown. Opened in 1991, the facility includes 12 bus platforms that serve both Barrie Transit and intercity coach operators. GO Transit, Ontario Northland, Greyhound and Hammond Transportation operate regularly scheduled services from the terminal to Toronto, Sudbury, North Bay and Midland making the station a regional hub for intercity travel. Intercity coach services are segregated to platforms 9-12 allowing for longer layovers without compromising Barrie Transit operations. The station also includes passenger amenities such as ticket counters, washrooms, heated waiting areas, retail and concessions as a well a Barrie Police kiosk. The area around the facility includes public plaza space, landscaping and bike parking.

The current facilities at Allandale Waterfront station were opened in 2012 when GO Rail service was extended north from the previous terminus at Barrie South Station. The bus facility includes eight signed bus stops with six saw-tooth bays along the north side of the rail corridor served by a one-way transit-only access road accessed from Essa Road and egressing onto the main station access road. A transit-only lane is also provided on the station access road to accommodate north bound left turn movements by transit vehicles. Four of the saw-tooth bays are reserved for Barrie Transit with two reserved for GO buses. Barrie Transit also uses two on-street stops on the east and west sides of Essa Road.

#### PLANNED TRANSIT EXPANSION

As part of the GO Regional Express Rail (RER) program two-way, all-day rail service is currently indelivery between Union Station and Allandale Waterfront GO and is expected to be operational by 2024. The full length of the Barrie Corridor will be fully electrified and double tracked. Several new stations are planned with two new intermodal connections to the Eglinton LRT and the Line 2 of the TTC subway. An environmental assessment (EA) for double tracking the entire length of the Barrie Corridor was completed in October 2017. The EA design calls for the addition of a second platform at Allandale Waterfront Station but does not indicate any property impacts.

The current RER service plan calls for hourly service between Union Station and Allandale Waterfront, with 15-minute service between Union Station and Aurora GO. In the peak period this service would be increased to every 30-minutes or better between Allandale Waterfront and Union Station. Service levels are planned to increase incrementally as double tracking and a new rail-rail grade separation is completed at the Davenport Junction in Toronto. The details of the RER service plan are still subject to change as the implementation of the project continues. Recent reports from Metrolinx have suggested that all services from Allandale Waterfront may run express between Aurora and Union Station with stops only at Downsview Park and Spadina Stations. The 2041 Regional Transportation Plan calls for 15-minute electrified RER service to eventually be extended to East Gwillimbury GO. There are no mentions in the 2041 plan about extending GO service north of Allandale Waterfront.

Simcoe County has completed detailed planning for a new inter-municipal regional transit service. The first phase of this service, between Barrie and Midland is scheduled to begin full operation in September 2018. The full network is planned to include five new routes, four of which will connect directly to Barrie

Transit. Current plans for the project envision most of the new routes connecting to Barrie Transit at Georgian College and the Royal Victoria Hospital rather than Allandale or Downtown. This service will likely replace the existing transit provisions between municipalities operated by Barrie Transit such as route 90 to Angus.

A full review of applicable transportation plans, committed expansions and surrounding services can be found in section four of the Policy Report contained in Appendix A.

## 2.3 BEST PRACTICE REVIEW

To inform the design of the new terminal, WSP undertook a detailed best practices report. The aim of this work was to ensure that a new facility in Barrie will incorporate state of the art of planning and design practices. This work also focused on ensuring that the design is future ready for the potentially disruptive changes currently facing the transportation sector. The review included an overview of basic terminal design typologies and their relative benefits for various design objectives. It also included a review of many new station designs both in Ontario and internationally to draw trends and observations from comparative projects.

To help understand flexibility for the future, the review also studied several emerging technologies which may impact the operation of the facility within its current lifecycle including dynamic bus bay assignment and charging for battery-electric vehicles. Various best practices and design guidelines were highlighted for specific station elements such as active transportation and urban integration. Finally, the review analysed several local peer networks with similar configurations of transit hubs at rail stations slightly outside of their downtown core.

A comprehensive review of terminal design typologies, peer networks in Ontario, emerging technologies and a global scan of design practices for bus terminals at commuter rail stations can be found in the Best Practices Report contained in Appendix B.

## **3 DESIGN CONSIDERATIONS**

## 3.1 PROGRAM REQUIREMENTS

The development of functional design alternatives for the proposed bus terminal at the Allandale GO Station was driven by the range of issues identified by Barrie Transit and the estimated future functional requirements identified as part of the transit servicing and scheduling review. As part of this study WSP worked with Barrie Transit to develop an assumed future transit network reflecting the modifications to serve the new terminal. This analysis informed the recommendations for the sizing and future capacity of the terminal.

The following facility program requirements were established:

- Passenger safety features are paramount in the design of the facility and features to minimize pedestrian circulation conflicts with vehicle traffic will be incorporated;
- Emphasis will be placed on connecting the terminal to the planned active transportation network as the provision of a safe, comfortable and convenient pedestrian environment is a key factor in increasing the use of public transportation;
- Full accessibility, in accordance with the Accessibility for Ontarians with Disabilities Act (AODA) will be
  provided as necessary, including accessibility features for physically, visually and hearing-impaired customers;
- A passenger services building and plaza is to be incorporated in the terminal design concepts to accommodate ticket sales, a concession area, washrooms, service information, bicycle storage and lockers, etc. In addition, the terminal design concepts will accommodate features and amenities to enhance the passenger environment including canopies along platforms, a heated/air-conditioned waiting area, benches, litter containers, newspaper boxes, public telephones, electronic information displays and a public-address system;
- A total of at least 12 bus platforms, sized for standard 12m city buses (B-12), are desired for the new transit terminal. In addition, a small but undefined bus layover area is anticipated to be required and will be incorporated into the design where feasible;
- Support facilities for operations personnel (washrooms, driver and staff lunchroom, supervisor's office, janitorial closet, electrical/mechanical room, fire detection and alarm, emergency lighting supply, parking) will be required for each of the terminal design concepts; and
- Access for service vehicles to perform routine service and maintenance without interrupting normal terminal operations.

## 3.2 FUTURE TRANSIT NETWORK ANALYSIS

To understand the design requirements of the new terminal and the transit network implications of moving the facility to Allandale from Downtown Barrie a study of transit network impacts was undertaken. Two scenarios were developed for the future bus network based on different approaches to network design. The development of the scenarios included analysis of current ridership and boarding patterns as well as a strategic focus on providing access to the GO Rail station in anticipation of increased demand coming from all-day service. Data from the GO Rail Passenger Survey and Transportation Tomorrow Survey were used to identify clusters of trip origins that may be served by GO Transit and may access the system from the northern and central parts of Barrie. Small modifications were proposed in both scenarios to provide enhanced service between these demand clusters and Allandale Waterfront GO Station. As

Downtown Barrie is a significant destination for travellers and will remain so in the future, the network configurations placed an emphasis on continuing to maintain high service to the downtown.

#### DESCRIPTION OF SCENARIOS

Scenario 1 involves diverting all the existing Barrie Transit routes to the Allandale Mobility Hub. This scenario creates the benefit of having a central hub location where travellers can easily transfer to any other route in the system. However, it brings the disadvantage of increased operating costs as some routes become longer and may require additional vehicles to maintain current headways. Because this scenario focuses on serving Allandale it provides the most direct coverage to the GO Rail station but reduces direct coverage to Downtown.

Scenario 2 involves strategically diverting some routes to service the hub while maintaining other routes with a focus on Downtown. This scenario includes strategic improvements to provide more direct service to the GO Station for areas where there are likely clusters of trip origins. Importantly, this scenario was designed to be cost neutral, meaning that operating costs are relatively close to the existing network and no additional buses are anticipated to be required. This conceptual transit network would have the benefit of increasing service to the GO station and providing more transfer options at Allandale station than currently exist today. It would also preserve good direct coverage to Downtown Barrie meaning that passengers who can currently access the downtown without transferring would likely still be able to do so. This is particularly important for routes in the north part of Barrie for which a diversion to Allandale station may be a longer trip. The routes which do not directly service the Allandale terminal have alternative places to layover for their recovery time and driver breaks so that will not need an off-street facility in the downtown.

#### SCENARIO ANALYSIS

Both scenarios were analysed using GIS software to understand their operating requirements and coverage impacts. Barrie Transit Staff provided detailed consultation on existing network characteristics and feasibility. The total operating distance of each scenario was compared against average operating costs provided by Barrie Transit. The potential changes to operating speed were also estimated and compared to the current average Barrie Transit vehicle operating cost per hour. From this speed analysis the number of additional vehicles was determined to maintain the current system-wide service level of 30-minute headways. Finally, the spatial software was used to understand the coverage provided by each network of a single-seat ride to four different key destinations around Barrie. These destinations included Downtown, Allandale Station, Georgian College and Park Place. The analysis captures the percentage of total residents who live within a 400m walk to a bus route that will take them directly to the specified location.

#### SCENARIO ANALYSIS RESULTS

The analysis showed that while scenario 1 provided significantly higher coverage to Allandale station, it did so at the expense of providing coverage to the downtown. Scenario 1 also incurred an increased annual operating cost and the purchase of two new buses to accommodate the longer total route lengths with longer roundtrip travel times. In scenario 1 direct coverage to the downtown dropped from 79% today to around 72%. Some of this coverage would also be impacted by longer and more circuitous trips to reach downtown, particularly from the Letitia Heights community as existing routes would only service the downtown in one direction. Because Downtown is Barrie's most significant travel destination, reduced coverage to the downtown has a disproportionately large impact on the attractiveness of the network. The first scenario also reduces coverage from the southwest of the city to the Dunlop Street corridor.

Scenario two was able to preserve only a small decrease in direct coverage to the Downtown while substantially increasing coverage to Allandale. As described, the new coverage areas were strategically developed to target areas likely to connect to GO Transit rail services at Allandale. The Allandale Waterfront GO station does not itself require complete transit coverage from the entire city as many residents may find it more convenient to access these services at Barrie South GO station where Barrie Transit connections are also available. Because this scenario still involves increasing the length of some routes to divert into the new terminal a small incremental operating cost increase is required. This is somewhat offset by improvements to operating speed. The speed improvements also allow for this conceptual network to be operated with the existing vehicle fleet.

SCENARIO	DESCRIPTION	COST INCREMENT	% OF RESIDENTS WITH DIRECT SERVICE COVERAGE
Transit Network Scenario 1	All routes diverted to serve Allandale Hub Additional Operating Cost	<ul><li>12.1% Annual Operating Cost Increase</li><li>2 Additional Buses</li></ul>	<b>To Downtown: 72%</b> To Allandale: 78% To Georgian College: 63% To Park Place: 62%
Transit Network Scenario 2	Select routes diverted to Allandale, service levels maintained to Downtown Barrie Cost Neutral	3.2% Annual Operating Cost Increase No Additional Buses	<b>To Downtown: 78%</b> To Allandale: 68% To Georgian College: 62% To Park Place: 64%
Existing Network	Current Barrie Transit Network	n/a	<b>To Downtown: 79%</b> To Allandale: 63% To Georgian College: 63% To Park Place: 63%

#### TRANSIT NETWORK SCENARIO FINDINGS

**Summary of Transit Network Scenario Analysis** 

Based on the outcomes of the coverage and operational analysis, scenario two was found to offer several clear benefits. Downtown, as the commercial and cultural heart of the city, is likely to remain the busiest destination for transit riders from across Barrie. By maintaining direct service coverage to the downtown, Scenario Two provides more beneficial service to riders. The robust structure of the Barrie Transit network provides additional transfer points between routes and locations for vehicles to layover with a hub at Allandale and an on-street transit stop in the downtown. Scenario two also avoids a significant increase in annual operating costs and the requirement to purchase additional vehicles. While not all routes serve the Allandale Hub, riders wishing to access GO Rail services from the southern part of the city may also use Barrie South GO Station.

## 3.3 TRAFFIC COUNT ANALYSIS

To provide a basic understanding of the surrounding traffic conditions and a basis for the traffic impact study, turning movement counts were collected for all of the study area intersections. The counts were conducted in February 2018, for an eight-hour period for the typical weekday and four-hour for weekend conditions. The counts were reviewed and compiled to reflect the weekday morning and afternoon peak hour conditions, and weekend peak hour condition. Traffic volumes for these three peak hours (morning, afternoon, and weekend) were further balanced to ensure consistency between two adjacent intersections. Truck percentages and peak hour factors were also calculated and applied in intersection capacity analysis.

The local road network consists of five municipal roads in additional to the station access roadway:

- Lakeshore Drive is a City Parkway traveling in the north-south direction north of Tiffin Street and in the east-west direction east of GO Station West Access. This corridor includes two-lane cross-section within the study area, and operates with signalized intersections at Tiffin Street/GO Station West Access and at GO Station East Access. Lakeshore Drive widens and accommodates an additional through lane on the east approach to Tiffin Street/GO Station West Access. The speed limit is 50 km/h on Lakeshore Drive within the study area.
- **Bradford Street** is an Arterial corridor in the north-south direction parallel to Lakeshore Drive, with a four-lane cross-section within the study area, and operates with a signalized intersection at Tiffin Street. The speed limit is 50 km/h on Bradford Street within the study area.
- *Tiffin Street* is also an Arterial corridor, providing access in the east-west direction and connect to Lakeshore Drive at the Allandale GO Station West Access. The segment in the study area includes two-lane cross-section and operates with signalized intersections at Bradford Street and at Lakeshore Drive/GO Station West Access. The channelized westbound right turn lane from Tiffin Street to Bradford Street forms a triangle landscaping island. The speed limit is 50 km/h on Tiffin Street within the study area.
- *Essa Road* is an Arterial corridor, traveling in the north-south direction and connects to Bradford Street at Tiffin Street intersection. It includes four-lane cross-section within the study area, and operates with traffic signals at Tiffin Street and at Gowan Street intersections. The signalized intersection at Gowan Street also accommodates an atgrade railway crossing. A 'bus-only' access to the GO Station is located 20 m north of the Essa Road and Gowan Street intersection. Within the study area, the speed limit is 50 km/h on Essa Road.
- *Gowan Street* is a Minor Collector traveling in the east-west direction and providing access to the GO Station (on the north side of the road) and local residential areas (on the south side of the road). It has a two-lane cross-section and operates with a signalized intersection at Essa Road. The speed limit is 50 km/h on Gowan Street.

The results of these counts were assessed using the 'Level of Service' (LOS) measure for signalized intersections. LOS provides an indication of the experience of using the intersection based on a mixture of measures and expressed as the average delay in seconds per vehicle. The measure takes into account factors such as signal timings and coordination, traffic volumes, fuel consumption and queueing.

The intersection capacity analysis results indicated that all the study area intersections and individual movements are operating at LOS 'C' or better for three peak hour conditions, except for the southbound left movement at the Lakeshore Drive and Tiffin Street intersection. This movement is operating with an unacceptable level of delay (at LOS 'F') during all the three peak hours with volume to capacity (v/c) ratio over 1.0. The higher delay for this movement is mainly caused by heavy traffic demands (400, 620, and 460 vehicles per hour during morning, afternoon, and weekend peak hour, respectively). This results in overall intersection LOS at 'E/F' for Lakeshore Drive and Tiffin Street intersection during afternoon and weekend peak hours.

LEVEL OF SERVICE	CONTROL DELAY (s/vehicle)	DESCRIPTION	
Α	< 10	Free flow of traffic	
В	10 - 20	Slight delay, good traffic flow	
С	20 - 35	Acceptable delay, acceptable traffic flow	
D	35 – 55	Potential unstable traffic flow,	
Е	55 - 80	Unstable traffic flow, queues may fail to clear	
F	>80	Congested operations, queues fail to clear	

Source: Highway Capacity Manual, Transportation Research Board, 2010

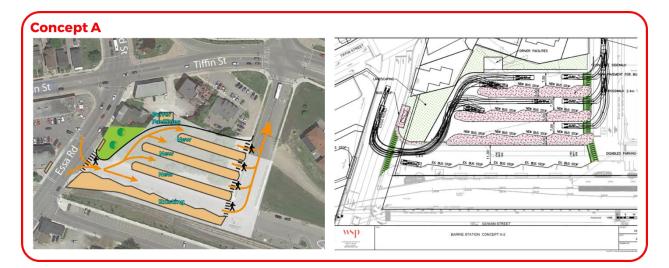
## **4 DESIGN RECOMMENDATIONS**

## 4.1 HIGH-LEVEL DESIGN OPTIONS

Three conceptual alternative bus terminal design configurations were considered in the early phase of the study to present high-level potential design directions to the project team. These alternative bus terminal configurations were developed based on the results of the Best Practice and Policy Reviews. The high-level alternative bus terminal design concepts shared through this process are presented below. Following consultation with Barrie Staff, two of the alternative configurations were evaluated through a preliminary screening. Based on this preliminary screening, one design concept was carried forward for further development with the full station program, local geometric standards and property requirements to produce the final proposed functional design. Concept A and concept C were advance the preliminary screening while concept C was advanced to the functional design.

## CONCEPT A:

Concept A provides a total of 11 additional bus bays distributed among three separate passenger platforms, and maintains the existing six bus bays adjacent to the rail corridor. This concept was predicated on the notion of maintaining the existing bus access and circulation operation, but expanding on the capacity of the bus terminal. This configuration allows buses serving the new stops to recirculate back through the terminal to access layover facilities as required. While it is a relatively space-efficient layout, the concept introduces several new potential vehicle/pedestrian conflict points; pedestrians would have to cross the paths of buses (potentially multiple times) to transfer between the proposed new bus platforms. Further, by distributing the new bus bays among multiple passenger platforms, duplicate passenger amenities (benches, shelters, lighting, etc.) would have to be installed.



## CONCEPT B:

Concept B provides a total of 11 additional bus bays, with eight bays consolidated around a central passenger island-platform, and three additional bays adjacent to the northern curb of the proposed terminal. This concept again maintains five of the existing six bus bays currently in operation along the south side of the site. This concept, by consolidating more passenger platforms around a single island, addresses some of the pedestrian crossing challenges present under Concept A, but at the expense of a

particularly-wide (15m) pedestrian crossing of the bus terminal circulation laneway. Further, the vehicular operations are somewhat more complicated and space-consumptive given the two-way bus circulation required around the proposed passenger island. The access configuration to the terminal would remain as exists today.



#### CONCEPT C:

This concept was developed to focus on addressing the drawback of Concepts A and B, by minimizing the need for pedestrians to cross vehicular circulation pathways, and simplifying the vehicular circulation requirements. The concept results in a total of nine bus bays in an outer-loop configuration terminal, with two more bays along the station access road and two additional passenger platforms on Essa Road for a total of 13 bays. This would allow passengers to transfer between bus services without crossing vehicular roadways, and access the rail platform by crossing one relatively narrow bus-only driveway. A central landscaped island would be provided to discourage pedestrian crossing of the bus terminal. In this concept, the Allandale GO Station access at Lakeshore Drive/Tiffin Street would be converted to bus-only operation, which would act as the sole point of access for the terminal; the existing Essa Road access would be closed to simplify vehicle circulation and pedestrian access. New transit-priority measures may be included on Tiffin Street to assist with bus circulation from the new singular access/egress point. Platforms would likely be allocated so that inter-city services were concentrated at the north of the site, allowing for shorter transfers between Barrie Transit services.



#### PRELIMINARY SCREENING

The range of design concepts were presented to the City of Barrie for review and discussion. Concept B was subsequently eliminated from consideration based on concerns regarding the proposed access operation on Essa Road, and the presence of obviously better performing alternatives; Concepts A and C. Stakeholder review raised potential challenges of relying on a new rail corridor crossing to access the terminal as this could create complications with potential rail expansion plans. As such, Concepts A and C were carried forward for a more detailed comparative assessment.

## ASSESSMENT CRITERIA

The following evaluation criteria were used to determine a preferred conceptual design:

## Transportation / Transit

**Ease of Bus Access** – This criterion considers two elements of bus access: travel distances, and turning movements. Further consideration is given to the potential for conflicts with other vehicles. Travel distances were measured from various points along the perimeter of the study area to compare the vehicular attractiveness of the transit terminal location to GO Transit, Barrie Transit, pedestrians, and cyclists. The comparison of turning movements includes the number of turning movements required (particularly left-turns) and their anticipated delays. For comparison purposes, the following anticipated bus approaches were assumed:

- For buses accessing from/to the north, their path was measured from Bradford Street at the southern access to the Wastewater Treatment Facility;
- For buses accessing from/to the south, their path was measured from Essa Road at Gowan Street;
- For buses accessing from/to the east (via Burton Avenue), their path was measured from Essa Road at Gowan Street; and
- For buses accessing from/to the west, their path was measured from Tiffin Street at Short Street.

**Ease and Safety of Pedestrian Linkages** – Each facility was designed primarily with transit operations in mind. However, with the station being integrated into a commercial district where pedestrian activity could be heavy, consideration must be taken to provide a facility that will allow for safe pedestrian access. Terminal configurations that would result in the need for pedestrians to cross wide accesses or encourage unsafe crossing were rated lower than those that provided easy, safe access to the platforms.

**Impact on Area Traffic Operations** – This criterion was measured according to the number and location of accesses to the terminal facility. Alternative designs that require multiple accesses/egresses could result in a greater disruption to traffic by increasing the possible points of conflict. In addition, locations that would access/egress from high volume streets will pose a greater risk of conflict than those using minor streets.

## Land Use

**Relationship to Municipal and Provincial Policies** – Describes the proposal's general adherence to the vision for the area as articulated by official planning documents.

**Impact on Existing Properties** – This criterion considers qualitatively the impact of the proposed design on adjacent properties, including their access operations, visual impacts, and potential for noise impacts.

## Engineering Considerations

Efficiency of Site Use – This criterion forms a quantitative measure of how efficient the design concept makes use of the available space, by calculating the area required per bus bay, for each of the design concepts.

**Potential for Future Expansion** – Building on the evaluation of alternative sites for the Construction Cost criteria, this criterion will compare physical constraints such as major utilities and existing infrastructure beyond the construction limits of the current program requirements. Concepts that provide greater potential to accommodate future expansion of the transit facilities will be rated higher than those that are constrained.

**Construction Cost** – This criterion will be based on a qualitative comparison of construction costs for the alternative sites. The evaluation will draw on the conceptual facility layouts to be developed and available base mapping and utility information. Options that can be implemented at reduced cost and with minimal impact to existing infrastructure and major utilities will be rated higher than those that require additional new infrastructure, or significant modifications to existing infrastructure and utilities.

CRITERIA	<b>CONCEPT A</b>	CONCEPT C	
Transportation/ Transit Considerations			
Ease of Bus Access	The option of accessing the site via either Tiffin Street or Essa Road provides for flexibility and redundancy for bus operations. The Essa Road access, as existing, can be problematic in times of heavy traffic volumes. Further, egress operations could potentially be impacted by traffic congestion and conflicts associated with operating in mixed traffic with other station users.	This option requires all buses to access the terminal via the controlled Tiffin/Lakeshore intersection, resulting in marginally-longer travel distances for some bus services. The conversion of the access roadway to bus-only use is, however, expected to improve access and egress operations by eliminating potential for conflicts and congestion associated with general traffic.	
Access from the North (Bradford St)	150, 1 left-turn 43s intersection delay	240m, 1 left-turn, 1 right-turn 44s intersection delay	
Access from the South (Essa Rd)	50m, 1 right-turn 3s intersection delay	310m, 2 right-turns 37s intersection delay	

## 4.2 EVALUATION OF DESIGN CONCEPTS

Access from the West	120m 1 right turn	250m 1 right turn	
Access from the West120m, 1 right-turn(Tiffin St)59s intersection delay		250m, 1 right-turn 60s intersection delay	
Ease and Safety of Pedestrian Linkages	This option requires bus passengers to cross at least one, or up to three, vehicle pathways to transfer between buses and/or the rail platforms. While designated pedestrian crossing points are provided and signed accordingly, this introduces several potential pedestrian/vehicle conflict points.	This option, by situating all bus platforms along the outer perimeter of the terminal, allows for passengers to transfer between bus platforms and the rail station without having to cross a vehicular path. Recognizing that this may not be convenient for all movements, the provision of a pedestrian crossing of the main station access roadway provides a single point at which pedestrians would have to cross a vehicular path. The closure of the access to general traffic also allows for the introduction of an active transportation pathway directly to the station platforms, further improving pedestrian and cycling access to the station.	
Impact on Area Traffic Operations	This option requires all the southbound buses to occupy the median lane on Essa Road while entering the station. This condition will affect the southbound traffic flow on Essa Road, which could result in vehicles changing lane to overtake these buses. With high southbound traffic volumes on Essa Road, this option can potentially create long queues up to the upstream intersection at Tiffin Street/Bradford Street.		
	Land Use Considerations		
Relationship to Municipal and Provincial Policies	The Allandale Station area falls within the 'city centre' designation in the City's Official plan and is identified as both an Urban Growth Centre and a Major Transit Station Area by the Province. Investments to create an integrated transit hub at the site are supportive of land use policies  Prevents a continuous street frontage on Essa Rd Corridor by requiring the main access station access road.  Allows for a continuous building and park frontage along Essa Rd by closing the existing station access road. Generally improves the pedestrian experience in the area.		

Impact on Existing Properties	None of the options explored were found to have a significant impact on surrounding properties. The identified parcel of vacant city-owned land is sufficient to accommodate all the program elements			
	Engineering Consideration	ons		
Efficiency of Site Use	550m2/bus bay	760m2/bus bay		
Potential for Future Expansion	This option allows for incremental expansion of bus bays using new islands, eventually facilitating expansion to 17 bays, five more than at the current Barrie Transit Terminal. Site operations are constrained by the unsignalized access point on Essa Rd.	This option provides only one additional bus bay more than the current facility but improves the likely operational capacity of the site. Bus bays may be expanded to the east of the bus loop in the future with minor site work and relocation of the employee parking.		
Construction Cost	The largest cost item of the proposal is the station building which is not affected by either option. This option does not present any significant cost increments.	This option creates a slightly larger footprint but is not anticipated to represent a significant cost premium over Option A.		
Conclusions				
Recommendation	Do Not Carry Forward	Carry Forward for Further Study		
Low Performing		High Performing		

Considering all the above, Concept C was determined to be the simplest, safest, and most efficient of the alternatives considered. Concept C does result in minor increases in bus travel distances over Concept A. However, Concept A has other notable drawbacks in pedestrian/cyclist access, and complexity without commensurate improvements in bus operations. Ultimately, Concept A creates several compromises for the sake of a minor improvement in bus travel distances, and is therefore not recommended. Concept C was carried forward to a full functional concept design.

## 4.3 RECOMMENDED DESIGN

The recommended functional design for the terminal is a loop configuration with a transit-only access from the intersection of Lakeshore Drive and Tiffin Street. The design includes 11 bus bays within the loop and an additional two bays on Essa Road for a total of 13 bus bays. Passenger movements are focused to avoid interaction with transit vehicles except at one crossing on the north side of the loop which will be well marked. A station services building is situated in a plaza along the site's Essa Road frontage. Private vehicle access to the site is moved exclusively to the existing eastern station access road from Lakeshore Drive which is located closer to the rail tunnel and with good proximity to the rail passenger pick-up and drop-off area.



#### **BUS SERVICE CONFIGURATION**

Nine bus bays are configured around the central island, four of these are existing bus bays along the south which are being preserved from the existing site. By concentrating Barrie Transit services in the bays along the south and central portions of the site, most transfers to GO rail and between buses may be accommodated without pedestrians having to cross the bus driveway. The two linear bus stops on Essa Road allow Barrie Transit the flexibility to operate express services that may not enter the rest of the terminal facility. There is potential to add additional on-street bus bays on the west side of Essa Road if the adjacent plaza site redevelops at some point in the future. Two new bus bays are proposed for the western side of the access road. These bays may be allocated to intercity coach services where passengers are less likely to transfer between vehicles. The new bays are accommodated within the existing road right-of-way and should not require additional property acquisition.

#### SITE ACCESS

The current mixed-traffic station access road is re-purposed as a transit-only access to the site. This will preserve the reliability and speed of transit vehicles accessing the terminal, in line with the City of Barrie's objectives to increase transit mode share and the Metrolinx objectives of encouraging alternate modes of station access to driving and parking at the station. The intersection maintains a transit priority phase which preserves any future need for enhanced transit-priority at the site. The ability for buses to move in any direction at the intersection is also preserved, allowing for future operational flexibility. New bus-only turn lanes are provided to the facility from Tiffin Street and Lakeshore Drive. The eastbound righthand turn lane from Tiffin Street to allows transit vehicles to bypass a potential que from the eastbound through movement. A new transit-only left lane westbound on Tiffin Street provides buses with reliable access to the left turn movement onto Essa Road. Bollards and a curb cut are proposed at the southeast corner of the loop to preserve an emergency secondary site access.

#### PASSENGER AMENITIES

A glass-enclosed vestibule area is shown adjacent to the station building which can allow for a heated waiting area to remain open when the rest of the station building may be closed. Canopies are extended from the station building across the main bus platforms providing weather protected access to the station building. New heated shelters are shown at the north side of the bus loop along with two additional new covered bike shelters in addition to the current heated shelters and sheltered bike parking which will be preserved. The new bike shelters are in an area of high visibility to discourage theft and will fulfill the Metrolinx objective of increasing overall bike parking at the station.

#### LANDSCAPING & PUBLIC REALM

At the western side of the bus loop emphasis is placed on maintaining a continuous sidewalk along Essa Road and improved public realm experience. New plantings and gardens are possible in this plaza space. The two-storey station building is positioned to frame Essa Road while preserving sightlines from the street into the bus terminal for safety. The main bus loop is configured around a landscaped island. This island will prevent pedestrian crossing of the bus loop with a special raised central curb and offers opportunities for enhanced visual amenities in detailed design. A landscaped area north of the bus loop is maintained to provide a buffer to the existing businesses and an opportunity to potentially preserve some of the site's existing mature vegetation.

#### ACTIVE TRANSPORTATION

A multi-use path is extended from the existing facility along the waterfront into the heart of the station. This will ensure that the waterfront trail asset is leveraged to encourage access to the station by active transportation. An enhanced pedestrian and cyclist crossing is shown at the east side of the intersection to increase pedestrian safety as the intersection experiences a very high volume of southbound left turns from Lakeshore Drive. An additional crosswalk is shown across Essa Road immediately north of the rail corridor crossing.

## 4.4 PROPOSED ROAD NETWORK MODIFICATIONS

The functioning of the surrounding road network is fundamentally linked to the operations of the terminal itself. Congestion accessing the terminal creates a negative experience for transit riders and can significantly hurt on-time performance around a central hub. Because the preferred functional design involves altering the existing transit access point to the site, attention was given to potential modifications to the surrounding the road network which can ensure the speed and reliability of buses accessing the site. These proposed road network modifications were limited to minor alterations based on the current traffic counts and condition and age of the infrastructure. The proposed alterations were included in the scenarios assessed in the traffic impact study. Further study and design is required to refine the proposed road network changes.

## SUMMARY OF PROPOSED TERMINAL ACCESS MODIFICATIONS:

- 1 Closure of the current station access on Essa Rd.
- 2 Severing the station access road so that public access to the Passenger Pick-up and Drop-off is exclusively through the signalized intersection east of the historic station building
- 3 Conversion of the western station access road to a bus-only facility
- 4 Introduction of transit priority measures on Tiffin Street



**Overview of Terminal Access Modifications** 

#### SUMMARY OF PROPOSED TRANSIT ACCESS MODIFICATIONS:

- 1 Introduction of a bus-only lane on Tiffin Street that merges into a general left-hand turn lane before the intersection of Tiffin Street and Essa Road/Bradford Street with potential for this turn lane to be extended to the intersection in the future
- 2 Introduction of a bus-only right turn lane at Tiffin Street and Lakeshore Drive to allow buses to bypass any queued traffic for the eastbound through (two bus lengths or 20m)
- 3 Reduction of Tiffin Street and Lakeshore Drive eastbound through to one lane from existing two and conversion of far-left lane to a transit-only left turn lane
- 4 Continuation of transit priority measures at Tiffin Street and Lakeshore and new Transit Priority measures incorporating the dedicated left turn phase at Tiffin Street and Essa Road/Bradford Street



**Overview of Surrounding Road Network Modifications** 

#### LAKESHORE DRIVE AND TIFFIN STREET INTERSECTION

This intersection represents the most challenging point in the study area. The conversion of the station access road to transit only traffic may help to improve the intersection by providing additional space for left hand turns however a transit-only phase may still be needed to allow buses to make the westbound left onto Bradford Street. Tiffin Street has been modified to have one westbound receiving lane with the slip right turn to Bradford shortened to extend from the single lane. The Traffic Impact Study included a scenario including provision for two northbound left lanes to help buses bound for the westbound right onto the Bradford Street make the lane transition. Alternatively, a right on red prohibition may be implemented at the Lakeshore southbound right to reduce conflicts with buses changing lanes on Tiffin Street to make the westbound right onto Bradford Street.



Lakeshore Dr – Tiffin St – Station Access Road Modifications

## TRANSIT PRIORITY MEASURES

A dedicated transit lane is provided starting at Lakeshore east of the station access road and extending along Tiffin Street to the left-hand turn lane for Essa Road. This lane will allow for buses to avoid any congestion delays with through traffic, conflicts with vehicles turning right from Lakeshore or vehicles slowing to turn right onto Bradford. Special signage and design consideration will be needed to ensure the legibility of this transit-only lane and reduce the chance of private vehicles entering it.

A transit-only eastbound right from Tiffin has also been included to ensure reliable transit access to the station access road and avoid any potential queuing of the eastbound through movements. A right turn channelized lane for buses is not recommended as it may conflict with pedestrians and cyclists. Special consideration is anticipated to mitigate the conflicts with pedestrian traffic at this intersection as it is intended to provide a primary access point between the station and the multi-use trail along the waterfront.

## INFRASTRUCTURE MODIFICATIONS

The proposed road network changes require some minor modifications to the existing hard infrastructure. As the intersection is in good condition, the changes were designed to preserve as much of the existing infrastructure as possible. Curb modifications would be required on the west side of Lakeshore Drive north of Tiffin St, the center median of Lakeshore Drive south of Tiffin Street and on south side of Tiffin Street. This work would likely take the form of minor works. A general line item for intersection modifications was included in the overall cost estimate.



Potential Curb Modifications

## TIFFIN STREET AND ESSA RD/BRADFORD STREET INTERSECTION:

The transit-only lane on Tiffin Street will merge with the general vehicle EBL at this intersection. This is because a separate WBL lane was determined to be impractical for the opening day time horizon because of property impacts. The dedicated left phase will be maintained with provision for transit signal priority coordination with station access road intersection. This configuration has been identified for the opening day of the station.



**Opening Day Configuration at Tiffin Street and Bradford Street / Essa Road** 

## POTENTIAL TRANSIT SIGNAL PRIORITY MEASURES:

The City has indicated that transit priority measures should be included in this area to save travel time and operational costs. The existing transit priority timing measures may be included at the station access road but will have to be coordinated with pedestrian phase timings. There is potential that signal priorities may be coordinated between the Lakeshore and Tiffin intersection and the Tiffin and Essa intersection with a radio frequency transit signal priority system. This could potentially allow for coordinated movements allowing buses to have priority for both left turn movements to access Essa Rd southbound from the new bus terminal. This type of analysis is not included in the current scope of this study as more detailed intersection movements may require a micro-simulation model.

## FUTURE ROAD NETWORK CONSIDERATIONS

In the future, the City has identified the potential expand this intersection to accommodate a complete transit-only lane with a dedicated transit-only phase. This would likely require a separate transit-only phase to provide a safe WBL that avoids conflicts with buses and private vehicles. A separate phase would also allow buses to more easily access the curb lane to serve stops on Essa Rd. It is likely that some property acquisition would be required on northwest side of the intersection to facilitate the geometry of the WBTH to Tiffin Street. This would need to be further studied if this long-term design solution is pursued and may be coordinated with any future redevelopment on the subject properties.



Potential Future Condition at Tiffin Street and Bradford Street/Essa Road

## 4.5 TRAFFIC IMPACT STUDY FINDINGS

The existing intersection capacity analysis results indicate that all of the study area intersections and individual movements are operating at Level of Service (LOS) 'C' or better during morning, afternoon, and weekend peak hours, except for the southbound left turn movement at the Lakeshore Drive and Tiffin Street intersection. This movement is currently operating at LOS 'F' during all the three peak hours with volume to capacity (v/c) ratio over 1.0.

## TRAFFIC STUDY METHODOLOGY

Future total traffic volumes were estimated for the opening year (2024) to incorporate future background traffic growth and future site traffic demands. A compound annual growth rate of 2% was applied to the existing (2018) balanced traffic volumes based on Barrie Traffic Staff recommendation, to estimate future background traffic volumes for three peak hour conditions. Future site traffic volumes were estimated considering the future bus routes serving the new mobility hub, and additional pick-up and drop-off vehicle trips that may generate from the growth of GO Transit ridership. All the bus trips will be using GO Station West Access and all new pick-up and drop-off vehicle trips will be using GO Station East Access.

For the future (2024) opening day analysis, two different signal timing priorities were established to provide a range of the likely traffic impacts. In the first set of analysis the signal timings were optimized to prioritize bus movements and to coordinate transit left hand turns from the station access road onto Tiffin Street and from Tiffin Street onto Essa Road. In the second set of analysis, the signal timings were optimized to reduce overall intersection delay, potential resulting in less priority for transit vehicles but reducing delay for other movements. These two sets of analysis provide a bracket to understand the potential range of delay at this level of study detail. The performance of the intersections is likely to fall within this bracket based on the final prioritization decisions that made in detailed design. The analysis for this work was completed using Synchro software that provides formula based results. A full microsimulation may be undertaken to fully optimize signal timings as part of the detailed design.

Additionally, two intersection lane configuration scenarios were assessed for the northbound approach at the Lakeshore Drive/Tiffin Street and Station West Access intersection. Scenario 1 included a single lane for northbound left movement at GO Station West Access, while Scenario 2 included dual lanes for northbound left movement at GO Station West Access. The analysis results for Scenario 2 show higher traffic delays for the Tiffin Street intersections, as compared to Scenario 1.

#### TRANSIT SIGNAL PRIORITY

Additionally, Transit Signal Priority (TSP) and signal coordination could be considered to reduce the vehicular delays and transit delays in the study area. Dedicated bus-only turn lanes are recommended for eastbound right and westbound left movements at Lakeshore Drive and Tiffin Street. Additional through lane in the east-west direction may be considered beyond 2024 to provide additional vehicular capacity on Tiffin Street.

#### SUMMER TRAFFIC CONDITIONS

The study area is uniquely situated to experience heavy traffic volume peaks on summer weekends as visitors from the GTA access surrounding recreational opportunities. Congestion on Highway 400 can create traffic infiltration onto city streets including within the Allandale area. Because the traffic counts for this study were completed in the winter months, comparative counts from the summer were provided by the City of Barrie in order to understand the likely impact of summer weekend traffic. Data provided for the intersection of Essa Road and Gowan Street from a Friday in June 2015, demonstrated an increase of 49% in approaching volume during the PM peak compared to the traffic counts in February. Analysis of the intersection performance with these increased traffic volumes revealed a reduced, but acceptable Level of Service.

Summer weekend traffic volumes are likely to further increase delays at the Tiffin Street and Lakeshore Drive intersection which operates near capacity at peak periods throughout the year. However, the volume increase at this intersection may not be as great as at Essa Road and Gowan Street because traffic diverting from Highway 400 is likely to somewhat disperse using Bradford Street. The increased delays likely on summer weekends further underscore the need to provide transit priority lanes in the area to ensure that reliable transit service is maintained during these periods of peak demand on the local road network. Additional study of the impact of summer weekends volumes on the Tiffin Street and Lakeshore Drive intersection may be considered for a future phase of this work.

#### **FINDINGS & RECOMMENDATIONS**

When signal timings plans were optimized to facilitate bus movements the overall intersection LOS dropped from 'A/B' in the future background traffic condition to 'B/C' in the future total traffic condition during the morning peak hour, from 'A/C/D' to 'B/D/E' during the afternoon peak hour, and from 'A/B' to 'A/C' during the weekend peak hour (Table 5). Especially for the intersection at Lakeshore Drive and Tiffin Street, the eastbound through and southbound left movements are expected to experience higher delays and longer queues during the afternoon peak hour. These impacts are mainly resulting from higher traffic demands and optimized signal timing settings to improve bus movements egressing from the station (the northbound left movement at Lakeshore Drive and Tiffin Street intersection).

When signal timing plans were optimized for general traffic operation at Lakeshore Drive and Tiffin Street intersection and at Essa Road and Tiffin Street intersection, both the intersections are expected to operate at an overall LOS 'D' or better during the three peak hours. Furthermore, the overall intersection delays are expected to be reduced by 2 to 12 seconds during the three peak hours (as compared to the future total traffic condition with optimized signal timing plans for bus movements).

Interaction			Intersection	
Intersection LOS (Delay) for	Condition	Lakeshore Drive and Tiffin Street	Essa Road and Tiffin Street	Lakeshore Drive and GO Station East Access
	Background Traffic	B (18 s)	B (18 s)	A (8 s)
AM Peak Hour	Total Traffic (optimized for bus)	C (21 s)	C (30 s)	B (11 s)
	Total Traffic (optimized for traffic)	B (19 s)	C (22 s)	B (11 s)
	Background Traffic	D (37 s)	C (24 s)	A (8 s)
PM Peak Hour	Total Traffic (optimized for bus)	E (59 s)	D (45 s)	B (16 s)
	Total Traffic (optimized for traffic)	D (47 s)	D (36 s)	B (16 s)
	Background Traffic	B (20 s)	B (19 s)	A (6 s)
Weekend Peak Hour	Total Traffic (optimized for bus)	C (24 s)	C (30 s)	A (9 s)
	Total Traffic (optimized for traffic)	C (21 s)	C (22 s)	A (9 s)

**Overall Intersection LOS Review** 

The complete traffic impact study can be found in Appendix D.

## 4.6 COST ESTIMATE

The estimate has been prepared to a 'Level D' basis of accuracy with figures provided conservatively to protect for future design progress. WSP developed unit price estimates based on comparable recent projects in the region. These examples ranged from 50% to 75% design completion and were selected to reflect the most current design standards and requirements from Metrolinx. Barrie Staff reviewed all cost estimates and unit costs based on recent local examples to more accurately reflect the local market and design precedents. Percentages were included for minor items, utilities and professional services and a reasonable contingency was added to account for the design items which will be addressed in future phases of planning and design. Archeological work is not included in the estimate and is assumed to be accounted for separately.

The functional design for the site is structured to strategically preserve as much existing infrastructure as possible. This includes maintaining four of the existing bays along the south side of the bus loop as well as the existing asphalt on the station access road and transit access driveway from Essa Road. Existing concrete sidewalk and curb infrastructure not impacted directly by the proposed work is assumed to

remain in place and is not included in the cost estimate. New asphalt work is assumed at a higher unit rate to account for excavation laying of granular. Existing asphalt areas are assumed to require a cosmetic repaving following construction to create a uniform appearance, repair any wear and tear from construction and allow for any minor grading work.

The project as depicted in the functional design drawings is estimated to cost approximately \$7.2 million to achieve full design and implementation.

## 4.7 ADDITIONAL DESIGN CONSIDERATIONS

## SIGNAGE AND WAYFINDING

The closure of the western station access road to private vehicles may create initial confusion and requires specific direction from around the area to the station's Passenger Pick-Up and Drop-Off area. The introduction of significant new transit-only infrastructure will also require deliberate design approaches as such facilities are not common in Simcoe County. Many similar bus terminals in the region struggle with frequent instances of drivers accidently or intentionally entering the bus-only area. Special signage and pavement treatment should be considered to avoid these problems. Furthermore, the site has been planned to minimize pedestrian and vehicle conflicts however there are inevitably places where pedestrian crossings are necessary. Special features such as lighting, signage and pavement should be considered to pedestrians and vehicle operators.

#### STATION SERVICES BUILDING

The station services building has been designed with sufficient room with accommodate the suggested program. The building may also include an enclosed vestibule which can continue to serve as a weather shelter for passengers even while the rest of the station building is closed. The exact layout and detailed program within the station building remains to be determined a future phase of design. Given the historic surrounding buildings and established character of the Allandale area it is likely that specific consideration will need to be given to these features. For the purposes of the functional design drawing, the station building is shown suggestive of the adjacent historic Allandale Station.

## **5 CONCLUSION**

## 5.1 FINDINGS

The Allandale site represents a feasible location for the relocation of the main Barrie Transit Terminal and offers several benefits to the City. The site can accommodate the program requirements of Barrie Transit service as well as GO Transit and inter-city coach operators while preserving significant existing infrastructure and encouraging investment in the surrounding area. This study has demonstrated a functional design that includes 13 bus bays on the existing property and which can operate safely and efficiently. The study has also demonstrated that Barrie Transit operations can be redirected to utilize the site with minimal incremental cost and disruption to the existing bus network. Finally, the traffic impact study has demonstrated that the site reconfigurations are unlikely to have a significant traffic impact on the surrounding road network.

By reconfiguring the existing station access road to a transit-only facility, operations at the site can be improved by removing the existing unsignalized intersection at Essa Road. The recommended system of bus-only lanes facilitates reliable and fast transit access to the central hub and GO Rail station, advancing the goals of Barrie Multimodal Transportation Master Plan, the Metrolinx Regional Transportation Plan and the GO Rail Station Access Strategy. Closing the bus access at Essa Road also improves the pedestrian experience along an emerging intensification corridor by creating space for an attractive public plaza and station services building. The location of the proposed bus facility also provides substantial buffers from the surrounding businesses and residential areas including the potential to retain existing mature vegetation along the north side of the property. The configuration of the bus bays around a landscaped island allows for focused pedestrian crossings, rational separation of bays amongst operators and preserves for long-term transit expansion.

## 5.2 NEXT STEPS

This work has advanced a functional design for the mobility hub site. Its intention has been to assess the feasibility of relocating the City's central bus terminal to Allandale station and to confirm that the site can satisfactorily meet the program requirements and high-level cost of developing the site. Further work is required to develop the functional design presented here into a detailed design with a refined cost estimate that can be taken to procurement and construction. Future work will include a more detailed investigation of considerations such as site utilities, geology and topography as well as architectural elements such as the design and materiality of the new station building, landscaping and shelters. Further work is also required to fully refine the traffic analysis at the site to optimize any potential transit signal priority measures through micro-simulation.

The functional design presented here provides the basis for further consultation with stakeholders, partner agencies and the local community within the City's broader planning framework of Allandale and the Essa Road intensification corridor.

Allandale Mobility Hub Study Final Report





## Allandale Mobility Hub Study **Policy Review Report**



March 2018



# vsp

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## 1. Background

The City of Barrie has retained WSP to study the costs and feasibility of shifting the Barrie Transit Terminal from its current location on Maple Avenue in downtown to a new site immediately adjacent to the Allandale GO Station. The relocation of the transit facility creates space in the downtown for a new farmers market and event space while consolidating the transit hub function at the GO Rail station to coincide with the arrival of all-day rail service.

The Allandale neighbourhood is a historic community with a wealth of civic amenities and natural heritage. Included within the Downtown Barrie Urban Growth Centre, the neighbourhood has also been a significant focus of planning and strategic investment by the City of Barrie for several years. This planning and investment framework, combined with the transportation and public realm amenities of the area are likely to lead to significant intensification in the future.

Situated at a natural crossroads at the head of Kempenfelt Bay, the station site has been a transportation hub for over 160 years. The station will serve as the northern terminus of two-way, all-day rail service from Toronto by 2024 given it an important role both within the City of Barrie, Simcoe County and to other points north. An active freight short-line railway owned by the City of Barrie skirts the site and crosses Essa Road just north of the station property.

It is important that the functional design work include a policy review to fully contextualize the policy framework for the site in terms of land use planning, urban design and transportation.

## 2. Key Findings

- While the Allandale area is currently relatively low density, the City of Barrie, in accordance with provincial planning policies, has established planning policies allowing for significant intensification of the area
- It is likely the community will grow significantly over the lifespan of any new transit facility
- Parking is highly constrained at the station meaning that ridership growth will have to come from other alternative means of station access
- Parking constraints at Allandale GO make the station attractive for public transit users
- The role of the station at the terminus of the GO Rail line means that other services may use the hub as a connection to destinations beyond the City of Barrie

## 3. Land Use Planning Policies

## 3.1 Growth Plan for the Greater Golden Horseshoe (2017)

In general terms, the Provincial Growth Plan for the Greater Golden Horseshoe (herein referred to as the 'Growth Plan') strives to support complete communities through the efficient use of land and resources. The Growth Plan defines a complete community as:

"Places such as mixed-use neighbourhoods or other areas within cities, towns, and settlement areas that offer and support opportunities for people of all ages and abilities



to conveniently access most of the necessities for daily living, including an appropriate mix of jobs, local stores, and services, a full range of housing, transportation options and public service facilities. Complete communities are age-friendly and may take different shapes and forms appropriate to their contexts."

The Growth Plan further expands upon the elements of a complete community:

"Applying the policies of this Plan will support the achievement of complete communities that:

- a) feature a diverse mix of land uses, including residential and employment uses, and convenient access to local stores, services, and public service facilities;
- b) improve social equity and overall quality of life, including human health, for people of all ages, abilities, and incomes;
- c) provide a diverse range and mix of housing options, including second units and affordable housing, to accommodate people at all stages of life, and to accommodate the needs of all household sizes and incomes;
- d) expand convenient access to:
  - *i.* a range of transportation options, including options for the safe, comfortable and convenient use of active transportation;
  - *ii. public service facilities, co-located and integrated in community hubs;*
  - *iii.* an appropriate supply of safe, publicly-accessible open spaces, parks, trails, and other recreational facilities; and
  - iv. healthy, local, and affordable food options, including through urban agriculture;
- *e) ensure the development of high quality compact built form, an attractive and vibrant public realm, including public open spaces, through site design and urban design standards;*
- *f) mitigate and adapt to climate change impacts, build resilience, reduce greenhouse gas emissions, and contribute towards the achievement of low-carbon communities; and,*
- *g) integrate green infrastructure and low impact development."* (Growth Plan, Section 2.2.1(4))

The proposed Allandale Station Mobility Hub is characterized as a 'Major Transit Station Area' within the terminology and constructs of Provincial policy. The Growth Plan (2017) defines a 'Major Transit Station Area' as:

"The area including and around any existing or planned higher order transit station or stop within a settlement area; or the area including and around a major bus depot in an urban core. Major transit station areas generally are defined as the area within an approximate



*500 metre radius of a transit station, representing about a 10-minute walk."* (Growth Plan (2017), Glossary)

To place this in context, 'higher order transit' is defined by the Growth Plan (2017) as:

"Transit that generally operates in partially or completely dedicated rights-of-way, outside of mixed traffic, and therefore can achieve levels of speed and reliability greater than mixed-traffic transit. Higher order transit can include heavy rail (such as subways and inter-city rail), light rail, and buses in dedicated rights-of-way." (Growth Plan (2017), Glossary)

Any future land use or development in proximity of the proposed Allandale Station Mobility Hub must occur in accordance with the Provincial principles for growth articulated above. However, development within proximity of the Allandale Station Mobility Hub must also occur at greater densities given that downtown Barrie and the lands contemplated for the new Allandale Transit Mobility Hub in particular, are identified as an 'Urban Growth Centre' by the Growth Plan.

Urban Growth Centres are the regional focal points designed to accommodate population and employment growth. These areas are intended to be the priority strategic areas along with transit corridors and station areas, to provide for intensification. Such areas are also the focus for municipal investment in transit, infrastructure and public service facilities. The Growth Plan provides that:

"Urban growth centres will be planned:

- a) as focal areas for investment in regional public service facilities, as well as commercial, recreational, cultural, and entertainment uses;
- *b)* to accommodate and support the transit network at the regional scale and provide connection points for inter- and intra-regional transit;
- c) to serve as high-density major employment centres that will attract provincially, nationally, or internationally significant employment uses; and
- *d) to accommodate significant population and employment growth."* (Growth Plan, Section 2.2.3(1))

The capacity for growth within the Urban Growth Centre to a large degree depends on the base population projections for the City as a whole. The Province has been very prescriptive in assigning population and growth projections to municipalities within the Growth Plan area. The Growth Plan requires that:

"Population and employment forecasts contained in Schedule 3 will be used for planning and managing growth in the GGH to the horizon of this Plan in accordance with the policies in subsection 5.2.4." (Growth Plan, Section 2.2.1(1))

Section 5.2.4 of the Growth Plan compels municipalities to apply the population and employment forecasts for managing growth to the Growth Plan horizon of 2041. The population and employment forecasts applied to the City of Barrie are provided in Table 1 below.



	Population	Employment
2031	210,000	101,000
2036	231,000	114,000
2041	253,000	129,000

#### Table 1: Growth Plan Population and Growth Projections for the City of Barrie

The City of Barrie has recently completed an Intensification Study (February 2018) in which the City's population projections for the 2041 planning horizon were allocated to various locations and forms of development within the City including: the urban growth centre, nodes and corridors, 'other' built boundary, greenfield and the new secondary plan areas. In reference to the lands surrounding the proposed Allandale Station Mobility Hub, relevant population allocations include those projections that the City has undertaken for the City's Urban Growth Centre and to the Nodes and Corridors.

The Essa Road corridor extends west from the proposed Allandale Station Mobility Hub and is a connecting link between Highway 400 and the City's lakefront. It is one of a number of nodes and corridors within the City and one of the City's priority corridors. The City has also recently initiated a study of the Essa Road corridor to identify redevelopment and intensification opportunities. Population projections that will influence development in proximity of the Allandale Station Mobility Hub are provided in Table 2 below.

Area	2016 Census		2031 Growth Plan		2041 Growth Plan	
	Population	%	Population	%	Population	%
Urban Centre	4,500	3%	13,400	6%	17,600	7%
Nodes & Corridors*	6,500	4%	12,00	6%	24,500	10%

#### TABLE 2: BREAKDOWN OF POPULATION ALLOCATIONS

Source: "Population and Employment Trends: Changing the Conversation About Intensification", City staff presentation to Council February 5, 2018

% = Percentage of overall City population

\* = These figures are a cumulative total for all nodes and corridors of which the Essa Road corridor is only one such corridor.

Generally, municipalities cannot designate a supply of land beyond the 2041 time horizon contained within the Growth Plan. However, the Growth Plan does provide for planning beyond this time horizon in 'strategic growth areas' such as the Urban Centre, transit station areas and nodes and corridors (including the Essa Road corridor) as follows:

"Within delineated built-up areas, municipalities may plan for development beyond the horizon of this Plan for strategic growth areas that are delineated in official plans and subject to minimum density targets, provided that:

*a) integrated planning for infrastructure and public service facilities would ensure that the development does not exceed existing or planned capacity;* 



b) the type and scale of built form for the development would be contextually appropriate; and

c) the development would support the achievement of complete communities, including a diverse mix of land uses and sufficient open space." (Growth Plan, Section 5.2.4(5))

There is significant development potential for lands in proximity of the Allandale Mobility Station Hub. Generally, this will take the form of redevelopment or infill development as there are very few vacant lots in this area.

The Provincial Growth Plan establishes minimum intensification targets for residential development within the Growth Plan Area municipalities. These are as follows:

- From 2022 to 2031 50% of all residential development occurring annually within the City must occur through intensification within the City's built boundary (Growth Plan, Section 2.2.2(2))
- From 2031 to 2041 60% of all residential development occurring annually within the City must occur through intensification within the City's built boundary (Growth Plan, Section 2.2.2(1))

Currently the City's Official Plan requires that at least 40% of all residential development must occur within the built boundary. (City of Barrie Official Plan, Section 3.1.2.3(b)) This requirement will be updated to reflect the 2017 Growth Plan through as part of the City's upcoming Municipal Comprehensive Review targeted to occur in 2018/2019.

The City has noted an underutilization of lands within the Urban Growth Centre and along its Nodes and Corridors. In order to meet both the 2031 and the 2041 Growth Plan density and intensification targets, a significant amount of additional high density growth will need to occur within the Urban Growth Centre and along the City's nodes and corridors. The Essa Road Study is one ongoing study that will address this issue. It is also anticipated that the City will be initiating an Official Plan Review project this year with an anticipated completion date of Q4 2019.

In order to promote compact development forms within the Urban Centres and along Transit Corridors and Station Areas, the Province has identified a minimum density target that municipalities must plan to achieve by 2031 or sooner. In the case of the City of Barrie, the Growth Plan prescribes a minimum target density of **150 residents and jobs combined per hectare** for lands within the delineated Urban Centre and within 500 metres of the proposed Allandale Mobility Hub which is characterized as a 'Major Transit Station Area' within the Growth Plan. (Growth Plan, Section 2.2.3(2)(c) and 2.2.4(3)(c))

In delineating the boundaries of a Major Transit Station Area, the Growth Plan requires that municipalities:

"...will delineate the boundaries of major transit station areas in a transit-supportive manner that maximizes the size of the area and the number of potential transit users that are within walking distance of the station." (Growth Plan, Section 2.2.4(2)

The Growth Plan defines 'transit supportive' as meaning:

"Relating to development that makes transit viable and improves the quality of the experience of using transit. It often refers to compact, mixed-use development that has a high level of employment and residential densities. Transit-supportive development will



be consistent with Ontario's Transit Supportive Guidelines. (Based on PPS, 2014 and modified for this Plan)"

At present, the City of Barrie Official Plan does not delineate the boundaries of any Major Transit Station Area around the proposed Allandale Mobility Hub lands. This will need to be done as part of the municipality's conformity exercise to bring the current Official Plan into conformity with the 2017 Growth Plan.

The Growth Plan requires that such Major Transit Station Areas be planned and designed to provide multimodal access to the stations and that lands in proximity of the stations be protected for possible future expansion. In this regard, the Growth Plan provides that:

"All major transit station areas will be planned and designed to be transit-supportive and to achieve multimodal access to stations and connections to nearby major trip generators by providing, where appropriate:

a) connections to local and regional transit services to support transit service integration;

*b) infrastructure to support active transportation, including sidewalks, bicycle lanes, and secure bicycle parking; and* 

c) commuter pick-up/drop-off areas." (Growth Plan, Section 2.2.4(8))

"In planning lands adjacent to or near higher order transit corridors and facilities, municipalities will identify and protect lands that may be needed for future enhancement or expansion of transit infrastructure, in consultation with Metrolinx, as appropriate." (Growth Plan, Section 2.2.4(11))

The Growth Plan further requires that lands within the Major Transit Station Area must be planned to support a diverse mix of uses and compact built forms at densities sufficient to support existing and planned transit service levels. In particular, the Growth Plan provides that:

*"Within all major transit station areas, development will be supported, where appropriate, by:* 

a) planning for a diverse mix of uses, including second units and affordable housing, to support existing and planned transit service levels;

*b)* fostering collaboration between public and private sectors, such as joint development projects;

c) providing alternative development standards, such as reduced parking standards; and

d) prohibiting land uses and built form that would adversely affect the achievement of transit-supportive densities." (Growth Plan, Section 2.2.4(9))

"Lands adjacent to or near to existing and planned frequent transit should be planned to be transit-supportive and supportive of active transportation and a range and mix of uses and activities." (Growth Plan, Section 2.2.4(10))



"Within major transit station areas on priority transit corridors or subway lines, land uses and built form that would adversely affect the achievement of the minimum density targets in this Plan will be prohibited." (Growth Plan, Section 2.2.4(6))

Collectively, the policies of the Provincial Growth Plan provide strong policy support for a significant amount of population and employment growth within proximity of the Allandale Station Mobility Hub.

# 3.2 Bill 139 Planning Act Reform

*Bill 139, Building Better Communities and Conserving Watersheds Act,* 2017, proposes changes to a number of pieces of legislation including enactment of the *Local Planning Appeal Tribunal Act,* 2017; *Local Planning Appeal Support Centre Act,* 2017; amendments to the *Planning Act, City of Toronto Act,* 2006 and the *Ontario Planning and Development Act,* 1994; as well as amendments to the Conservation Authorities Act and various acts consequential to the enactment of the *Local Planning Appeal Tribunal Act,* 2017. This piece of legislation received Royal Assent in the Ontario legislature on December 12, 2017 and has a scheduled Proclamation date of April 3, 2018. As of April 3, 2018 the legislative changes and associated regulations introduced through Bill 139 will fully take effect.

While Bill 139 makes a number of changes to the Planning Act, there are a few amendments that will directly impact development in proximity of the Allandale Station Mobility Hub. Bill 139 provides that:

- Municipalities may now designate lands in proximity of an existing or planned higher order transit station such as the proposed Allandale Station Mobility Hub as a protected Major Transit Station Area. Such areas will need to be delineated in the City's Official Plan.
- Official Plan policies applicable to a Major Transit Station Area must at a minimum identify the minimum number of residents and jobs per hectare, permitted uses and minimum densities for buildings and structures for such areas
- The legislation provides that where lands have been designated as a 'Protected Transit Station Area' within an Official Plan, the boundaries of such designation and any associated policies can only be appealed by the Minister (except with very limited exceptions)
- Zoning By-laws establishing densities and height regulations for Major Transit Station Areas equally, cannot be appealed except where such an appeal is permitted by resolution of Council

These legislative changes afford an added level of certainty in establishing the planning policies that will regulate land uses in proximity of the Allandale Station Mobility Hub.

## 3.3 City of Barrie Official Plan

The Province has identified downtown Barrie as a focal point to accommodate growth and development by identifying this area as one of the Provincial Urban Growth Centres within the Greater Toronto Area. Barrie's Urban Growth Centre encompasses the historic downtown and surrounding lands as well as the proposed Allandale Station Mobility Hub site.

The Province established the specific limits of Barrie's Urban Growth Centre in 2008. These boundaries along with the density target of 150 residents and jobs combined per hectare have been incorporated within the Official Plan along with a number of additional policies to direct the amount and form of



growth in this area. Existing capacity for intensification in the Urban Growth Centre is articulated in Section 3.1.2.3(d) of the Official Plan:

"The City's Growth Management Strategy identifies that the built up area, as identified on Schedule I, can accommodate an additional 13,500 housing units, of which 39% are in the Urban Growth Centre and 61% are outside the Urban Growth Centre."

The **'City Centre' designation** that has been applied to the Urban Growth Centre lands is one of 5 commercial designations within the City's Official Plan. The City Centre designation permits a variety of residential, commercial and institutional uses at higher densities and compact form as follows:

"Lands designated City Centre are intended to provide a broad range of retail, service, office, institutional, public and residential uses to serve the general needs of Downtown residents as well as specialized functions for the entire community and market area. Retail stores, offices, hotels, institutional, and entertainment uses shall be integrated, where possible, with residential uses, community facilities, and open space. The City Centre includes the Downtown Barrie UGC which is planned to achieve a minimum gross density target of 150 residents and jobs combined per hectare as identified on Schedule I -Intensification Area. (Mod E (cc))" (City of Barrie Official Plan, Section 4.3.2.2(a))

"The following uses may be permitted within the City Centre designation: commercial activities ranging from local service and retail use to business and administration uses; residential development, including the residential use of upper floor of commercial buildings; cultural and institutional uses; leisure and recreational uses; major office uses, and all levels of government and special purpose public agencies. (Mod E (dd)) Commercial uses shall be located so as to avoid an undue concentration of uses that reduce the quality of the pedestrian environment or have the potential to negatively impact the City's downtown revitalization efforts. (OPA 007, By-law 2011-084))" (City of Barrie Official Plan, Section 4.3.2.2(b)

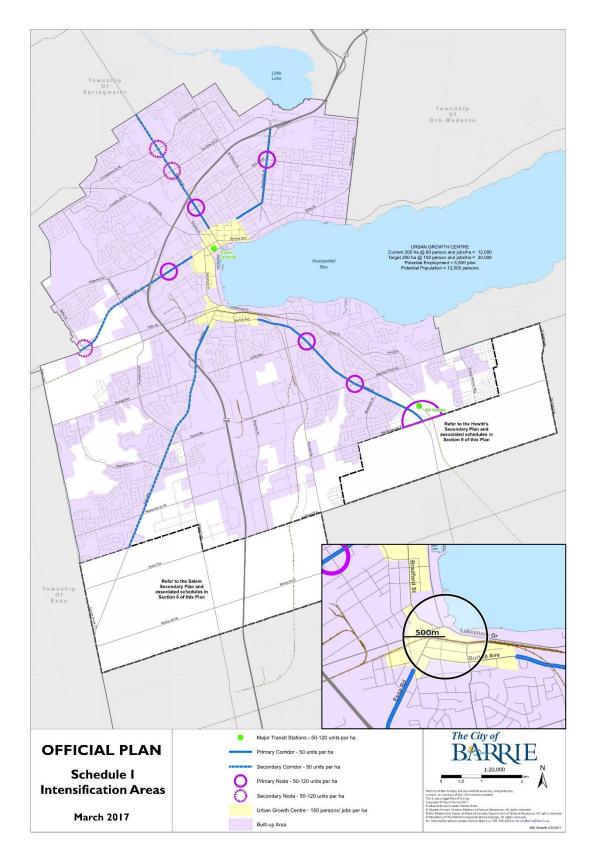
The Official Plan policies encourage a concentration of medium and higher density residential uses within the Urban Growth Centre and in proximity of a Major Transit Station Area in particular, in order to provide for live/work opportunities and to ensure an active downtown after business hours.

"Residential intensification is encouraged in a number of general locations in the City and shall be focused in the Urban Growth Centre, Intensification Nodes, Intensification Corridors, and the Major Transit Station Areas identified on Schedule I of this Plan

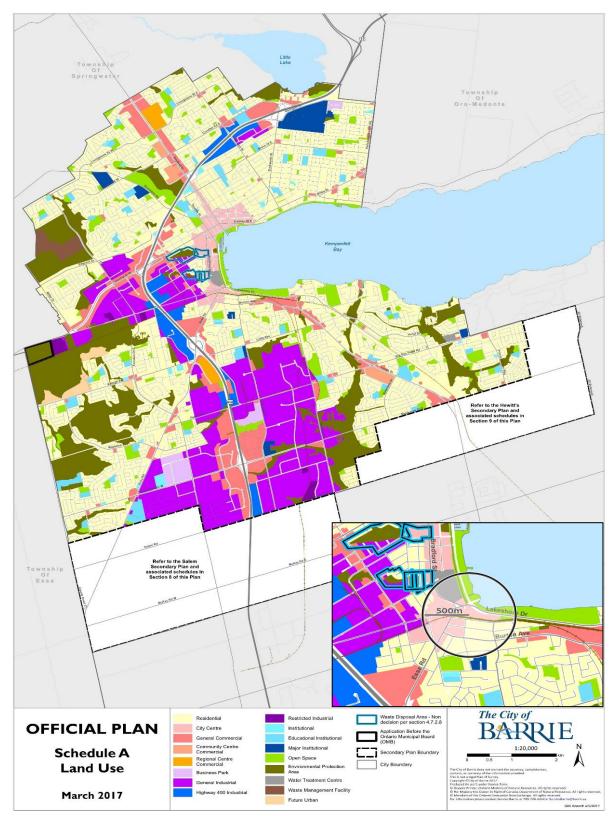
Development proposals for higher densities in other locations will be considered subject to the policies of Sections 3.3 and 4.2 of this Plan. (Mod E (s))" (City of Barrie Official Plan, Section 4.2.2.6(b))

"Intensification will contribute to development that is more compact and will efficiently use land and resources, optimize the use of existing and new infrastructure and services, support public transit and active transportation, contribute to improving air quality and promoting energy efficiency. (Mod E(t))" (City of Barrie Official Plan, Section 4.2.2.6(c))

#### City of Barrie Official Plan Intensification Schedule



#### City of Barrie Official Plan Land Use Schedule





Currently, the Official Plan provides for a target density of 50-120 units per hectare within a Major Transit Node.

"To achieve the goals of this section of the Plan, the following target densities shall be applied to the Urban Growth Centre, Intensification Nodes and Corridors and Major Transit Node identified on Schedule I of this Plan;

*iv) Major Transit Node* – *50 - 120 units per hectare (Mod E (x))*" (City of Barrie Official Plan, Section 4.2.2.6(g))

Lands in proximity of the proposed Allandale Station Mobility Hub but outside of the Urban Growth Centre boundary are designated for residential, general industrial and open space uses along the waterfront as shown on Schedule A of the Official Plan. Residential uses beyond the Urban Growth Centre are generally of a medium density form with some low density residential uses to the south beyond Burton Avenue.

The Official Plan further includes a number of policies to direct urban design both across the City and within the Urban Growth Centre in particular. These policies emphasize high quality design, pedestrian scale, views to Lake Simcoe, siting of tall buildings, compatibility and transition to lower density land uses beyond the Urban Growth Centre boundary.

The urban design policies of the Official Plan are complemented by a set of urban design guidelines for intensification areas in a document developed for the City in 2012 *"Urban Design Guidelines for Barrie's Intensification Areas"*. In particular, this document provides a number of urban design guidelines to shape development within the City's Major Transit Station Areas. This area would extend 500 metres (approximately a 10 minute walk) around the proposed Allandale Station Mobility Hub.

The goals of the guidelines for the Major Transit Station Area is to promote development that is transit supportive, promotes active streetscapes and that encourages active transportation alternatives. Excerpts from this document providing specific recommendations for the site design, streets and open spaces, built form and station design have been provided in Appendix A.

# 3.4 City of Barrie Zoning By-law

An Official Plan is the guiding policy document that establishes the vision, overarching goals, objectives, general land use principles and supporting policies to guide the City's evolution and development over the short and long term planning horizons. It is one of the municipality's primary tools to direct the actions of local government, shape development decisions and manage growth.

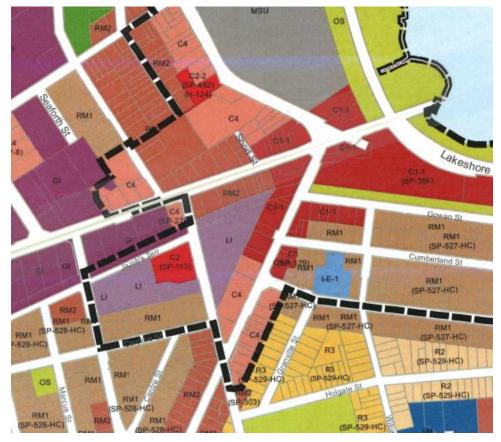
A Zoning By-law establishes and regulates the use of land by implementing the policies of the City's Official Plan. It provides the municipality with a legally enforceable means of regulating the use of land and applies performance standards that regulate the scale and intensity of development including:

- The types of buildings that are permitted and how they may be used;
- Where buildings and other structures can be located; and



• Lot sizes and dimensions, building heights, setbacks from the street, parking requirements associated with a permitted use and some aspects of building design.

Comprehensive Zoning By-law 2009-141 is the primary zoning by-law that regulates the use of land within the City of Barrie and the Allandale Station Mobility Hub Study Area in particular. Existing zoning within the study area is a mixture of zone categories.





#### Study Area

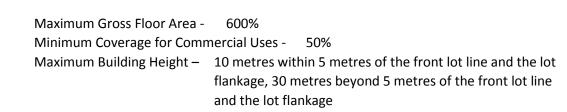
- Central Area One (C1-1) Commercial Zone (applicable to the proposed Allandale Station)
- Open Space (OS) Zone (along the Lake Simcoe shoreline)

The C1-1 Zone applying to the station lands and adjacent commercial uses permits a variety of commercial retail and service uses, office, accessory residential uses, institutional uses and uses permitted within the Residential Apartment Two (RA-2) Zone.

The C1-1 Zone has limited zone regulations as shown below:

Side Yard Setback Where C1-1 Zone Abuts a Residential Zone: 6 metres (does not apply to station lands)

Rear Yard Setback Where C1-1 Zone Abuts a Residential Zone: 7 metres (does not apply to station lands)



#### Proximity of the Study Area and Within the Urban Growth Centre

- General Commercial (C-4) Zone along Essa Road and Bradford Street
- Light Industrial (LI) Zone (west of station)
- Municipal Services and Utilities (MSU) Zone (water treatment plant)
- Residential Multiple One (RM-1) Zone (opposite station lands)
- Residential Multiple Two (RM-2) Zone (opposite station lands and along Tiffin Street).

Lands within the Urban Growth Centre boundary that are in proximity of the Allandale Mobility Hub are varied although the dominant zones are the General Commercial (C-4) Zone along Essa Road and Bradford Street which permits an assortment of commercial uses and the Residential Multiple One and Two (RM-1 and RM-2) Zones which provides for a range of low and medium density housing.

The existing zoning is fairly reflective of traditional zoning standards for such zones. Maximum heights being 14 metres (9 metres if adjacent to residential uses) for the C4 Zone and 10 metres for the residential zones. The current zoning has not yet been updated to reflect the policy direction of the Official Plan. Significant growth is anticipated in this area as the policy framework directs that significant growth be directed to areas within a Major Transit Station Area. The City has recently initiated the Essa Road Corridor Study to provide the context and necessary regulatory changes to promote increased density and development within this area.

#### 3.5 Future Development Potential

The Allandale area is a historic development node along the Lake Simcoe shoreline. As a result, lands surrounding the proposed Allandale Station Mobility Hub are already built out. There is very little vacant land in this area. However, many of the existing parcels are under developed. While there are currently no active applications in proximity of the Allandale station lands, there is a significant opportunity for infill development, redevelopment and lot consolidation. Such future development is anticipated in this area.

While the existing zoning in proximity of the Allandale Station lands does not facilitate the anticipated future growth potential of the area, the policy frameworks are in place at the Provincial level and to a lesser degree at the Official Plan level. The City has targeted to update the Official Plan through a Municipal Comprehensive Review that is scheduled for completion by Q4 2019. This will be the City's conformity exercise to update the Official Plan in accordance with the requirements of the 2017 Growth Plan. At the same time, the Essa Road Corridor Study will provide more specific direction as to the density, form and intensification of land uses along Essa Road.



All of these planning and legislative initiatives that have target completion for the short term will provide the necessary framework for development to occur in this area.

# 4. Transportation Plans

# 4.1 Metrolinx Regional Transportation Plan & GO Regional Express Rail

Allandale Waterfront GO Station is currently served by seven southbound departures in the morning and seven northbound arrivals in the evening on weekdays and three morning departures and evening arrivals on weekends. Trains on the Barrie corridor serve several stations in York Region and connect directly to Line 1 of the TTC Subway at Downsview Park Station. Bus service is currently available throughout the day when trains are not running, connecting to all-day rail service at Aurora GO.

As part of the GO Regional Express Rail (RER) program two-way, all-day rail service is currently indelivery between Union Station and Allandale Waterfront GO. The RER program is fully funded and is currently in design and procurement with completion anticipated by 2024. The full length of the Barrie Corridor is owned by Metrolinx and will be fully electrified and double tracked. New stations are planned to be added at 6<sup>th</sup> Line in Innisfil, Mulock Dr in Newmarket, Kirby Rd in Vaughan, Eglinton Ave in Toronto (Caledonia Station), Bloor St in Toronto (Bloor-Lansdowne Station) and Spadina Ave in Toronto. The two new stations in Toronto will include direct connections to the Eglinton LRT and the Line 2 of the TTC subway respectively. An EA for double tracking the entire length of the Barrie Corridor was completed in October 2017. The EA design calls for the addition of a second platform at Allandale Waterfront Station but does not indicate any property impacts. Separate EA's are underway for several of the new stations and the Rail to Rail grade separate at the Davenport Diamond.



Allandale Station Conceptual Design from Barrie Rail Corridor Expansion EA

Source: Metrolinx, Barrie Rail Corridor Expansion Project Environmental Project Report (2017)

The current RER service plan calls for hourly service between Union Station and Allandale Waterfront, with 15-minute service between Union Station and Aurora GO. In the peak period this service would be increased to every 30-minutes or better between Allandale Waterfront and Union Station. Service levels are planned to increase incrementally as double tracking and a new rail-rail grade separation is completed at the Davenport Junction in Toronto. The details of the RER service plan are still subject to



change as the implementation of the project continues. In March 2018 the Metrolinx Board received updated Initial Business Cases on the proposed new stations in which a new service model was proposed that would see all services from Allandale Waterfront run express between Aurora and Union Station with stops only at Downsview Park and Spadina Stations. In December 2017, rail service on the Barrie Corridor was increased to provide all-day service between Union Station and Aurora GO as well as weekend service throughout the year.

The 2041 Regional Transportation Plan (RTP) further outlines the visions and goals of the regional transportation network as well as planned rapid transit projects for over the 30-year planning horizon. The RTP calls for 15-minute electrified RER service to eventually be extended to East Gwillimbury GO. There are no mentions in the 2041 RTP about extending GO service north of Barrie including the .

# 4.2 GO Rail Station Access Plan

In response to the significant service expansions being delivered by the RER program, Metrolinx completed a revised GO Rail Station Access Plan in December 2016. The plan outlines high level strategies and objectives for how passengers will access different kinds of GO Rail Station and lays out specific plans and targets for each station. The plan envisions dramatically increasing the share of GO passengers who arrive at rail stations by local public transit.

The Allandale Waterfront GO station is identified as a Historic Suburban Town Center type station. This typology is characterized in the plan as having limited expansion opportunities because of surrounding established medium density development and a walkable environment with smaller block sizes. The station is seen as having significant potential to increase walking and cycling access and well as integration with local transit because of a central location within the local municipality.

There are currently 160 surface parking spaces at the station with an estimated utilization of over 90%. The plan calls for adding an additional 200 spaces at the station via alternative parking solutions. Alternative solutions may include things such as satellite lots connected by shuttles, shared parking with sites that have a different parking demand profile or peer-to-peer parking sharing which could allow local property owners to market available parking spaces to GO customers. The plan also calls for exploring opportunities for Metrolinx to lease additional lands in the area for parking including the open space area at the northwest corner of Lakeshore Drive and Tiffin Street.

The plan does not call for any expansion of the bus terminal at the site, but also does not expressly note the possibility of relocating the bus terminal from Downtown Barrie. The transit mode share target of 28-30% is slightly higher than the overall targets for the GO system indicating that Allandale has above average potential for access by local transit. Using data from the GO Rail passenger survey the plan identifies the areas of Letitia Heights, Sunnidale, Edgehill, Ardagh and Holly as concentrations of GO Rail passengers that could be served by local transit.

Furthermore, the plan calls for some specific interventions on the station property related to passenger pick-up and drop-off. This includes and on-street vehicle waiting area along Bayview Drive as well as unspecified medium-term measures to resolve conflicts between drop-off traffic and transit vehicles on the north side of the station.



# Current and Target Mode Split for Access to Allandale Waterfront GO Station (2016 GO Rail Access Plan)

Mode	Current Share (2015)	Target Share 2031
Walking	9%	14-16%
Local Transit	7%	28-30%
Micro-Transit	n/a	tbc
Cycling	0%	3-5%
Pick Up/Drop Off	20%	26-28%
Drive and Park	64%	26-28%
Carpool	0%	3-5%

Barrie South Station is identified as a Gateway Suburban Transit Node in the station access plan. This type of station is characterized by large catchment areas and a location on the periphery of the urban built area. As such, Barrie South station is planned to accommodate a larger mode share of pick-up and drop-off as well as drive and park passengers. The current parking utilization is around 80% and the plan identifies an expansion of 160 additional parking spaces at the station for a total of 779 parking spaces. By 2031 public transit is expected to accommodate 14-16% of the mode share to the station, up from around 1% today. An incremental expansion of the bus network in South Barrie, timed to meet train departures is suggested, along with long term expansion of the bus terminal at the station to potentially include a dedicated bus access or other transit priority measures. The Barrie South Station is seen as accommodating the overflow parking demand from Allandale Station.

Mode	Parking Expansion	New Transit Facilities	Other Facilities
Allandale GO	200 new spaces through alternative solutions (360 total)	None	<ul> <li>- 64 new covered and 24 secured bike parking spaces</li> <li>- Modifications to PPUDO</li> <li>- Add 12 on-street waiting areas (potentially on Bayview Drive)</li> </ul>
Barrie South GO	160 new spaces (779 total)	Potential improvements in medium term	<ul> <li>- 32 new covered and 16 secure bike parking spaces</li> <li>- Reconfigure PPUDO to reduce vehicular conflicts</li> </ul>

#### Infrastructure Recommendations in the GO Rail Station Access Plan

## 4.3 2011 Multi-Modal Active Transportation Master Plan Parking Study

The existing transportation master plan in Barrie was completed by Genivar (now WSP) in 2011. This study included a technical memorandum on parking in the downtown core and waterfront area. The study assumed a conservatively high auto access mode share of 73% for GO rail customers and found that parking deficiency of 389 parking spaces at Allandale Waterfront was offset by a similar surplus at Barrie South. Combined with an assumed increase in non-auto access mode share, the study concluded that there was no parking deficiency for GO Station access up to 2021. Furthermore, the study recommended increased transit access to Allandale GO station, parking restrictions in the surrounding community and a fare differential to encourage more customers to use Barrie South station and relieve the limited parking availability at Allandale.



# 4.4 Multi-Modal Active Transportation Master Plan Update:

The City of Barrie is currently undertaking an update to its Multi-Modal Active Transportation Master Plan (MMATMP) to address needs to 2041. This study will include a review of existing conditions, identify transportation improvements needed to support growth, assess alternatives based on different mode split scenarios and development a long-range implementation plan. WSP has also been retained to work on this study and the results of the Allandale Mobility Hub study will be shared with the MMATMP work in order to coordinate efforts.

## 4.5 Simcoe County Transit Study

The County of Simcoe is currently studying establishing an inter-municipal service at the country level which would operate in addition the existing services which currently operate between urban areas. This may include services such as the existing Route 90 to Angus and CFB Borden operated by Barrie Transit and GO Transit's route 68 which currently provides local service between Barrie and East Gwillimbury. Both of these routes currently use Allandale Station although the GO Bus only makes a brief stopover and currently lays over at the downtown terminal. In addition to the existing routes, Simcoe County is also proposing three additional routes which would operate from Barrie to Orillia, Midland and Wasaga Beach. Each route is assumed to operate on a 60 minute headway. The interim reporting completed up until August 2016 does not specify where this service would terminate in Barrie. It is reasonable to assume that such a service would require a recovery period at a terminal facility with facilities for drivers.

## 4.6 Barrie-Collingwood Railway

The Barrie-Collingwood Railway operates immediately adjacent to the Allandale Station on the south side of the Metrolinx tracks with a junction east of Essa Road. The railway then proceeds to cross Essa Road at grade with an un-signalized crossing. The railway is owned by the City of Barrie and operated under contract by Cando Rail Services as a short-line freight operation. It is currently used by a very limited number of industrial customers in the city and provides a connection to the MacTier subdivision of the CP mainline near its mile marker #58 at Utopia. The railway was once part of a continuous route to Collingwood however the portion of the tracks west of Utopia were abandoned in 2011 as maintenance was no longer financially viable. The Town of Collingwood portion of the rail corridor is currently being considered for conversion to a rail trail. Some portions of the track west of the CP mainline have recently been upgraded for rail car storage.

The financial viability of the line in Barrie remains uncertain as it does not typically cover its annual operating costs. Cando Rail Services is currently studying the potential of building a fuel transfer facility near the main rail junction at Utopia which could improve the long term viability of the railway. If additional sources of revenue are not found it is possible that the railway may face difficulty financing major state of good repair upgrades.



The rail right of way is being preserved to protect for the potential of rail passenger service to Collingwood in the long term. There are currently no plans to extend passenger rail service beyond Allandale Waterfront GO as there is limited demand and rebuilding the tracks would represent a significant expense. Metrolinx currently has no plans to extend service beyond Allandale Waterfront in the 2041 Regional Transportation Plan and the project is not included in the lists of potential project to take place beyond 2041.

For the purposes of the Allandale Mobility Hub Study, the corridor will be treated as an active freight siding with a protected right of way. The limited amount of traffic on the line, currently estimated to be under 5 trains per month, however means that the line will not be assumed to represent a significant operating obstacle to Barrie Transit operations.



Barrie Collingwood Railway Route Map

Source: Railway Association of Canada, Google Maps

# **5** Additional Information

## 5.1 GO Rail Passenger Survey and Cordon Counts

#### Station Ridership:

The 2014 Rail cordon count found that there were 478 boardings at the Allandale Waterfront GO Rail Station, the 2016 station access plans cited a slightly lower number of 325 home station passengers. The station access plan also assumed that the station would continue to fall within the lower end of GO Rail stations potentially attracting between 1000-2000 daily passengers by 2031.



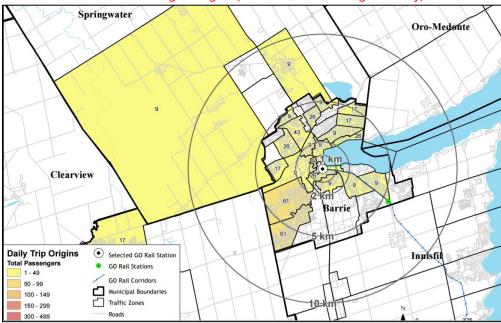
#### Passenger Origins:

The 2015 GO Rail Passenger Survey estimated passenger origins for the Allandale GO Station by extrapolating from a relatively small sample of responders who use Allandale as their home station. As such, the report cautions that the accuracy of the extrapolations are less certain than other GO Stations. Key findings from the survey include that over 81% of passengers come from within 5km of the station, making the site ideal for walking and cycling access. Very few people come from between 5-15km from the station and almost 11% of passengers come from more than 15kms, likely reflecting the terminus function of the station site.

mp Ongins	s by Distance	e nom Allan	uale wateri	IONE GO Sta		J Rall Passel	iger survey	()
<500m	500m- 1km	1-2km	2-3km	3-4km	4-5km	5-10km	10- 15km	15+
1.9%	5.4%	7.3%	12.7%	34.4%	20.0%	5.4%	1.9%	10.9%

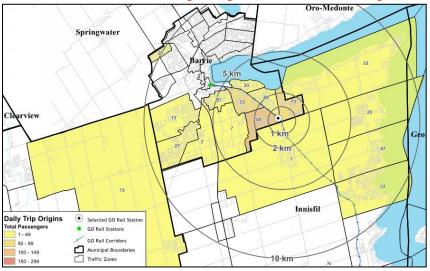
Trip Origins by Distance from Allandale Waterfront GO Station (2015 GO Rail Passenger Survey)

The distribution of origins identified particular clusters of GO customers in the Holly and Ardagh neighbourhoods of the city. Other secondary concentrations are found in the Letitia Heights and Cundles neighbourhoods. The neighbourhoods in the southwest of Barrie overlap with the catchment area for Barrie South GO station and may be inclined to access that station following parking expansion or the completion of a proposed new bridge over Highway 400 connecting Big Bay Point Road and Harvey Road. The catchment area for Barrie South station draws heavily from the immediately surrounding new subdivisions and generally from the southern portion of Barrie and the shoreline communities of Lake Simcoe. Almost no passengers were surveyed accessing Barrie South station from the northern portion of Barrie.



#### Allandale GO Station Passenger Origins (2015 GO Rail Passenger Survey)

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Barrie South GO Station Passenger Origins (2015 GO Rail Passenger Survey)

## 5.2 GO Fare Policy

It is possible that GO fare policy is responsible for creating a disincentive to use the Allandale Waterfront GO Station. The GO Transit fare zones in Barrie are configured such that Allandale and Barrie South stations fall in different zones, making trips from Allandale Waterfront GO more expensive than from Barrie South. The following table summarizes the fare differences for a trip to Union Station.

earrent rate Binerenee between Ananadie Waterholt and Barne South Go Stations					
	Allandale Waterfront GO	Barrie South GO			
Adult Single Ride	\$14.45	\$13.95			
Adult with PRESTO	\$12.83	\$12.39			
Typical Month	\$282.26	\$272.58			

Current Fare Difference between Allandale Waterfront and Barrie South GO Stations

For a regular commuter, using the Barrie South Station would save slightly less than \$10/month. This price point may be a significant enough factor to influence station access choice for sensitive commuters who use the service regularly or those who live close between stations.

# 5.3 Site History

For much of its history, the Allandale Station area was developed as a rail yard and served as a junction between railways destined for Muskoka and Collingwood. The lands which are currently being considered for a new transit terminal appear to have been occupied for over 50 years by a lawn bowling club which was moved to a new site by the City during the redevelopment of the Allandale Station lands. The station was first served by rail in 1853 as the temporary terminus of the Ontario, Simcoe and Huron Union Railway before its eventual extension to Collingwood in 1855.

#### Extent of Former Railway Lands in the Allandale Area



Source: Ontario Railway Map Connection <a href="http://ontariomap.webs.com/">http://ontariomap.webs.com/</a>

Allandale Station Area circa 1950



AERIAL VIEW OF KEMPFELT BAY AND BARRIE, ONTARIO, CANADA

Source: Barrie Historical Archive

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#### Allandale Station Area Circa 2000



Source: Barrie Historical Archive



#### City of Barrie Urban Design Guidelines for Major Transit Station Areas

#### General

- a) Ensure a high concentration of density and people working and/or living within a 10 minute walk (500 metre) walking distance of the station. High densities may be achieved through a variety of building forms.
- b) Plans should encourage a mix of uses near the station rather than dispersed, segregated uses. These uses should encourage activity in the area at all times of day, and should include convenience retail uses that support transit passengers, including cafés, dry cleaners, etc.

#### Site Design

- c) New local roads should be provided, where necessary, to ensure efficient pedestrian, cycling, transit and vehicle circulation through the Major Transit Station Area.
- d) Clear, weather protected outdoor paths should be provided to ensure comfortable connections for those transferring between different modes of transportation. As development occurs in Major Transit Station Areas, these connections can be integrated into new buildings.
- e) Local trails and bicycle routes should provide direct links to transit facilities.
- f) Auto dependent uses should be discouraged within Major Transit Station Areas, such as drive through retail and car wash facilities.
- g) Limiting surface parking should be a key objective at Major Transit Station Areas. A variety of opportunities should be explored, including bicycle parking and storage adjacent to building entrances, preferential parking for auto-share and hybrid vehicles, shuttles to/from key locations within the City, etc.
- h) Transit facilities should be located at public places in Major Transit Station Areas, such as community centres, parks and public open spaces, schools, and community facilities such as a library or gallery.

#### **Streets and Open Spaces**

- Streetscape improvements should provide a clearly defined pedestrian route to/from the Major Transit Station Area. This route should be distinguished from vehicular traffic areas by easily navigated, barrier free sidewalks, open spaces, walkways and/or well-marked crosswalks.
- j) At Major Transit Station Areas, social activities should be fostered through the inclusion of streetscape elements such as high quality street furniture.



- k) Major Transit Station Areas should include public open spaces, including parks and plazas, that serve as gathering/recreational spaces for the local and wider community.
- Wayfinding signage should be located throughout Major Transit Station Areas, ensuring easy navigation for those arriving to the City by transit.
- m) Public art, enhanced landscaping, and other landmark features should be provided at Major Transit Station Areas to reinforce the significance of these sites.

#### **Built Form**

- n) The station building within a Major Transit Station Area should be designed and massed as a landmark building to reinforce the importance of the site, and assist with wayfnding throughout the City.
- o) Buildings within Major Transit Station Areas should include canopies (and other weather protection at the building edge), and internal passages where feasible, to ensure a continuous, weather-protected connection to the station.
- p) It is particularly important that buildings within the Major Transit Station Areas have a pedestrian scaled building base (up to 4-storeys), in order to ensure an environment where pedestrians can comfortably walk or cycle to and from the station area

#### **Station Design**

- q) The main entrances at transit stations should include transit-related amenities (i.e. signage, ticket machines, etc.), as well as amenities for those who may be waiting for a connection (i.e. public art, seating, food, etc.).
- Transit stations should include an abundance of bicycle parking and storage facilities, in order to encourage users to connect to the station through active modes of transportation (i.e. cycling).
- s) Station design should adhere to the principles of Crime Prevention Through Environmental Design (CPTED) to ensure the safe usage of the station at all times of day.

Allandale Mobility Hub Study Final Report





# Allandale Mobility Hub Study Best Practices Report



March 2018



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Appendix: Best Practice Station Examples



# 1. Background

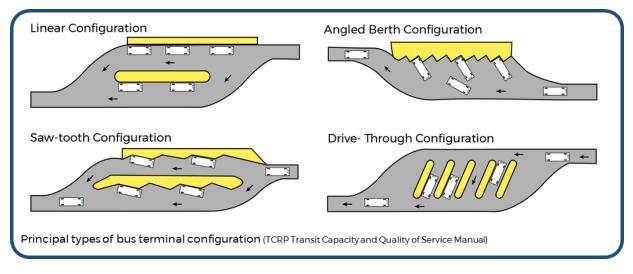
The City of Barrie has retained WSP to study the costs and feasibility of shifting the Barrie Transit Terminal from its current location on Maple Avenue in downtown to a new site immediately adjacent to the Allandale GO Station. The relocation of the transit facility creates space in the downtown for a new farmers market and event space while consolidating the transit hub function at the GO Rail station to coincide with the arrival of all-day rail service. The opportunity to create a new transit terminal at a landmark site within the City of Barrie presents an opportunity to adapt best practices from across the GO network and other transit terminals around the world. This review presents considerations of best practices from other sites which may be incorporated into the design of the new facilities at Allandale.

# 2. Terminal Design Typologies

## 2.1 General Types of Bus Terminal Configuration

The literature identifies a clear lack of common approach throughout the world in the optimal type of bus terminal configuration. Many different approaches are used based on different operating cultures. In the UK and New Zealand for example it is much more common to find reverse out facilities which are not favoured in Canada because of perceived operational complexity. In The Netherlands, most facilities use a drive-through configuration which increases spatial efficiency and is thought to improve legibility by positioning the front of the bus directly at the passenger waiting area. Other jurisdictions seek to limit pedestrian conflicts with buses as much as possible to ensure operating efficiency. Determining the optimal design for a facility is a reflection of the design values and operating priorities of the key stakeholders and facility managers.

The literature identifies four key configurations of bus terminal typologies: linear, angled berth, sawtooth and drive-through. Linear configurations allow buses to pull up flush with a straight curb and may include side platforms or islands. This configuration has the advantage of being space efficient and requiring little alteration from existing roadways. Its primary disadvantage is that it is able to accommodate fewer vehicles because of the space required for turnouts. Linear bays work best in locations where buses are not frequently passing each other such as places where buses stop only briefly or where service is infrequent. Saw-tooth configurations adapt linear terminals by angling each bus bay to reduce the amount of space needed to park and maneuver. They are most commonly found at bus terminals as they provide a mix of operational and spatial efficiency. Angled berths allow for greater efficiency relative to passenger boarding curb space. They work best in situations where buses tend to be parked for longer periods or time, where passengers are gathering in a centralized waiting areas and where there is a controlled access area for the buses to reverse into. Drive-through configurations allow for some of the spatial benefits of angled bays without requiring reversing. They also have the advantage of ensuring that the main route display information on the front of the bus is angled toward the passenger waiting area. Drive-through bays create the most pedestrian to vehicle conflicts and create difficulties for passengers with special accessibility needs.



Research has found that drive-through island configurations are typically the most spatially efficient but can introduce conflicts with pedestrians and require dynamic signage to maximize their capacity potential. Angled configurations, at angles of up to 90 degrees, are spatially efficient but increase operating complexity and may require a strict pedestrian exclusion area. More linear facilities are attractive for passenger comfort and reduce conflicts but experience problems with walking distances and passenger legibility beyond a certain size.

# 2.2 Reduced Footprints

Reducing bus terminal footprints can provide opportunity to consolidate passenger amenities, reduce walking times and accessibility concerns and free up land for transit oriented development opportunities. There are several best practices for reducing terminal footprints. The most important consideration is the configuration of the surrounding bus network. Timed transfers and pulses at terminals provide increased passenger mobility but dramatically increase the site footprint. Recovery times at stations also increases facility size. This can be overcome by providing off-site layover facilities so that multiple routes can share a single pick-up and drop-off bay.

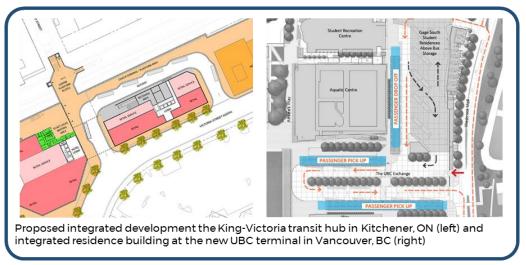
Bay assignments are often planned based around consistency and passenger legibility. Dynamic signage can help to overcome to this and provide flexibility to communicate changes and route assignments to passengers. Through-routing typically assigns two bays for each route, however in places where routes are frequent enough, routes may simply stop on-street allowing for faster operation and reduced delay for passenger travelling through the station. Bramalea GO station is an example of an existing facility where frequent services stop at enhanced on-street bays rather than entering the GO station property.

# 2.3 Urban Integration

A typical bus terminal is often segregated from the main public right of way in an enclosed facility. This allows for more seamless passenger transfers and a clean operating environment but may not optimize the potential for transit oriented development. Intensification and development of surrounding lands is often cited as an important objective of transit investment. The Allandale area has been clearly identified through provincial and municipal plans as a site for urban intensification. Throughout North America several best practices can be found in how to integrate transit terminals with land development. Reduced footprints are amongst the most common of such strategies as smaller bus terminals allow for more developable land and minimize disruption in the fabric of urban street

frontages. Examples such as the new Union Station Bus terminal in Toronto, Marine Drive in Vancouver and the dynamic terminals in Europe and Australia demonstrate how bus operations can be adjusted to open up land for redevelopment. The proceeds of land development may be directed back into further transit enhancements.

Aside from a reduced station footprint several other techniques are available. Urban areas generally require a fine grained street grid and bus terminals which allow new connections in the local street grid can create new development blocks as well as improve site circulation. Surrey City Center in British Columbia is an example of a location where the transit site is being broken up with new road connections to create new land parcels for development. Through an integrated planning process the operational needs of transit and the parcel needs of land-use planners were accommodated in a way that also utilized new roadways to shorten vehicle circulation travel times. The new UBC transit terminal in Vancouver is being planned with separate pick-up, drop-off and layover bays in order to reduce footprints and integrate a new student residence into the site.



Another common technique for accommodating intensification is to create a linear or square shaped parcel that preserves development blocks. Drive-through or multi-island configurations are often best suited for achieving this. Examples of this type of configuration include Christchurch, New Zealand and Central Park Station in Denver. This approach creates a square shaped transit terminal which avoids leaving wedges of land or irregularly shaped spaces that are less attractive for development. Linear terminal facilities can also be effective at preserving development blocks by wrapping around development sites. At the King-Victoria transit hub in Kitchener, plans call for coach and some local bus services to use such a facility with a one way transit-only access road surrounding a mixed-use development. This configuration likely requires the building's loading bay be accessed from the transit right of way meaning that some service vehicles such as delivery and garbage trucks must be granted access to the bus loop.

A final important best practice for urban integration is to preserve street frontages. Many of the best practice station examples include measures to either provide retail at the street frontage or protect for eventual redevelopment. Springfield station in Oregon and Ogden Central Station in Utah are both examples of facilities with a comparable scale and context to Allandale that include street fronting retail as well as preserve opportunities for further development. Large scale transit oriented development



projects typically require a lot depth of 40-50m to be viable but smaller retail frontages may take advantage of other station infrastructure such as share parking and servicing, to reduce land requirements. Street-facing retail benefits the broader neighbourhood while also providing increased activity at the site and services for local users. As the retail market at Allandale is likely somewhat limited, such facilities may be planned for future implementation by leaving appropriate space.

# 3. Bus Terminals at Commuter Rail Stations

### 3.1 Across the GO Rail System

Typical bus facilities at GO Rail stations use saw tooth bus bays configured in a tear drop or horseshoe. The Hamilton GO Centre uses reverse-out bays with static signage while the Union Station Bus Terminal employs dynamic signage to increase bus bay capacity. Most terminals at GO Rail stations are on Metrolinx property although select locations are owned and operated by the local municipality. Examples of municipal ownership include, Guelph GO and Brampton GO. Many GO stations include dedicated access and egress routes for public transit vehicles, something which is particularly important at stations with large park and ride facilities which generate significant traffic congestion.

Bus terminals at GO Rail stations typically do not include separate layover facilities or extensive on-site circulation with most layover occurring in the bay. Stations served by both GO and local buses typically segregate service areas by provider with GO buses often occupying the platforms closest to the rail facilities. At terminal stations, GO buses can layover for long periods of time due to infrequent schedules and long recovery times necessitated by travel on congested highways with variable travel times.

Stations being planned for the Bowmanville extension are considering designing loops that include dedicated layover parking and the ability to circulate back through the site to allow for separate layovers.



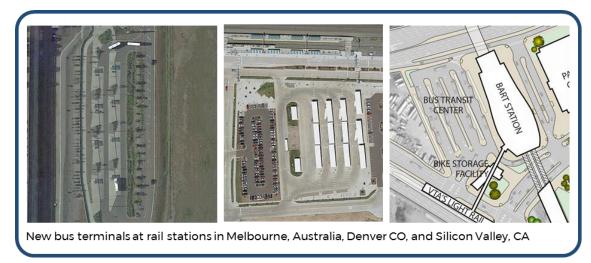
Recently completed bus terminals at CO Rail Stations in Burlington and Cormley

GO Station bus terminals typically emphasize separating bus and pedestrian conflicts with most new facilities orienting bays around the perimeter of the bus loop and leaving a center island dedicated to landscaping or storm water management. Newly completed bus terminals at Burlington GO and Gormley GO stations are examples of this. At Burlington GO station an existing island platform terminal was replaced by a horseshoe design with a landscaped interior. Across the GO Rail network, island platforms are somewhat rare. The recently completed Guelph central terminal is an exception, here an island platform was used, likely because of space constraints, with two pedestrian crosswalks connecting to the rail platform. New subway stations such as those on the recently opened Spadina

Subway extension avoid pedestrian conflicts either through side platforms or pedestrian tunnels connecting to central islands.

# 3.2 North American and Global Rail Stations

There is a wide variety of terminal configurations applied throughout North American rail stations. Island and finger platforms are common at new stations being built in Salt Lake City and Denver. New commuter rail stations in Florida typically arrange bus platforms along the station frontage allowing for direct connection to rail platforms but require a dedicated cross-walk for park and ride and other station users. Many stations in Australia and California attempt to bring the local road network close to the station, allowing for short loops that minimize diversion from the local road network. Stations in California in particular integrate attractive public realm spaces immediately adjacent to the facility. Recently renovated stations on the Cal Train system in Silicon Valley also include distinct facilities for micro-shuttles such as those operated by local tech campuses.



While most new terminal facilities at Canadian rail stations do emphasize separating pedestrian conflicts, this is not necessarily the case with new transit terminal facilities in other jurisdictions. Island platforms are quite common in recently completed transit projects throughout the United States. Site plans for several new stations in Denver include a series of one-sided island platforms with several pedestrian conflict points. In Australia, new rail stations in Melbourne are serviced by linear terminals that require some pedestrian and bus conflicts. Bus terminals at newer stations in Montreal have adopted lollypop or horseshoe configurations similar to those found at many GO rail stations.

Further details of examples of transit terminals throughout North America be found in the appendix.

# 4. Dynamic Bus Bay Assignment

# 4.1 Background

Dynamic bus bay systems allow the operators of a bus terminal facility to assign bus routes, or each bus arriving at the facility, to a specific bus bay in real time. This is opposed to conventional operations where routes are scheduled to have a specific platform whenever they arrive. The primary benefit of dynamic bus bay systems is that they allow for bays to be used more intensively, which potentially (depending on the service frequency of the bus routes and other factors) could reduce the number of bus bays required to serve a given volume of service or number of bus routes. Several different systems exist throughout the world with some able to use the CAD/AVL technology currently installed on many vehicles in Ontario. Routes are typically assigned to a group of bays to improve customer legibility.

Customers wait in a central area and are provided with audible and visual information about when their bus is arriving and what bay to proceed to.

Aside from a potential reduction in the number of bays required, the technology also provides several other benefits. A



Customer waiting areas in dynamic stations: Christchurch, NZ (left) and Perth Australia (right)

commonly cited benefit of the technology is the perception and feel of a modern transit experience. Studies have shown that the user experience is not encumbered by the process and the added passenger communication systems help to improve the perception of bus transit. The technology also allows agencies to concentrate passenger amenities in a central waiting area, improving safety and the potential viability of station retail.

The system is currently widely used in The Netherlands where it has been employed at central rail stations since the 1990's. Several cities in Denmark have also used the technology to compress bus facility footprints in downtown areas as part of integrated development projects. More recently, the technology has been implemented in Australia, New Zealand and the UK. In Christchurch, New Zealand and Perth, Australia the system was incorporated into new central bus hubs to help facilitate surrounding redevelopment schemes as well as to create an 'airport like' waiting experience for bus passengers to improve the attractiveness of public transit.

# 4.2 Applicability at Allandale Waterfront

Because the site is not particularly space constrained at the present time, Allandale Waterfront is unlikely to be an immediate candidate for dynamic assignment. However the staggered arrival times of buses at the facility may allow for dynamic assignment to reduce the required number of bays if there is pressure to open up lands at the station for additional development. Dynamic assignment may also be provided as a means of expanding future capacity without having to increase the number of bays. The presence of third party operators such as Simcoe County Transit, Greyhound and Ontario Northland



increases the complexity of a dynamic assignment system as these vehicles must either be coordinated with the system, segregated into a separate area or specifically dealt with a site manager.

The design of the new terminal may however protect the eventual implementation of the technology through measures such as concentrating customer amenities in a central waiting and either providing digital signage at each bay or protecting for it in the future by including necessary duct and conduit space. This protection has been done at other facilities including the Marine Drive bus terminal in Vancouver. Metrolinx is currently working on digital signage strategy at its stations which may provide a coordination opportunity with the City of Barrie. Digital signage that communicates bus route and arrival information may be implemented as part of a future dynamic assignment system without requiring its full scale operation. Metrolinx is also working on the first implementation of Dynamic Assignment in North America at the new Union Station Bus Terminal, providing an opportunity for coordination of technological and design requirements.

# 5. Electric Vehicles

#### **5.1 State of Practice**

Over the past several years there have been significant advances in the technology behind batteryelectric-buses (BEB) which has made their implementation increasingly practical for urban mass transit operators. Both internationally and within North America transit operators in a variety of different urban conditions are actively piloting electric bus operations. This includes large cities such as Los Angeles, which has committed to fully electrifying the second largest bus fleet in North America, to peer cities such as Brampton.

In Canada, several cities are currently in the process of procuring and testing battery electric vehicles. Vancouver, Edmonton and Toronto are all testing electric bus operations including special adaptations designed to improve the reliability of battery technology in extreme cold weather. Brampton Transit is currently involved in a collaborative pilot project with the Canadian Urban Transit Research & Innovation Consortium (CUTRIC) to test different electric bus systems from local manufacturers and equipment suppliers. Several large bus manufacturers including Nova Bus and New Flyer are actively bringing battery electric buses to market as are several start up corporations such as Proterra, based in California and the global leader BYD Auto based in China.

#### **5.2 Benefits and Implementation**

The primary benefits of electric buses are a dramatic reduction in local air pollutants. Electric buses eliminate the harmful emissions from diesel buses which may increase health risks in urban areas. Electric buses also operate more quietly than conventional diesel buses potentially dramatically improving the public perception of public transit buses. The vehicles are typically low-floor allowing for improved capacity and accessibility. Buses currently being brought to market may also offer reduced lifecycle costs because of reduced maintenance requirements, however initial capital costs remain significantly higher. Barrie may consider electric buses for implementation in areas where noise and perception is of particular concern such along the waterfront or within the downtown core.

# **5.3 Station Design Considerations**

The potential implementation of battery electric buses may have design implications for bus terminals such as the Allandale Mobility Hub. Current technology requires approximately 7-10 minutes for vehicles to fully charge and provide sufficient distance to cover a generous route length. A common method for charging at terminals is assisted automatic overhead docking stations which allow buses to pull forward making a connection with a wired connection using a small guide track on the roof of the bus. Such systems are currently being piloted in Seattle, Washington and Helsinki Finland. Wireless charging technology, potentially embedded under the roadway surface are also currently being studied and piloted.

New bus terminals may consider including some form of protection for future overhead electric charging facilities as well as design for capacity to allow for charging at the terminal. This could include protecting space for necessary transformer equipment and overhead clearance for charging stations. Conducting charging at a central hub allows for increased efficiency of scale but requires that buses layover longer at a central facility where space may be at a premium. Barrie Transit may consider which routes are likely to adapt electric buses and



protect for charging facilities only at bays that are planned to allocate to such routes. A dynamic assignment system may allow for different routes to use the bay with the charging facility so as to stagger charging times as necessary. As battery technology continues to improve, charging times and technologies will continue to evolve rapidly, making planning for a specific requirement difficult.

# 6. Integration with Active Transportation

Allandale station benefits from extensive connections to a surrounding active transportation network. The surrounding street network has a high intersection density allowing for good pedestrian movements as the area intensifies. To the south and west, surrounding uses are located close to the station and the Essa Road intensification corridor provides an attractive walking destination from the station. To the north, the site is adjacent to significant parkland and multi-use trail connections. As with many rail stations, transit infrastructure presents a connectivity challenge particularly to the south and east as there are few opportunities to cross the rail corridor and the GO rail layover facility located to the southeast of the station site along Lakeshore Dr.

## **6.1 Pedestrian Interaction**

Pedestrian best practices can be found in the form of complete streets guidance. Many large Canadian cities, such as Toronto, Edmonton, Calgary and Vancouver have developed complete streets guidelines which may be utilized as resources for the Canadian context. Internationally, the National Association of City Transportation Officials (NACTO) has produced the Urban Street Design Guide (2012) which includes resources and guidance about designing for pedestrian safety. Traffic calming and pedestrian safety measures are particularly important around rail stations where pick-up and drop-off traffic is prone to rushing to make train connections. Design features such as narrows lanes, reduced curb radii, pedestrian bulb out outs and other refuge points can help improve station access for pedestrians.



The Metrolinx Mobility Hub Guidelines speak to the importance of clearly marking pedestrian routes to the station that follow the most direct and simple path. Pedestrians are likely to follow the most direct route between destinations and pedestrian infrastructure should reflect this as much as possible rather than attempting to divert pedestrian movements by removing sidewalks or placing barriers. Station facilities are also important opportunities to utilize transit infrastructure to provide new connections through the community. Several GO Rail stations currently serve as important links across the rail corridor within their community. Port Credit and Exhibition GO station both position their pedestrian rail underpasses to align with local streets and provide connections to communities and attractions on both sides of the tracks.



Rail station infrastructure connecting communities at Exhibition and Port Credit GO Stations

## **6.2 Cycling Facilities**

The GO Rail Passenger survey indicates that the vast majority of passengers accessing Allandale Waterfront GO Station come from within 5km, making the site an ideal candidate for increased cycling mode share. Accommodating cycling at the station involves providing both for safe access routes and secure bike parking. Similar to pedestrian facilities, cycling routes should allow for a connection that is direct as possible and follows desire lines. Segregated cycling facilities provide a more comfortable and safe experience and have been found to improve perceptions of safety both by drivers and cyclist. Improving the quality and segregation of cycling facilities can help to attract a broader demographic to the cycling mode share. Raised curbs, bollards, or fully separate lanes are common approach to segregating cycling and vehicle traffic. In Quebec, provincial cycling policies prioritize infrastructure connections directly to the station building. At Mascouche Station, outside of Montreal, bike facilities at stations on a new rail extension were symbolically connected directly to the front door of the station, both facilitating good connections and providing a visual que to the safety and priority of cyclists.

GO rail commuters typically leave their bikes parked at the station for long periods of time at stations which experience fluctuating levels of passenger activity. This creates a situation where bike theft is a significant problem. Passengers who have their bike stolen from the station are less likely to continue seeking active transportation station access. Across the network, GO has added sheltered bike parking that keeps bike dry throughout the day. These facilities are usually placed in high-traffic areas and are built of glass in an attempt to keep the parked bikes as visible as possible. It is a best practice to keep bike parking in a well-lighted and highly trafficked area.



Secure bike parking (Toronto) and direct bike paths to the rail station (Montreal)

Metrolinx is currently moving towards providing secure bike parking at more rail stations. Secure facilities allow limited access to a bike parking facility for registered users. The station may be actively monitored by staff or simply controlled by fob entry key. The GO Rail Station Access Plan identifies Allandale as a potential site for secure parking, providing an opportunity for collaboration with Metrolinx. Some of the best bike parking stations in the world also include bike maintenance facilities to allow riders to perform common minor maintenance such as pumping tires or tightening bolts. Both Chicago and Washington DC have bike parking stations at their central transit hubs. Toronto Union Station includes a secure parking facility which is managed by the City and staffed during the day. In The Netherlands, bikes are a significant station access mode share and most large stations are supported by multi-level bike parking structures often accommodating thousands of bicycles.

## 7. Peer Network Examples

The following section looks at two similar examples of hubs in close proximity where one hub is located at a rail station and another is located in the downtown commercial area.

## 7.1 Burlington ON

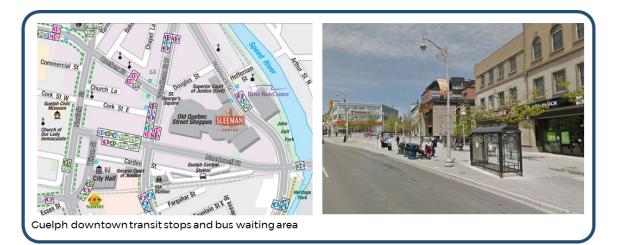
Burlington is an example of a location where the local transit agency provides hubs at a GO Station and a downtown business area which are approximately two kilometers apart. Both areas are designated as mobility hubs creating an opportunity to provide a corridor between the two sites. With a more established culture of GO Rail ridership, Burlington concentrates more service at the GO Station but provides overlapping routes to the downtown terminal as well ensuring a low headway connection for transfers between the two hubs. The downtown hub is located one block off of the main commercial street and includes high quality weather shelters as well as a small station building.

# vsp



#### 7.2 Guelph, ON

Guelph has a much less established culture of GO Rail ridership than Burlington having only received rail service in recent years. GO Rail service at Guelph is less than at Allandale Waterfront with only four trains daily in each direction between Kitchener and Toronto Union Station. The municipality completed at new transit terminal at a historic rail station adjacent to the GO platforms in 2017 replacing an onstreet hub within the downtown core. The station is still in very close proximity to the downtown core and routes continue to serve a cluster of on-street stops located closer to the main attractions and retail areas. Stop clusters are concentrated around two sides of an intersection, a strategy which helps to disperse bus movement while also shortening walking distances between bus transfers outside of the transit hub. A similar configuration is used for the downtown transit stop cluster in Kingston, Ontario with stops clustered on the north east corner of an intersection. In Kingston, a one-way road configuration allows for routes to be stopped on the same side of the street.



### Appendix: Best Practice Station Examples

(See Attached)

Barrie



Hamburg Poppenbüttel Station, image credit: Architizer

### **Best Practice Station Examples**

**March 2018** 

Barrie

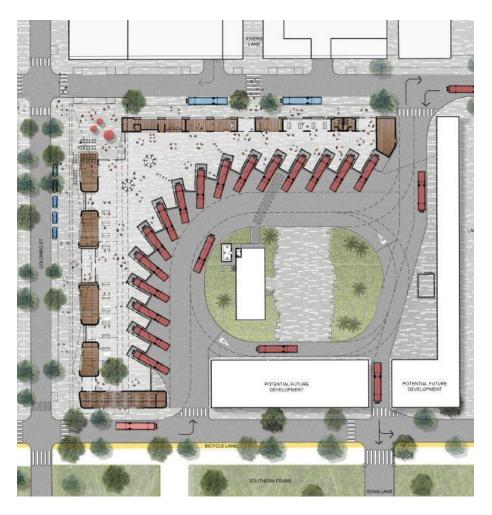
## **Christchurch Bus Exchange**

Christchurch, New Zealand

- Fully dynamic assignment and reverse out bus operation
- High quality passenger environment with climate control and retail amenities integrated to street
- Structured to allow for future joint development on the site as market conditions evolve







Barrie

## **Nijmegen Central Station**

Nijmegen, The Netherlands

- Fully dynamic assignment claimed to reduce station footprint by 70%
- Drive-through configuration allows for small footprint and square site plan
- Concentrated passenger waiting area
- Layover on local side streets to maximize station capacity
- Drive through creates accessibility challenges



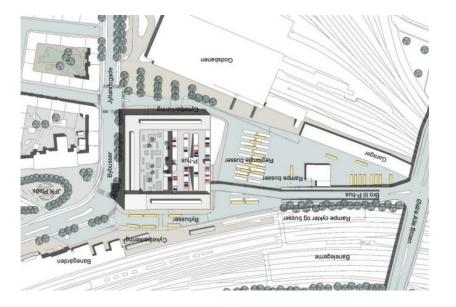


## **Aalborg Central Station**

Aalborg, Denmark

### Key Features

- A large 1960's era bus terminal was replaced by a smaller facility using dynamic terminal assignment
- Reduced footprint allowed for land sale to fund project and rejuvenate downtown area with new retail





Barrie

Barrie

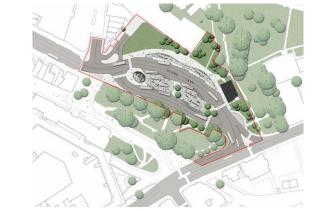
## **Chatham Waterfront Station**

Kent, UK

- 12 bays communicated as four platforms with a dynamic information to allocate individual bays
- Moved to waterfront from central downtown location as part of development
- Has experienced issues with wind and weather conditions
- Incorporated within broader park setting of redeveloped waterfront area







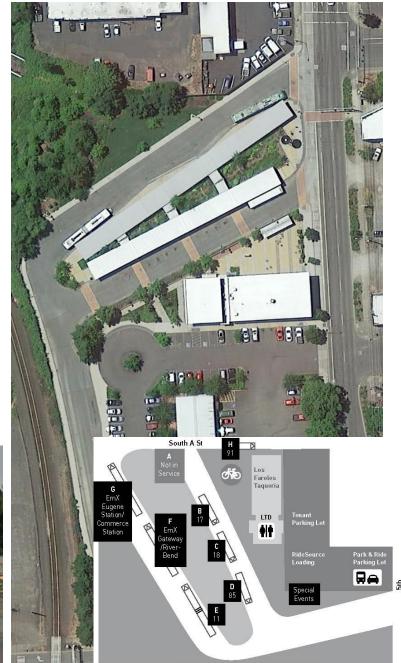


### Springfield Station Eugene, Oregon

### Key Features

- Eight bays accommodating eight routes including frequent articulated service from a BRT corridor
- Incorporates street retail, washrooms, sheltered bike parking, small park and ride and ride hail meeting point
- Dedicated bus-only access roads on surrounding streets
- Swale with natural planting and public art to channelize pedestrian crossings





Barrie

## **Marine Drive Terminal**

Vancouver, BC

### Key Features

- Several high-frequency routes share one pick-up and one drop-off bay
- Dynamic assignment was protected for but was not implemented
- Separate layover with driver facilities
- Integrated with a joint development project following opening of transit line
- Small footprint for high passenger volumes





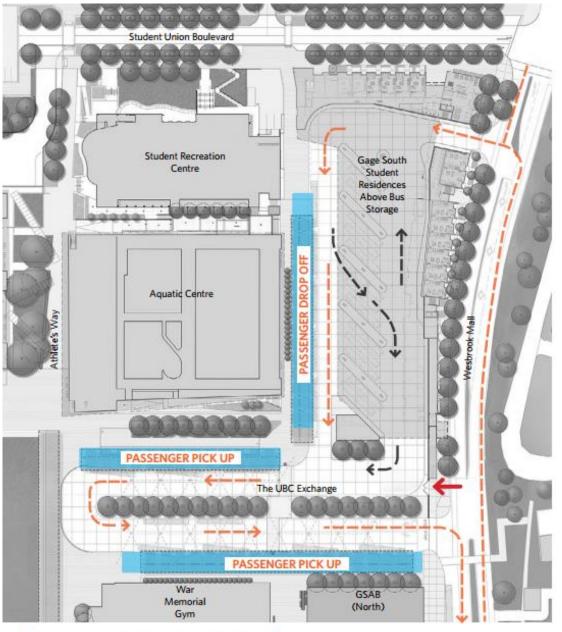
Barrie

# UBC Terminal

### Key Features

- Distinct areas for passenger pick-up and drop-off allowing for separate layover parking and reduced footprint
- Layover facility integrate within a new student residence development
- Surrounding route network largely terminates at the site





🔶 — passenger drop-off and pkk-up 🛛 🗲 — bus layover

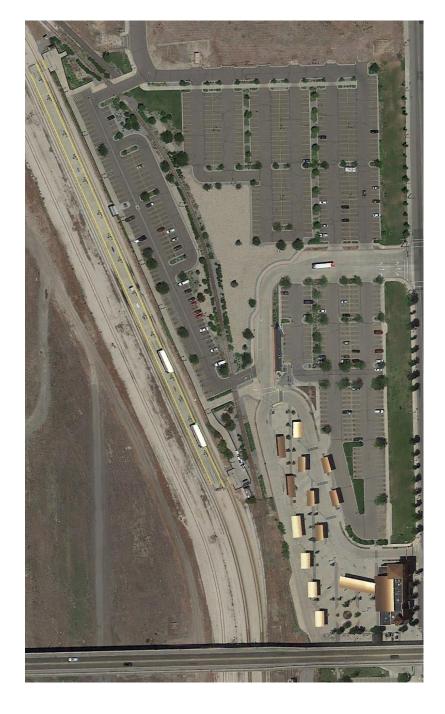
emergency access



### Ogden Transit Centre Salt Lake City, UT

- 11 Local Regional Bus Routes and Intercity Greyhound service
- 10 bays + 2 reverse out coach bays
- Protects street frontage for future retail and office developments using shared parking
- Opened in 2002 and expanded to accommodate new commuter rail service operating approximately hourly in 2008
- 'Finger' platform with three bus access points, six pedestrian crossing points







Barrie

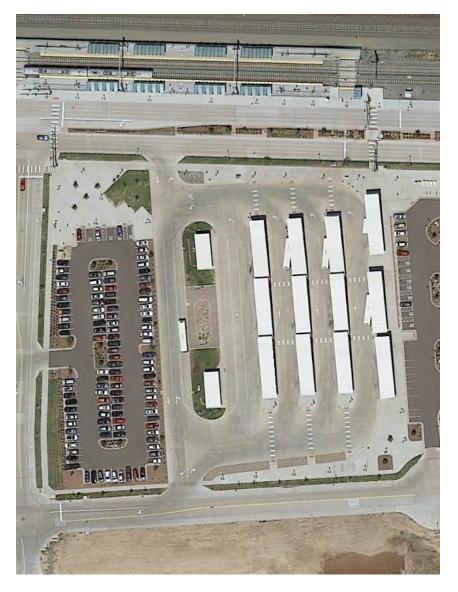
## **Central Park Station**

Denver, CO

- 12 bays serving 10 local transit routes
- Opened in 2016 as part of completely new commuter rail line
- Delivered as part of large P3 package
- Special drop-off points for micro-shuttles
- Two pedestrian crossing at mid point of islands as well as marked crossings at ends of each island
- Integrated within large surrounding area redevelopment plans





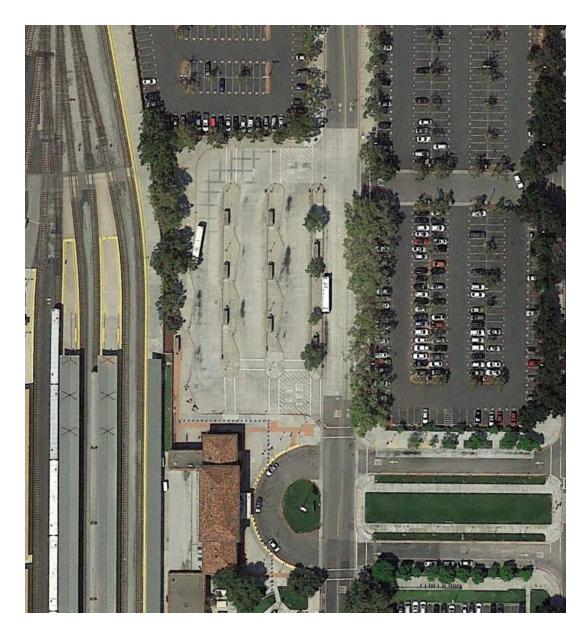


## **San Jose Diridon Station**

San Jose, CA

- Multi-modal Hub with LRT, Intercity Rail, Commuter Rail, Local Buses and Intercity Coaches
- 12 bays service 12 local bus routes, four intercity coach operators and several employer shuttle buses
- Drive through configuration with a combination of saw-tooth and linear bays







Barrie

### Gare Mascouche Montreal, QC

- Terminal station of a new commuter rail line opened in 2014
- 9 Bays serving 10 different route directions
- Landscaped center swale to prevent pedestrian conflicts
- Direct connection via off-road cycling path
- Compact bus terminal design, few pedestrian conflicts





Barrie

### Guelph Central Station Guelph, ON

- Includes Greyhound, Via Rail, GO Transit and Guelph Transit users
- Re-opened as an integrated facility in February 2017
- Ticket counters, washrooms, driver lunchroom and management facility located in historic station building
- 15 routes and 5,000 daily passengers
- Compact footprint in irregular station site





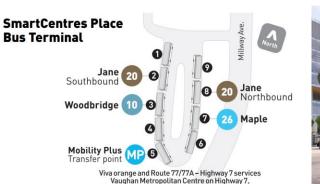


## **Smart Centers' Terminal**

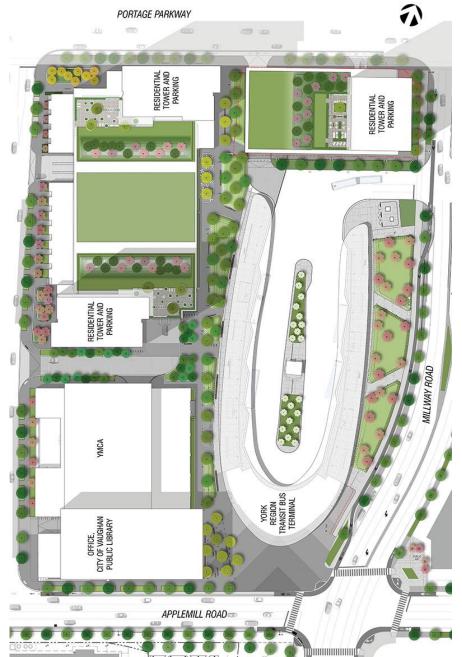
Vaughan, ON

- Nine bays, including space for articulated vehicles
- Serving three routes and accessible transit
- High quality urban design and building materials
- Centre landscaped median with public art component covering vent shaft
- Connected by tunnel to subway station
- Budgeted cost of \$32.1 million







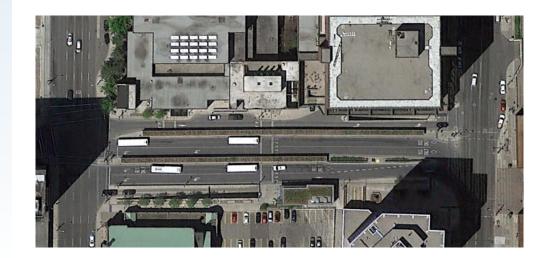


Barrie

### MacNab Terminal Hamilton, ON

### **Key Features**

- 7 platforms with 10 routes
- Two level passenger services building with washrooms and real-time arrive display information
- New public realm space
- Glass barriers provide wind shelter and focus pedestrians at a single crossing
- Award winning design excellence





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### West Harbour Station Hamilton, ON

- 3 platforms including one on street
- Express routes pass the station on-street
- Sheltered bus waiting area
- Bus loop integrated within a new public realm plaza that includes public art and extensive landscaping
- Part of \$50 million new GO Station development
- Limited rail service planned to expand in the future











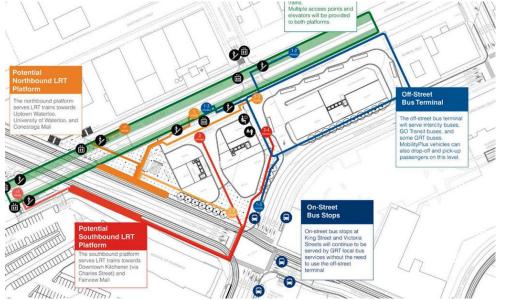
### King-Victoria Transit Hub Kitchener, ON

- Ambitious project to develop an integrated transit hub along with significant office, residential and retail development
- RFQ released in 2017 currently under review to find a master developer
- Includes \$43 million in public funding
- Will eventually include Light Rail, GO Rail and intercity coach buses
- Bus terminal wraps around development block





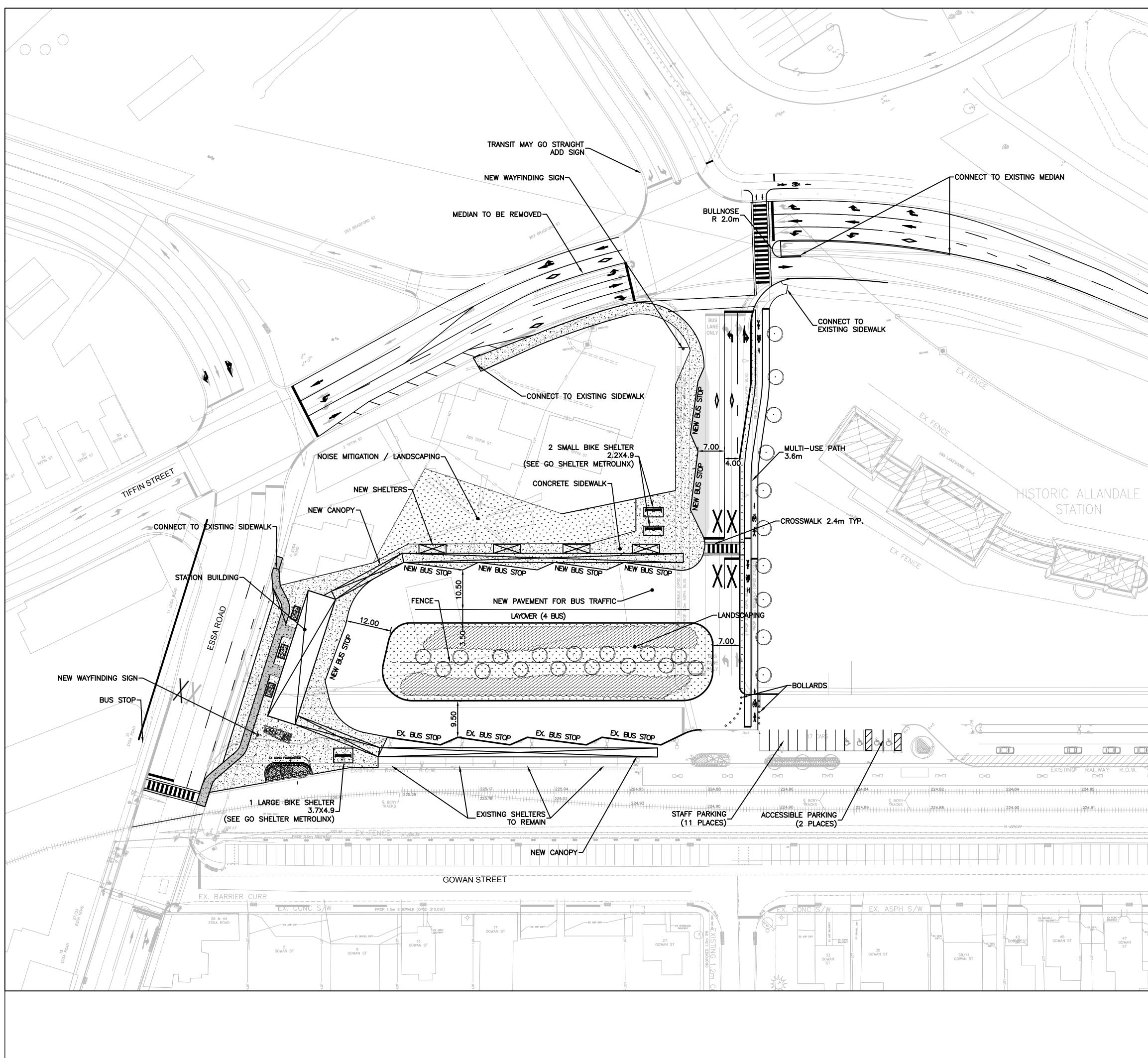




Allandale Mobility Hub Study Final Report

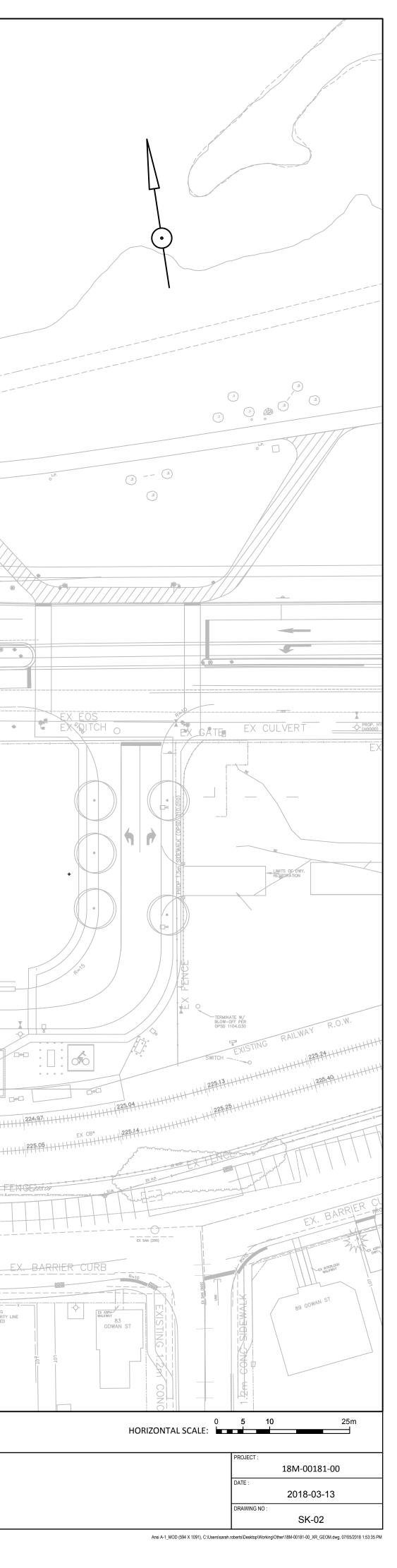


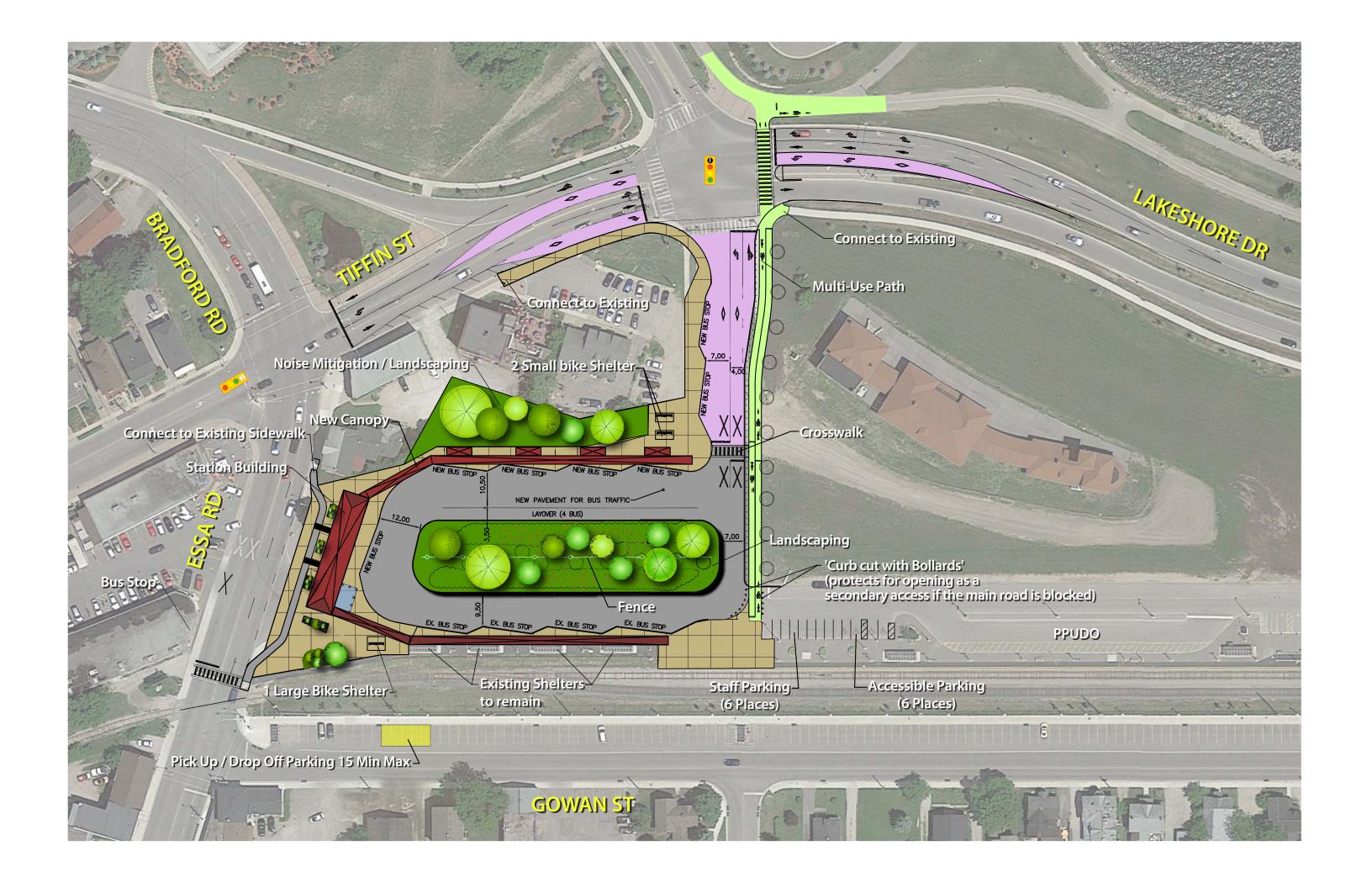




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Allandale Mobility Hub Study Final Report





#### **CITY OF BARRIE**

### BARRIE ALLANDALE MOBILITY HUB STUDY Traffic Impact Study Report

MAY 31, 2018

FINAL



# wsp



### BARRIE ALLANDALE MOBILITY HUB STUDY TRAFFIC IMPACT STUDY REPORT

**CITY OF BARRIE** 

FINAL

PROJECT NO.: 18M-00181 DATE: MAY 31, 2018

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### **1 INTRODUCTION**

The City of Barrie is conducting a mobility hub study of the Allandale Waterfront Station to assess the feasibility of relocating Barrie Transit's central terminal to the site. This has the potential to benefit the city by improving transit connections to GO Rail in advance of the of start of all-day service and of freeing up space in the downtown for a new civic amenity. The existing bus terminal located on Maple Street in downtown Barrie serves Barrie Transit as well as other inter-city operators (GO Bus, Greyhound, and Ontario Northland). The proposed new transit terminal at Allandale would concentrate services to create an intermodal regional transportation hub at a prime redevelopment and intensification site. The City has long term growth plans to encourage intensification and investment in the historic Allandale community.

As part of the Allandale Station Mobility Hub Study, a Traffic Impact Study (TIS) was conducted for the existing (2018) and future (2024) planning horizons to assess the potential impacts of the proposed improvements on the transportation network. The report identifies the existing traffic operational issues, future capacity constraints, and recommended improvement measures to mitigate the future traffic impacts.

This TIS report summarizes the study area (in Section 2), Existing (2018) traffic conditions (in Section 3), Future (2024) background traffic conditions (in Section 4), Future (2024) total traffic conditions (in Section 5), Study Findings (in Section 6) and Recommendations (in Section 7).

### 2 STUDY AREA

The existing Allandale GO Station is located at the southeast corner of Essa Road/Bradford Street and Tiffin Street intersection, approximately 1.5 km south of Barrie downtown. Currently, the station can be accessed via three intersections – using two Lakeshore Drive intersections to the north platform, and using Essa Road and Gowan Street intersection to the south platform. An additional bus-only access is located north of the Essa Road and Gowan Street intersection.

The study area for the traffic analysis is presented in Figure 1. The following five intersections in the vicinity of existing Allandale GO Station were analyzed in this study:

- Essa Road/Bradford Street and Tiffin Street intersection (currently signalized),
- Lakeshore Drive and Tiffin Street intersection (currently signalized),
- Lakeshore Drive and GO Station East Access (currently signalized),
- Essa Road and Gowan Street intersection (currently signalized), and
- Essa Road and GO Station South Access (inbound bus-only and currently not signalized).

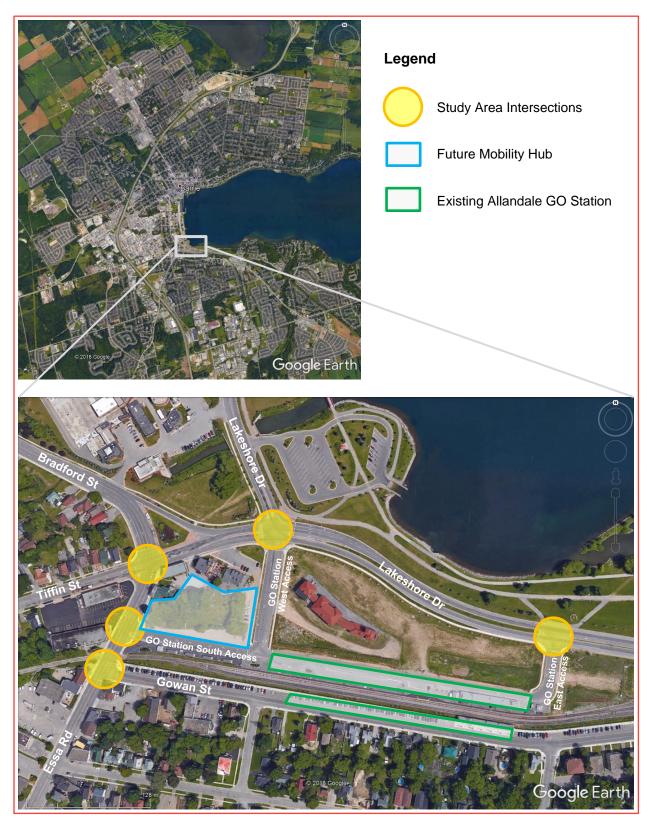


Figure 1: Study Area

### 3 EXISTING (2018) TRAFFIC CONDITIONS

#### 3.1 EXISTING (2018) ROAD NETWORK

The study area includes following corridors:

- Lakeshore Drive is a City Parkway traveling in the north-south direction north of Tiffin Street and in the eastwest direction east of GO Station West Access. This corridor includes two-lane cross-section within the study area, and operates with signalized intersections at Tiffin Street/GO Station West Access and at GO Station East Access. Lakeshore Drive widens and accommodates an additional through lane on the east approach to Tiffin Street/GO Station West Access. The posted speed limit is 50 km/h on Lakeshore Drive within the study area.
- *Bradford Street* is an Arterial corridor in the north-south direction parallel to Lakeshore Drive, with a four-lane cross-section within the study area, and operates with a signalized intersection at Tiffin Street. The posted speed limit is 50 km/h on Bradford Street within the study area.
- *Tiffin Street* is also an Arterial corridor, providing access in the east-west direction and connect to Lakeshore Drive at the Allendale GO Station West Access. The segment in the study area includes two-lane cross-section and operates with signalized intersections at Bradford Street and at Lakeshore Drive/GO Station West Access. The channelized westbound right turn lane from Tiffin Street to Bradford Street forms a triangle landscaping island. The posted speed limit is 50 km/h on Tiffin Street within the study area.
- *Essa Road* is an Arterial corridor, traveling in the north-south direction and connects to Bradford Street at Tiffin Street intersection. It includes four-lane cross-section within the study area, and operates with traffic signals at Tiffin Street and at Gowan Street intersections. The signalized intersection at Gowan Street also accommodates an at-grade railway crossing. A 'bus-only' access to the GO Station is located 20 m north of the Essa Road and Gowan Street intersection. Within the study area, the posted speed limit is 50 km/h on Essa Road.
- *Gowan Street* is a Minor Collector traveling in the east-west direction and providing access to the GO Station (on the north side of the road) and local residential areas (on the south side of the road). It has a two-lane cross-section and operates with a signalized intersection at Essa Road. The posted speed limit is 50 km/h on Gowan Street.

The existing intersection lane configurations for the study area intersections are presented in Figure 2.

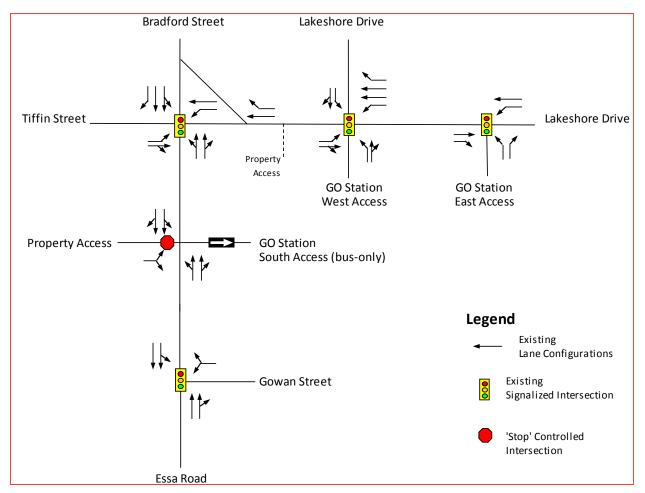


Figure 2: Existing Intersection Lane Configurations

#### 3.2 EXISTING TRANSIT OPERATIONS

The Barrie Transit currently operates ten bus routes serving the study area, as described below and presented in Figure 3:

- Route 1A (Georgian Mall-northbound) and Route 1B (Welham-southbound) operate along Bradford Street/Essa Road with a 30-min headway during Monday to Saturday daytime, and a 45-min headway during Monday to Saturday evening and on Sunday.
- Route 4A (East Bayfield-northbound) and Route 4B (South GO-southbound) operate along Bradford Street/Essa Road with a 35-min headway during Monday to Saturday daytime, and a 65-min headway during Monday to Saturday evening and on Sunday. During Monday to Friday morning hour (5:45 to 6:45), service headways is reduced to 30-min for both routes.
- Route 7A (Bell Farm-northbound) and Route 7B (Bear Creek-southbound) operate along Bradford Street/Tiffin Street with a 30-min headway during Monday to Saturday daytime, and a 60-min headway during Monday to Saturday evening and on Sunday.
- Route 8A (Yonge-southbound) and Route 8B (Park Place-northbound) operate along Bradford Street/Essa Road with a 30-min headway during Monday to Saturday daytime, and a 60-min headway during Monday to Saturday evening and on Sunday.
- Route 90A (Angus Borden-eastbound) and Route 90B (Peacekeepers Way-westbound) operate along Tiffin Street with five departures daily from Monday to Friday.

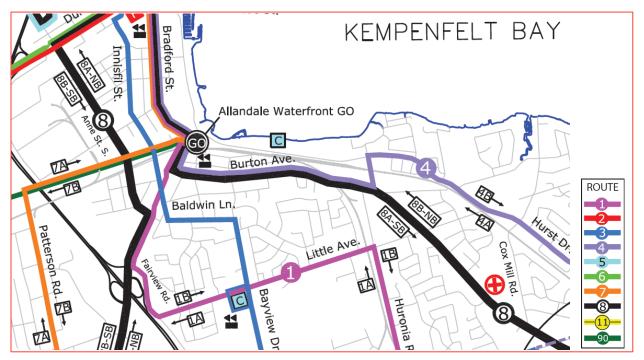


Figure 3: Barrie Transit Map (partial) Source: Barrie Transit

In addition to the Barrie Transit, GO Train operates from Allandale GO to Union Station during the morning period and in the opposite way during the afternoon peak period. There are seven train departs from Allandale GO to Union Station during weekday mornings (between 5:18 and 7:18) and three train departs during weekend mornings (between 9:15 and 11:15), and vice versa during the afternoon periods. The GO Bus provides services between Allandale Waterfront GO and Brantford GO station daily with a headway of 45 min to 70 min.

#### 3.3 EXISTING TRAFFIC VOLUMES

The turning movement counts (TMCs) were collected for the study area intersections in February 2018, for eighthour period for the typical weekday and four-hour for weekend conditions. The TMCs were reviewed and compiled to reflect the weekday morning and afternoon peak hour conditions, and weekend peak hour condition. Traffic volumes for these three peak hours (morning, afternoon, and weekend) were further balanced to ensure consistency between two adjacent intersections. Truck percentages and peak hour factors (PHFs) were also calculated and applied in intersection capacity analysis.

The existing (2018) traffic volumes for the study area intersections are presented in Figure 4. The collected traffic counts are included in in Appendix A.

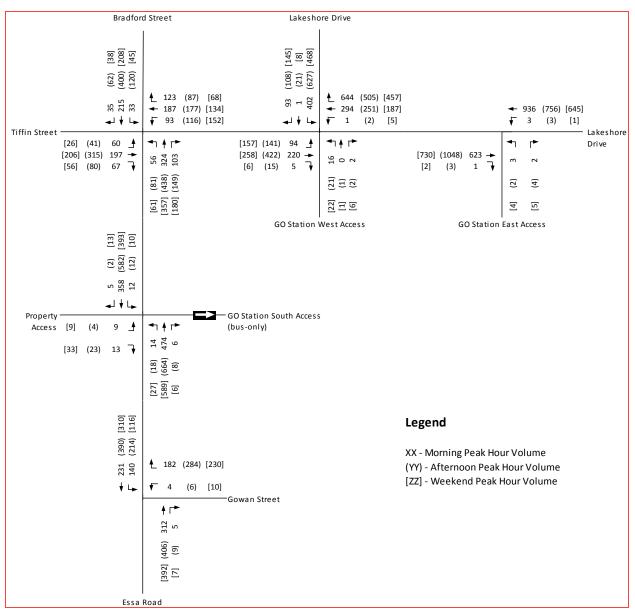


Figure 4: Existing (2018) Traffic Volumes

## 3.4 SIGNAL WARRANT ANALYSIS

Signal warrant analysis was conducted for Essa Road and GO Station South Access using existing (2018) eight-hour traffic counts as per the Ontario Traffic Manual (OTM) Book 12 guidelines. The result of signal warrant analysis indicates that traffic signal is not justified at Essa Road and GO Station South Access for the existing traffic conditions. The analysis summary is presented in Figure 5.

Detailed signal warrant analysis report is presented in Appendix B.

	ustification	Compliance	Signal J	Signal Justified?		
5	usuncation	compliance	YES	NO		
Minimum Vehicular	A Total Volume	90 %		~		
Volume	B Crossing Volume	14 %				
. Delay to Cross	A Main Road	90 %		~		
Traffic	B Crossing Road	16 %				
. Combination	A Justificaton 1	14 %	-	V		
	B Justification 2	16 %				
.4-Hr Volume		18 %				
	3	1 3 1	1	1		
. Collision Expe	erience	0 %		•		
	ξ	8 8	1	3		
. Pedestrians	A Volume	Justification not met		V		
	B Delay	Justification not met		V		

Figure 5: Signal Warrant Analysis Results for Essa Road and GO Station Access Intersection

## 3.5 EXISTING INTERSECTION CAPACITY ANALYSIS

The intersection capacity analysis was conducted using *Synchro 9* software package using the level of service criteria defined under Highway Capacity Manual (HCM).

Capacity is defined as the maximum number of vehicles that can pass over a road segment or through a intersection within set time duration. Capacity is combined with Level of Service (LOS) to describe the operating characteristics of a road segment or intersection. LOS is a qualitative measure that describes operational conditions and motorist perceptions within a traffic stream. The HCM defines six levels of service, LOS 'A' through LOS 'F', with 'A' representing the shortest average delays and 'F' representing the longest average delays. LOS 'D' is the typically accepted standard in urbanized areas. In addition, any movement with volume to capacity (v/c) ratio over 0.85 was considered as critical movement with traffic operational concern, as per the City's Traffic Impact Study Guidelines.

For unsignalized intersections, only the movements that must yield right-of-way experience the delay. Accordingly, minor street approach delays are reported herein for unsignalized conditions. It is typical for 'stop' sign controlled side streets and driveways intersecting major streets to experience longer delays during peak hours, while the majority of the traffic moving through the intersection on the major street experiences little or no delay.

Table 1 presents the intersection delay criteria for signalized and 'stop-sign' controlled intersections.

Level of	Intersection Delay Cri	iteria (seconds per vehicle)				
Service	Signalized	'Stop-Sign' Controlled/Roundabout	- Traffic Operation			
А	≤ <b>1</b> 0	≤ 10				
В	$> 10.0$ and $\le 20.0$	> 10.0 and $\leq$ 15.0	Acceptable operation			
С	> 20.0 and ≤ 35.0	> 15.0 and $\le$ 25.0				
D	$> 35.0$ and $\leq 55.0$	> 25.0 and ≤ 35.0				
E	> 55.0 and ≤ 80.0	> 35.0 and ≤ 50.0	Marginally Acceptable – occasional queuing			
F	> 80.0	> 50.0	Unacceptable – persistent queuing			

#### Table 1: Intersection Level of Service Criteria

The existing traffic operating performances for the signalized intersections were assessed based on average vehicular delay and resulting LOS using *Synchro* report. The traffic operational performance for the unsignalized intersection (at Essa Road and GO Station South Access) was assessed based on HCM 2000 report, as the *Synchro* report for the unsignalized intersection was showing unrealistic delays.

The intersection capacity analysis results for existing (2018) peak hour conditions are presented in Table 2. Detailed *Synchro* reports are presented in Appendix C.

#### Table 2: Existing (2018) Intersection Capacity Analysis Results

Morning Peak Hour

Intersection	Ove Interse		Weekday AM Peak					
	Delay	LOS	Movement	Delay (s)	LOS	V/C	Queue <sup>1</sup> (m)	
			EBL	7	А	0.14	14	
			EBTR	8	А	0.21	32	
			WBL	16	В	0.00	1	
Lakeshore Dr. &			WBT	14	В	0.18	27	
Tiffin St.	34	С	WBR	5	А	0.61	22	
(Signalized)			NBL	32	С	0.15	7	
			NBTR	30	С	0.01	2	
			SBL	119	F	1.15	135	
			SBTR	6	А	0.24	10	
			EBL	9	A	0.12	10	
		В	EBTR	10	А	0.34	34	
			WBL	11	В	0.21	15	
Essa Rd. & Tiffin St. (Signalized)	14		WBT	9	А	0.23	25	
(			NBLTR	19	В	0.67	34	
			SBLT	17	В	0.41	18	
			SBR	6	А	0.09	4	
			EBT	4	А	0.39	94	
			EBR	4	А	0	1	
Lakeshore Dr. & GO Station East Access	4	А	WBL	1	А	0	1	
(Signalized)	4	~	WBT	4	А	0.55	109	
			NBL	27	С	0.04	1	
			NBR	18	В	0.02	1	
Essa Rd. & Gowan			WBLR	14	В	0.62	16	
St.	6	А	NBTR	3	А	0.14	12	
(Signalized)			SBLT	3	А	0.21	15	
Essa Rd & GO Station South			EBLR	12	В	0.05	1	
Access	1	А	NBL	1	A	0.01	1	
(Unsignalized)			SBL	1	А	0.02	1	

#### Afternoon Peak Hour

Intersection	Overall Intersection		Weekday PM Peak					
Intersection	Delay	LOS	Movement	Delay (s)	LOS	V/C	Queue <sup>1</sup> (m)	
			EBL	8	А	0.20	19	
			EBTR	11	В	0.38	65	
			WBL	16	В	0.00	1	
Lakeshore Dr. &			WBT	15	В	0.17	24	
Tiffin St.	116	F	WBR	4	А	0.54	20	
(Signalized)			NBL	33	С	0.20	9	
			NBTR	29	С	0.02	2	
			SBL	371	F	1.75	219	
			SBTR	8	А	0.30	14	
			EBL	13	В	0.09	9	
		В	EBTR	16	В	0.53	65	
			WBL	18	В	0.39	26	
Essa Rd. & Tiffin St. (Signalized)	18		WBT	13	В	0.24	29	
(elgilalized)			NBLTR	18	В	0.69	48	
			SBLT	22	С	0.73	43	
			SBR	4	А	0.11	5	
			EBT	7	A	0.61	242	
			EBR	4	А	0	1	
Lakeshore Dr. & GO Station East Access	5	A	WBL	1	А	0.01	1	
(Signalized)	5	~	WBT	2	А	0.44	68	
			NBL	28	С	0.01	2	
			NBR	19	В	0.02	2	
Essa Rd. & Gowan			WBLR	15	В	0.74	12	
St. 7	7	А	NBTR	3	А	0.18	19	
(Signalized)			SBLT	5	А	0.40	35	
Essa Rd & GO			EBLR	11	В	0.06	1	
Station South Access	1	А	NBL	1	А	0.02	1	
(Unsignalized)			SBL	1	А	0.03	1	

#### Weekend Peak Hour

Intersection	Overall Intersection		Weekend Peak					
Intersection	Delay	LOS	Movement	Delay (s)	LOS	V/C	Queue <sup>1</sup> (m)	
			EBL	7	А	0.21	21	
			EBTR	8	А	0.24	37	
			WBL	16	В	0.01	2	
Lakeshore Dr. &			WBT	14	В	0.12	17	
Tiffin St.	63	Е	WBR	4	А	0.49	18	
(Signalized)			NBL	32	С	0.14	10	
			NBTR	30	С	0.04	4	
			SBL	205	F	1.36	158	
			SBTR	7	А	0.35	14	
			EBL	9	A	0.05	5	
		В	EBTR	11	В	0.34	36	
			WBL	13	В	0.36	25	
Essa Rd. & Tiffin St. (Signalized)	15		WBT	10	А	0.18	19	
(0.9.000000)			NBLTR	19	В	0.70	41	
			SBLT	16	В	0.37	19	
			SBR	5	А	0.08	5	
			EBT	5	А	0.46	123	
			EBR	4	А	0	1	
Lakeshore Dr. & GO Station East Access	4	A	WBL	2	А	0	1	
(Signalized)	4	~	WBT	2	А	0.38	54	
			NBL	27	С	0.03	3	
			NBR	16	В	0.04	2	
Essa Rd. & Gowan			WBLR	14	В	0.67	17	
St.	6	А	NBTR	3	А	0.17	16	
(Signalized)			SBLT	4	А	0.25	19	
Essa Rd & GO Station South			EBLR	12	В	0.09	2	
Access	1	А	NBL	1	А	0.03	1	
(Unsignalized)			SBL	1	А	0.02	1	

The intersection capacity analysis results indicate that all the study area intersections and individual movements are operating at LOS 'C' or better for three peak hour conditions, except for the southbound left movement at the Lakeshore Drive and Tiffin Street intersection. This movement is operating with an unacceptable level of delay (at LOS 'F') during all the three peak hours with volume to capacity (v/c) ratio over 1.0. The higher delay for this movement is mainly caused by heavy traffic demands (400, 620, and 460 vehicles per hour during morning, afternoon, and weekend peak hour, respectively). This results in overall intersection LOS at 'E/F' for Lakeshore Drive and Tiffin Street intersection during afternoon and weekend peak hours.

# 4 FUTURE (2024) BACKGROUND TRAFFIC CONDITIONS

### 4.1 FUTURE (2024) BACKGROUND TRAFFIC VOLUMES

To assess the future background traffic conditions, the future background traffic volumes for the expected opening year (2024) were estimated based on the background traffic growth and additional pick-up and drop-off trips generated from the growth of GO Transit ridership.

#### 4.1.1 BACKGROUND TRAFFIC GROWTH

The future (2024) background traffic volumes were estimated using a growth rate of 2% per annum, as suggested by the City based on the review of future travel demand. The projected future (2024) background traffic volumes for morning, afternoon, and weekend peak hour conditions are presented in Figure 6. It is noted that the existing bus volumes in the study area were excluded from the background traffic growth projection; the existing bus volumes were added back to the total background traffic volume estimate, after growth rate was applied to the existing non-bus volumes.

#### 4.1.2 PICK-UP AND DROP-OFF TRIPS

Trips are expected to be generated by the proposed growth of GO Transit ridership and enhanced GO Transit service; additional pick-up and drop-off vehicle trips were included for the future background traffic estimate. A total of 25 vehicle trips and 7 vehicle trips were estimated for the weekday peak hours and for the weekend peak hour respectively. Details of the pick-up and drop-off trip estimates are presented in Appendix D.

The pick-up and drop-off vehicle trips were distributed to study area roadways based on the observed travel patterns of existing traffic. The future (2024) pick-up and drop-off vehicle trips are presented in Figure 7.

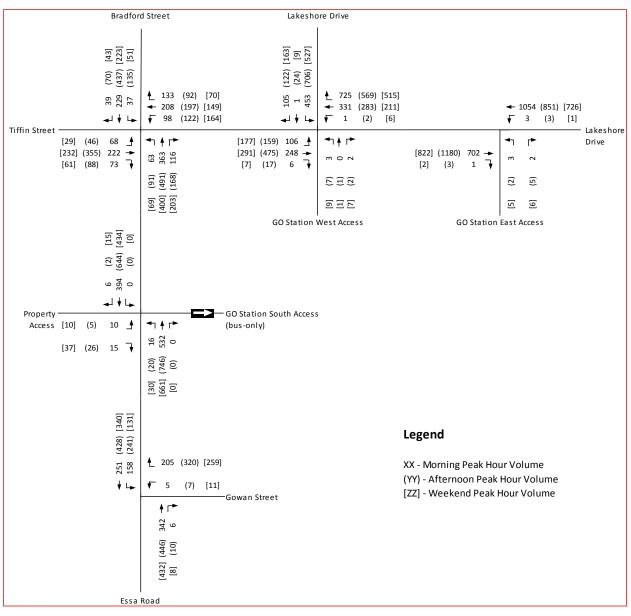


Figure 6: Future (2024) Background Traffic Growth

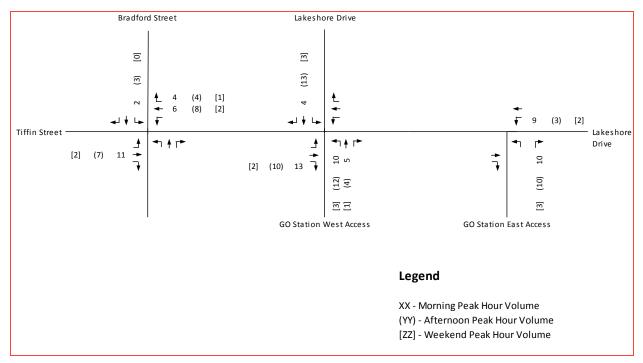


Figure 7: Future (2024) Pick-Up and Drop-Off Vehicle Trips

#### 4.1.3 TOTAL BACKGROUND TRAFFIC VOLUMES

The total background traffic volumes include the existing bus volumes, future background traffic growth for the non-bus volumes, and additional pick-up and drop-off trips generated from the growth of GO Transit ridership, as presented in Figure 8.

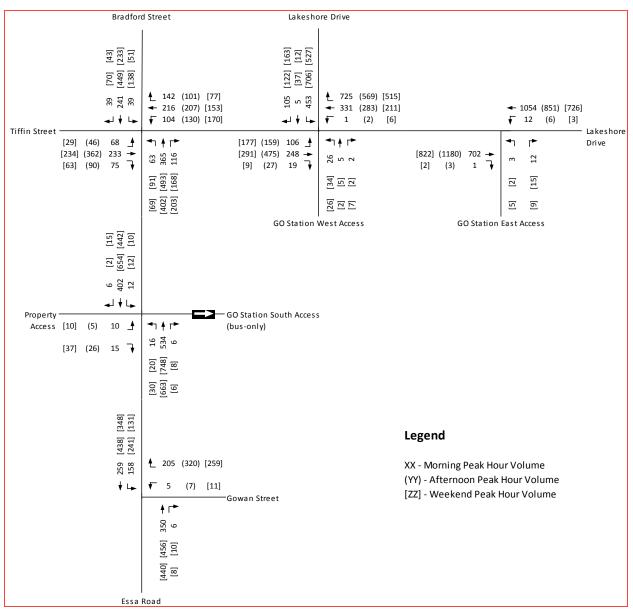


Figure 8: Future (2024) Total Background Traffic Volumes

# 4.2 FUTURE (2024) INTERSECTION CAPACITY ANALYSIS-BACKGROUND TRAFFIC CONDITION

Intersection capacity analysis for the future background traffic condition was conducted using *Synchro 9* software. Existing *Synchro* models were used for the analysis; existing signal timing plans were optimized to accommodate the future traffic demands. The evaluation elements for intersection capacity analysis include average delays, LOS, v/c ratio, and queue length. The analysis results for future (2024) background traffic condition are presented in Table 3, for all the three peak hour conditions.

Detailed Synchro reports are presented in Appendix E.

#### Table 3: Future (2024) Intersection Capacity Analysis Results – Background Traffic Condition

#### Morning Peak Hour

Intersection	Overall Intersection		Weekday AM Peak					
Intersection	Delay	LOS	Movement	Delay (s)	LOS	V/C	Queue¹ (m)	
			EBL	19	В	0.26	24	
			EBTR	22	С	0.36	59	
			WBL	25	С	0.00	1	
Lakeshore Dr. &			WBT	28	С	0.32	40	
Tiffin St.	18	В	WBR	7	А	0.74	32	
(Signalized)			NBL	27	С	0.11	10	
			NBTR	26	С	0.02	4	
			SBL	27	С	0.74	101	
			SBTR	4	А	0.15	10	
		в	EBL	10	A	0.13	13	
			EBTR	11	В	0.36	46	
	18		WBL	11	В	0.24	20	
Essa Rd. & Tiffin St. (Signalized)			WBT	11	В	0.24	33	
(0.9.10200)			NBLTR	26	С	0.74	49	
			SBLT	22	С	0.49	26	
			SBR	6	А	0.10	5	
			EBT	7	А	0.50	114	
			EBR	4	А	0	1	
Lakeshore Dr. & GO Station East Access	8	A	WBL	2	А	0.02	1	
Station East Access (Signalized)	0	~	WBT	8	А	0.69	147	
			NBL	31	С	0.04	2	
			NBR	13	В	0.16	0	

Intersection	Overall Intersection		Weekday AM Peak					
	Delay	LOS	Movement	Delay (s)	LOS	V/C	Queue <sup>1</sup> (m)	
Essa Rd. & Gowan			WBLR	16	В	0.67	18	
St.	6	А	NBTR	3	А	0.16	14	
(Signalized)			SBLT	3	А	0.24	18	
Essa Rd & GO			EBLR	12	В	0.06	1	
Station South Access	1	А	NBL	1	А	0.02	1	
(Unsignalized)			SBL	1	А	0.01	1	

Note: 1. Queue length reflects the 95<sup>th</sup> percentile queue length

#### Afternoon Peak Hour

Intersection	Overall Intersection		Weekday PM Peak					
	Delay	LOS	Movement	Delay (s)	LOS	V/C	Queue <sup>1</sup> (m)	
			EBL	21	С	0.35	34	
			EBTR	36	D	0.66	117	
			WBL	25	С	0.01	2	
Lakeshore Dr. &			WBT	28	С	0.28	35	
Tiffin St.	37	D	WBR	7	А	0.67	27	
(Signalized)			NBL	31	С	0.25	13	
			NBTR	26	С	0.02	4	
			SBL	78	Е	1.06	237	
			SBTR	5	А	0.20	15	
			EBL	17	В	0.10	13	
			EBTR	22	С	0.59	105	
			WBL	27	С	0.51	43	
Essa Rd. & Tiffin St. (Signalized)	24	С	WBT	17	В	0.27	46	
			NBLTR	24	С	0.78	72	
			SBLT	29	С	0.81	64	
			SBR	4	А	0.12	6	
Lakeshore Dr. & GO			EBT	11	В	0.73	288	
Station East Access	8	А	EBR	4	А	0	1	
(Signalized)			WBL	2	А	0.02	1	

Intersection	Overall Intersection		Weekday PM Peak					
intersection	Delay	LOS	Movement	Delay (s)	LOS	V/C	Queue¹ (m)	
			WBT	4	А	0.52	86	
			NBL	32	С	0.01	2	
			NBR	15	В	0.09	4	
Essa Rd. & Gowan			WBLR	16	В	0.78	11	
St.	8	А	NBTR	3	А	0.20	24	
(Signalized)			SBLT	6	А	0.46	46	
Essa Rd & GO			EBLR	11	В	0.07	1	
Station South Access	1	А	NBL	1	А	0.02	1	
(Unsignalized)			SBL	1	А	0.02	1	

Note: 1. Queue length reflects the 95<sup>th</sup> percentile queue length

#### Weekend Peak Hour

	Overall Intersection		Weekend Peak					
Intersection	Delay	LOS	Movement	Delay (s)	LOS	V/C	Queue¹ (m)	
			EBL	21	С	0.37	39	
			EBTR	24	С	0.41	67	
			WBL	25	С	0.02	3	
Lakeshore Dr. &			WBT	27	С	0.20	26	
Tiffin St.	20	В	WBR	6	А	0.62	24	
(Signalized)			NBL	28	С	0.13	11	
			NBTR	26	С	0.03	5	
			SBL	33	С	0.83	133	
			SBTR	4	А	0.22	13	
			EBL	11	В	0.05	7	
			EBTR	12	В	0.35	50	
			WBL	15	В	0.39	35	
Essa Rd. & Tiffin St. (Signalized)	19	В	WBT	11	В	0.19	26	
(Olynaii200)			NBLTR	25	С	0.77	61	
			SBLT	21	С	0.44	28	
			SBR	6	А	0.09	6	

Intersection	Overall Intersection		Weekend Peak					
Intersection	Delay	LOS	Movement	Delay (s)	LOS	V/C	Queue¹ (m)	
			EBT	7	А	0.55	154	
			EBR	5	А	0	1	
Lakeshore Dr. & GO Station East Access	6	А	WBL	2	А	0.01	1	
(Signalized)	0		WBT	3	А	0.45	64	
			NBL	32	С	0.04	3	
			NBR	17	В	0.07	2	
Essa Rd. & Gowan			WBLR	17	В	0.73	20	
St.	7	А	NBTR	3	А	0.19	20	
(Signalized)			SBLT	4	А	0.29	25	
Essa Rd & GO			EBLR	12	В	0.10	2	
Station South Access	1	A	NBL	1	А	0.03	1	
(Unsignalized)			SBL	1	А	0.01	1	

*Note: 1. Queue length reflects the* 95<sup>th</sup> *percentile queue length* 

The intersection capacity analysis results indicate that all the study area intersections and individual movements are expected to operate at LOS 'D' or better for all the three peak hour conditions, except for the southbound left movement at Lakeshore Drive and Tiffin Street intersection. This movement is operating at LOS 'E' during the afternoon peak hour with volume to capacity (v/c) ratio over 1.0. Signal timing plan was optimized to provide additional green time for this movement. The high delay for the southbound left movement is mainly caused by heavy traffic demands (over 700 vehicles per hour during the afternoon peak hour) allocated to single left turn lane.

# 5 FUTURE (2024) TOTAL TRAFFIC CONDITIONS

## 5.1 FUTURE (2024) TOTAL TRAFFIC VOLUMES

To assess the future total traffic conditions, the total traffic volumes for the expected opening year (2024) were estimated using the total background traffic volumes and assigned future bus trips.

#### 5.1.1 ADJUSTMENT OF TOTAL BACKGROUND TRAFFIC VOLUMES

According to the proposed plan for the new mobility hub, the following network modifications will be implemented to existing station accesses:

- The existing bus-only access (GO Station South Access) at Essa Road will be permanently closed.
- The existing GO Station West Access at Lakeshore Drive will be converted into a bus-only access and used by all the future bus routes
- The existing general-traffic using GO Station West Access will need to divert to GO Station East Access.

Therefore, future total background traffic volumes (excluding existing bus volumes) were further adjusted to reflect these modified bus station accesses, as presented in Figure 9.

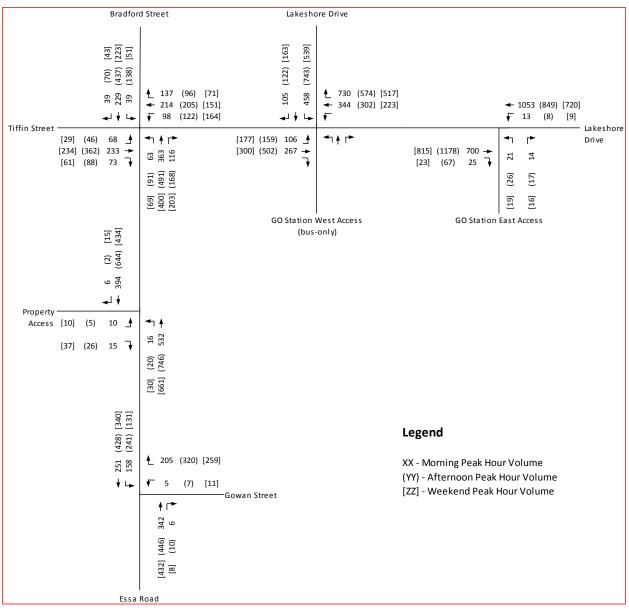
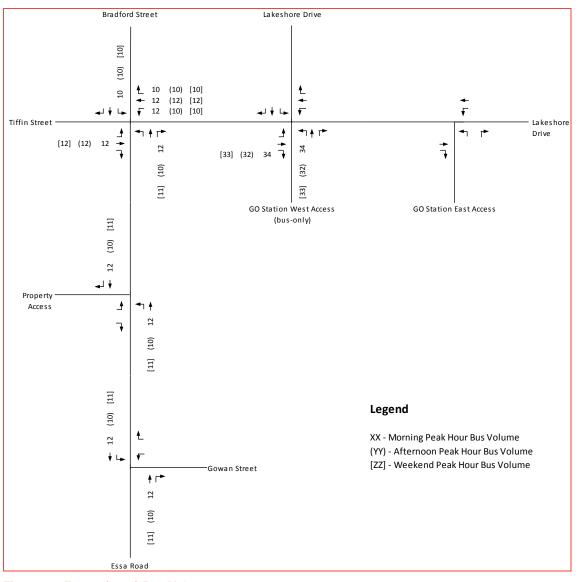


Figure 9: Future (2024) Adjusted Total Background Traffic Volumes

#### 5.1.2 FUTURE BUS VOLUMES

Future bus volumes for the opening year (2024) condition were taken from Scenario 1 of the transit servicing work of this study. This option was developed in consultation with Barrie Transit to simulate a reasonable future transit network in which all routes were altered to serve the hub. This scenario was chosen because it created the higher of the volumes of the two potential scenarios. Scenario 2 of the future bus network was designed to balance routes to serve the new hub while maintaining direct access to downtown.

All the Barrie transit routes serving the future mobility hub were assumed to operate with a 30-min headway in both directions during peak hours. Some other bus operations (Greyhound and Ontario Northland) were assumed to access the hub from Highway 400 via Essa Road and operate with 1-2 buses during the three peak hours. The total



bus trips are presented in Figure 10, for the opening year (2024) conditions. Detailed bus trip estimates were presented in Appendix D.

#### Figure 10: Future (2024) Bus Volumes

#### 5.1.3 TOTAL TRAFFIC VOLUMES

The 'Total' traffic volumes for study area intersections include future adjusted total background traffic volumes and future bus volumes, as presented in Figure 11. It is noted that by 2024, no bus service is expected on Lakeshore Drive within the study area. Therefore, no traffic volume was shown for the westbound left, northbound through and right, and southbound through movements at Lakeshore Drive and Tiffin Street intersection.

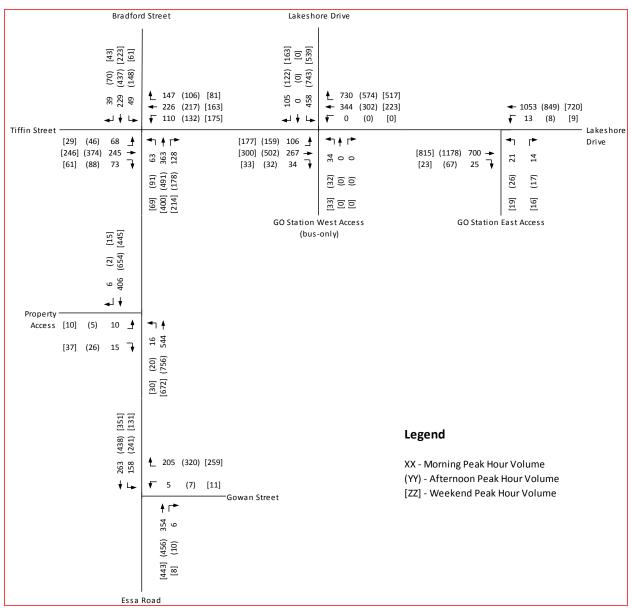


Figure 11: Future (2024) Total Traffic Volumes

## 5.2 FUTURE INTERSECTION LANE CONFIGURATIONS

For the future (2024) intersection capacity analysis, the following two intersection lane configuration scenarios were considered for the northbound approach at the Lakeshore Drive/Tiffin Street and Station West Access intersection, as presented in Figure 12:

- Scenario 1 with single lane for northbound left movement at GO Station West Access, and
- Scenario 2 with dual lanes for northbound left movement at GO Station West Access.

Scenario 1 with Single Left Turn Lane for Northbound Approach



Scenario 2 with Dual Left Turn Lanes for Northbound Approach



Figure 12: Future Opening Year (2024) Intersection Lane Configurations

Under both the scenarios, dedicated bus-only lane for eastbound right and westbound left movements are proposed at the Lakeshore Drive/Tiffin Street and Station West Access intersection. As presented in Figure 12, Scenario 1 assumes a single northbound left turn lane and a shared through/right lane for the northbound approach at GO Station West Access, and Scenario 2 assumes one dedicated left lane, and one shared left/through/right lane instead.

In this study, future total traffic analysis was conducted for Scenario 1 and presented in the next section; traffic analysis conducted for Scenario 2 was considered as a test scenario and documented in the appendix.

# 5.3 FUTURE (2024) INTERSECTION CAPACITY ANALYSIS-TOTAL TRAFFIC CONDITION

Intersection capacity analysis for the future opening year (2024) was conducted using *Synchro 9* software. Existing *Synchro* models were modified to reflect future intersection lane configurations; existing signal timing plans were optimized to accommodate future traffic demands. To improve bus movements and minimize the queue length for westbound movements at Essa Road/Bradford Street and Tiffin Street intersection, the following signal improvement measures were considered in the analysis:

- A protected plus permissive phase was proposed for the northbound left movement at Lakeshore Drive and Tiffin Street intersection (transit signal priority exists in the existing condition);
- A protected plus permissive phase was proposed for the westbound left movement at Essa Road/Bradford Street and Tiffin Street intersection (signal head with green arrow light exists in the existing condition); and,
- Actuated-coordinated phase setting was applied to Tiffin Street intersections at the Essa Road/Bradford Street and Lakeshore Drive intersection.

The evaluation elements for intersection capacity analysis include average delays, LOS, v/c ratio, and queue length. The analysis results for future (2024) total traffic condition (with lane configurations for Scenario 1) are presented in Table 4. Detailed *Synchro* reports are presented in Appendix F. The intersection capacity analysis for Scenario 2 (as a test scenario) was documented in Appendix G.

#### Table 4: Future (2024) Intersection Capacity Analysis Results – Total Traffic Condition

Morning Peak Hour

Intersection	Overall Intersection		Weekday AM Peak					
Intersection	Delay	LOS	Movement	Delay (s)	LOS	V/C	Queue¹ (m)	
			EBL	31	С	0.34	34	
Lakeshore Dr. & Tiffin St.			EBT	36	D	0.36	85	
			EBR	2	А	0.09	0	
	21	С	WBT	36	D	0.62	92	
(Signalized)	21	C	WBR	8	А	0.74	33	
			NBL	17	В	0.20	9	
			SBL	27	С	0.74	103	
			SBT	1	А	0.13	0	
			EBL	24	С	0.19	22	
		с	EBTR	29	С	0.56	84	
			WBL	4	А	0.20	6	
Essa Rd. & Tiffin St. (Signalized)	30		WBT	3	А	0.22	12	
(0.9.1			NBLTR	43	D	0.85	71	
			SBLT	37	D	0.63	38	
			SBR	3	А	0.10	2	
			EBT	8	А	0.53	118	
			EBR	3	А	0.02	4	
Lakeshore Dr. & GO Station East Access	11	В	WBL	3	А	0.03	2	
(Signalized)		D	WBT	11	В	0.74	157	
			NBL	35	С	0.29	7	
			NBR	12	В	0.18	0	
Essa Rd. & Gowan			WBLR	15	В	0.67	18	
St.	6	А	NBTR	3	А	0.16	15	
(Signalized)			SBLT	3	А	0.24	19	
Essa Rd & Property Access	1	А	EBLR	11	В	0.05	1	
(Unsignalized)	I	~	NBL	0	А	0.02	0	

#### Afternoon Peak Hour

Intersection	Ove Interse		Weekday PM Peak								
Intersection	Delay LOS		Movement	Delay (s)	LOS	V/C	Queue¹ (m)				
			EBL	28	С	0.45	29				
			EBT	68	Е	0.68	122				
			EBR	1	А	0.08	0				
Lakeshore Dr. &	50	-	WBT	34	С	0.54	80				
(Signalized)	DelayLOSMovementDelay (s)LOSV/Cshore Dr. & St. nalized)59EBL28C0.45EBT68E0.68WBT34C0.54WBR6A0.66NBL177B0.19SBL118F1.17SBTR1A0.15Rd. & Tiffin St. halized)45DEBT59E45DWBL43D0.69WBL43D0.690.820.82SBLT11B0.310.950.82SBLT43D0.950.930.95SBOR4A0.110.050.69Shore Dr. & GO on East Access16BWBL3A0.03MUDT7A0.050.690.690.69	27									
		0.19	8								
Tiffin St. (Signalized)       59       E       WBR       6       A         NBL       17       B         SBL       118       F         SBTR       1       A         Essa Rd. & Tiffin St. (Signalized)       45       D       EBL       27       C         WBL       43       D       WBL       11       B         NBLTR       36       D       SBLT       43       D	F	1.17	273								
			SBTR	1	А	0.15	0				
			EBL	27	С	0.15	17				
		D	EBTR	59	E	0.93	165				
			WBL	43	D	0.69	44				
	45		WBT	11	В	0.31	21				
			NBLTR	36	D	0.82	94				
			SBLT	43	D	0.95	75				
			SBR	4	А	0.11	6				
			EBT	22	С	0.89	328				
			EBR	4	А	0.06	8				
Lakeshore Dr. & GO	16	в	WBL	3	А	0.03	1				
(Signalized)	10	D	WBT	7	А	0.59	98				
			NBL	36	D	0.35	8				
			NBR	12	В	0.21	0				
Essa Rd. & Gowan			WBLR	16	В	0.77	19				
St.	7	А	NBTR	3	A	0.20	23				
(Signalized)			SBLT	6	A	0.41	42				
Essa Rd & Property Access	1	A	EBLR	11	В	0.06	1				
(Unsignalized)	I	А	NBL	1	А	0.02	1				

#### Weekend Peak Hour

Interception	Ove Interse		Weekend Peak								
Intersection	Delay	LOS	Movement	Delay (s)	LOS	V/C	Queue <sup>1</sup> (m)				
			EBL	32	С	0.42	53				
			EBT	36	D	0.41	93				
			EBR	1	А	0.09	0				
Lakeshore Dr. & Tiffin St.	24	C	WBT	31	С	0.40	59				
(Signalized)	24	C	WBR	Delay (s)         LOS         V/C         Query (m)           32         C         0.42         53           36         D         0.41         93           1         A         0.09         0           31         C         0.40         59           6         A         0.62         25           17         B         0.21         9           36         D         0.86         150           1         A         0.19         0           36         D         0.86         150           1         A         0.19         0           26         C         0.09         12           35         C         0.62         88           7         A         0.32         15           40         D         0.86         81           33         C         0.60         36           34         C         0.62         16           4         A         0.02         1           6         A         0.51         67           34         C         0.26         7           12 <td< td=""><td>25</td></td<>	25						
			NBL		9						
	Intersection         Delay         LOS         Movement         Delay (s)         LOS         V/C         C           shore Dr. & St. tailized)         24         EBL         32         C         0.42         0.41           EBT         36         D         0.41         0.09         0.41         0.09           St. tailized)         24         EBR         1         A         0.09         0.41           WBT         31         C         0.40         0.41         0.41         0.09         0.41           WBR         6         A         0.62         0.40         0.86         0.86         0.86           SBL         36         D         0.86         0.86         0.86         0.86         0.86           SBL         36         D         0.86	150									
			SBTR	1	А	0.19	0				
			EBL	26	С	0.09	12				
Essa Rd. & Tiffin St. (Signalized)		с	EBTR	35	С	0.62	88				
			WBL	7	А	0.32	15				
	30		WBT	6	А	0.17	15				
			NBLTR	40	D	0.86	81				
			SBLT	33	С	0.60	36				
			SBR	3	А	V/C         (m)           0.42         53           0.41         93           0.09         0           0.40         59           0.40         59           0.62         25           0.21         9           0.86         150           0.19         0           0.09         12           0.62         88           0.32         15           0.17         15           0.86         81           0.60         36           0.10         3           0.62         156           0.17         15           0.86         81           0.60         36           0.10         3           0.62         156           0.02         4           0.02         1           0.51         67           0.26         7           0.21         0           0.73         20           0.19         21	3				
		EBT	10	A	0.62	156					
			EBR	4	А	0.02	4				
Lakeshore Dr. & GO	٩	Δ	WBL	2	А	0.02	1				
(Signalized)	5	~	WBT	6	А	0.51	67				
			NBL	34	LOS         V/C         Quer (m)           C         0.42         53           D         0.41         93           A         0.09         0           C         0.40         59           A         0.09         0           C         0.40         59           A         0.62         25           B         0.21         9           D         0.86         15           A         0.19         0           C         0.09         12           C         0.62         88           A         0.17         15           D         0.86         81           C         0.60         36           A         0.17         15           D         0.86         81           C         0.60         36      A         0.101         3           A         0.62         15           A         0.62         15           A         0.62         7           B         0.21         0           B         0.73         20           A         0.19	7					
			NBR	12	В	0.21	0				
Essa Rd. & Gowan			WBLR	17	В	0.73	20				
St.	7	A	NBTR	3	A	0.19	21				
(Signalized)			SBLT	4	A	0.26	24				
Essa Rd & Property Access	1	А	EBLR	12	В	0.09	2				
(Unsignalized)	I	A	NBL	1	А	0.03	1				

With the optimized signal timing plans, all the intersection capacity analysis results indicate that all the study area intersections and individual movements are expected to operate at LOS 'D' or better for morning and weekend peak hour conditions. However, during the afternoon peak hour a few movements are expected to operate at LOS 'E/F' at Lakeshore Drive and Tiffin Street intersection. The southbound left movement at Lakeshore Drive and Tiffin Street intersection is expected to operate at LOS 'F' with v/c ratio over 1.0. The high delay for this movement is resulting due to projected heavy traffic demand (over 700 vehicles during afternoon peak hour) operating with a single left turn lane. As the dual left turn lanes are not being supported by the City staff for this southbound left turn movement, as the City intends to encourage vehicular traffic to use Bradford Street instead of Lakeshore Drive due to pedestrian safety issues at the crosswalks on Lakeshore Drive.

During afternoon peak hour, the eastbound through traffic on Tiffin Street at the Lakeshore Drive is expected to operate at LOS 'E' with 95<sup>th</sup> percentile queue length of 120 m, which could potentially back up to the upstream intersection at Essa Road/Bradford Street. The higher delay and longer queues for this movement are resulting due to less green time allocated to the east-west movements with the optimized signal timing plan to accommodate the higher traffic demand for the southbound left turn movement.

Similarly, the eastbound through traffic on Tiffin Street at Essa Road/Bradford Street is expected to operate at LOS 'E' during afternoon peak hour, due to the increased future traffic demands and advanced green phase (the protected phase) provided for the westbound left turn movement. To provide an acceptable level of service in afternoon peak hour, additional vehicular capacity may be required on Tiffin Street or Lakeshore Drive (for the southbound left movement).

# 5.4 REVIEW OF FUTURE BACKGROUND AND TOTAL TRAFFIC ANALYSES

With the planned mobility hub and network modifications (implemented to existing station accesses), traffic operational performances are expected to deteriorate in the future total traffic conditions for the adjacent intersections at Lakeshore Drive and Tiffin Street, at Essa Road and Tiffin Street, and at Lakeshore Drive and GO Station East Access.

When signal timings plans were optimized to facilitate bus movements (as described in Section 5.3), the overall intersection LOS dropped from 'A/B' in the future background traffic condition to 'B/C' in the future total traffic condition during the morning peak hour, from 'A/C/D' to 'B/D/E' during the afternoon peak hour, and from 'A/B' to 'A/C' during the weekend peak hour (Table 5). Especially for the intersection at Lakeshore Drive and Tiffin Street, the eastbound through and southbound left movements are expected to experience higher delays and longer queues during the afternoon peak hour. These impacts are mainly resulting from higher traffic demands and optimized signal timing settings to improve bus movements egressing from the station (the northbound left movement at Lakeshore Drive and Tiffin Street intersection).

A sensitivity scenario was conducted for the future total traffic condition, using optimized signal timing plans to reduce overall intersection delays and improve traffic operation. As presented in Table 5, when signal timing plans were optimized for general traffic operation at Lakeshore Drive and Tiffin Street intersection and at Essa Road and Tiffin Street intersection,

- both the intersections are expected to operate at an overall LOS 'D' or better during the three peak hours;
- the overall intersection delays are expected to reduce by 2 to 12 seconds during the three peak hours (as compared to the future total traffic condition with optimized signal timing plans for bus movements).

The complete intersection capacity analysis results and *Synchro* reports for this sensitivity scenario are presented in Appendix H.

#### **Table 5: Overall Intersection LOS Review**

		Intersection								
Intersection LOS (Delay) for	Condition	Lakeshore Drive and Tiffin Street	Essa Road and Tiffin Street	Lakeshore Drive and GO Station East Access						
	Background Traffic	B (18 s)	B (18 s)	A (8 s)						
AM Peak Hour	Total Traffic (optimized for bus)	C (21 s)	C (30 s)	B (11 s)						
	Total Traffic (optimized for traffic)	B (19 s)	C (22 s)	B (11 s)						
	Background Traffic	D (37 s)	C (24 s)	A (8 s)						
PM Peak Hour	Total Traffic (optimized for bus)	E (59 s)	D (45 s)	B (16 s)						
	Total Traffic (optimized for traffic)	D (47 s)	D (36 s)	B (16 s)						
	Background Traffic	B (20 s)	B (19 s)	A (6 s)						
Weekend Peak Hour	Total Traffic (optimized for bus)	C (24 s)	C (30 s)	A (9 s)						
	Total Traffic (optimized for traffic)	C (21 s)	C (22 s)	A (9 s)						

# 6 STUDY FINDINGS

The following key notes summarize the study findings:

- Five intersections near existing Allandale GO Station were analyzed in this study. Other than Essa Road and GO Station South Access intersection, all the study area intersections are signalized. The signal warrant analysis result confirms that traffic signals is not warranted for this intersection for the existing traffic demand.
- Existing intersection capacity analysis results indicate that all the study area intersections and individual movements are operating at LOS 'C' or better during morning, afternoon, and weekend peak hour, except for the southbound left turn movement at the Lakeshore Drive and Tiffin Street intersection. This movement is currently operating at LOS 'F' during all the three peak hours with v/c ratio over 1.0.
- Future total background traffic volumes were estimated for the opening year (2024) to incorporate existing bus volumes, background traffic growth (2% per annum) for non-bus volumes, and additional pick-up and drop-off trips generated from the growth of GO Transit ridership.
- The intersection capacity analysis results for future total background traffic conditions indicated that all the study area intersections and individual movements are operating at LOS 'D' or better for all the three peak hour conditions, except for the southbound left movement at the Lakeshore Drive and Tiffin Street intersection (at LOS 'E' during the afternoon peak hour).
- Future total traffic volumes were estimated for the opening year (2024) to incorporate future total background traffic volumes and future assigned bus volumes:
  - Future total background traffic volumes (excluding existing bus volumes) were adjusted to reflect network modifications implemented to bus station accesses (Figure 9).
  - Future bus volumes were estimated considering the future bus routes serving the new mobility hub. All the bus trips will be using GO Station West Access (Figure 10).
- The key findings of the future (2024) analysis results for the total traffic condition are noted below:
  - All the study area intersections and individual movements are expected to operate at LOS 'D' or better for morning and weekend peak hour conditions.
  - However, during the afternoon peak hour a few movements are expected to operate at LOS 'E/F' at Lakeshore Drive and Tiffin Street intersection.
- The review of the future background and total traffic analyses indicated that,
  - with the planned mobility hub and network modifications (implemented to existing station accesses), traffic operational performances for the adjacent intersections are expected to deteriorate in the future total traffic conditions, as compared to future background traffic conditions (presented in Table 5);
  - when signal timings plans were optimized to facilitate bus movements in future total traffic condition, the overall intersection LOS dropped from 'A/B' in future background condition to 'B/C' in future total traffic condition during the morning peak hour, from 'A/C/D' to 'B/D/E' during the afternoon peak hour, and from 'A/B' to 'A/C' during the weekend peak hour;
  - the traffic impacts are mainly resulting from higher traffic demands and optimized signal timing settings to improve bus movements egressing from the station;
  - when signal timings plans were optimized to reduce overall intersection delays and improve traffic operation, both the intersections on Tiffin Street are expected to operate at an overall LOS 'D' or better during three the peak hours; the overall intersection delays are expected to reduce by 2 to 12 seconds during three the peak hours, as compared to future total traffic condition with optimized signal timing plans for bus movements.

# 7 RECOMMENDATIONS

For the future (2024) total traffic conditions, the following signal improvement measures are recommended to improve bus operations and reduce delays for bus movements:

- A protected plus permissive phase was proposed for the northbound left movement at Lakeshore Drive and Tiffin Street intersection (transit signal priority exists in the existing condition);
- A protected plus permissive phase was proposed for the westbound left movement at Essa Road/Bradford Street and Tiffin Street intersection (signal head with green arrow light exists in the existing condition); and,
- Actuated-coordinated phase setting was applied to Tiffin Street intersections at the Essa Road/Bradford Street and Lakeshore Drive intersection.

Additionally, Transit Signal Priority (TSP) could be considered to further reduce the transit delays in the study area. Dedicated bus-only turn lanes are recommended for eastbound right and westbound left movements at Lakeshore Drive and Tiffin Street. Additional through lane in the east-west direction could be considered beyond 2024 to provide additional vehicular capacity on Tiffin Street.

The primary purpose of the traffic analysis for this study was to assess the various alternatives developed for the Allandale Transit Station. The traffic analysis has been conducted using Synchro based model, which estimates vehicular delays using empirical formulas considering traffic volumes, intersection lane configuration and signal timing plans, etc. It is of note that delay estimated using Synchro model may be not precise as estimated using a micro-simulation model (such as Vissim and Aimsun). The micro-simulation model takes into account potential impacts of skipped signal phases, signal gap-out, transit call, etc. Considering the limitations of Synchro model, we recommend City to conduct further study using a micro-simulation model to assess impacts of transit priority measures within the study area.

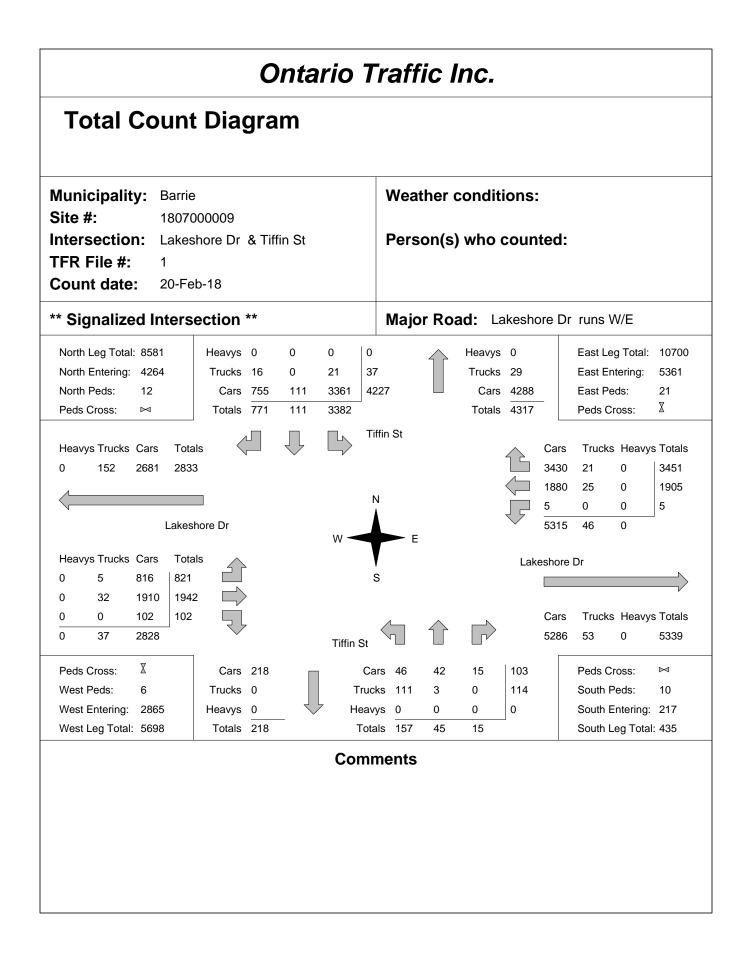
# **APPENDIX**

# A Existing Traffic Count Reports

Morning Peak Diagram	Specified Period           From:         6:00:00           To:         9:00:00		ne Hour Peak om: 8:00:00 o: 9:00:00						
Municipality:BarrieSite #:1807000009Intersection:Lakeshore Dr & Tiffin StTFR File #:1Count date:20-Feb-18	Weather conditions: Person(s) who counted:								
** Signalized Intersection **	Major Road: Lak	eshore D	r runs W/E						
North Entering: 485 Trucks 3 0 6	Heavys Trucks 76 Totals	5 733	East Leg Total:1548East Entering:939East Peds:4Peds Cross:∑						
Heavys Trucks Cars Totals	īffin St	Cars 640	Trucks Heavys Totals						
Lakeshore Dr	N	$\begin{array}{c} 292 \\ \hline \\ 933 \end{array}$	2 0 294 0 0 1 6 0						
W	E	Lakeshore	Dr						
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	S								
0 0 5 5 0 1 314 Tiffin St		Cars 603	Trucks Heavys Totals 6 0 609						
		5 13	Peds Cross: ⋈ South Peds: 1						
	$\begin{array}{cccc} vys & 0 & 0 & 0 \\ tals & 16 & 0 & 2 \end{array}$	0	South Entering: 18 South Leg Total: 25						
	ments								

Mid-day Peak Diag	ram	Specifi From: To:	_				One Hour Peak           From:         13:00:00           To:         14:00:00			
Municipality:BarrieSite #:1807000009Intersection:Lakeshore Dr & TitTFR File #:1Count date:20-Feb-18	fin St	Weather conditions: Person(s) who counted:								
** Signalized Intersection **		Major I	Road: La	akesho	re Dr	runs W	//E			
North Leg Total:1152Heavys0North Entering:593Trucks3North Peds:2Cars108Peds Cross:Image: Construction of the second	0 0 0 0 5 8 6 471 5 6 476	1	Heavys Trucks Cars Totals	7 552	_	East Leg East Ent East Peo Peds Cre	ering: ds:	1334 639 2 ∑		
Heavys Trucks Cars Totals 0 21 315 336		iffin St			Cars 426 203	Trucks 5 5	0	5 Totals   431   208		
Lakeshore Dr	10/	N F		<b>F</b>	203 0 629	0 10	0 0 0	0		
Heavys Trucks Cars Totals	vv			Lake	eshore	Dr				
0 1 126 127 0 7 209 216		S						$ \rightarrow $		
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	Tiffin St				Cars 683	Trucks 12	Heavys 0	s Totals 695		
Peds Cross: X Cars 9 West Peds: 0 Trucks 0		ars 4 ( cks 13 1	) 3 1 0	7 14		Peds Cro South Pe		⊠ 0		
West Entering:     346     Heavys     0       West Leg Total:     682     Totals     9	Hear	/ys_0	0 0	0		South Er South Le	ntering:	21		
		ments				Ooun Ee	y iotai	. 50		

Afternoon Peak Diagram	Specified Period         One Hour Peak           From:         15:00:00         From:         16:30:00           To:         18:00:00         To:         17:30:00							
Municipality:BarrieSite #:1807000009Intersection:Lakeshore Dr & Tiffin StTFR File #:1Count date:20-Feb-18	Weather conditions: Person(s) who counted:							
** Signalized Intersection **	Major Road: Lakeshore Dr runs W/E							
North Leg Total:         1402         Heavys         0         0         0         0           North Entering:         755         Trucks         1         0         1         2           North Peds:         7         Cars         107         21         625         7           Peds Cross:         Image: March 108         21         626         7	Heavys 0 Trucks 2 Cars 645 Totals 647 Heavys 0 East Leg Total: 1807 East Entering: 758 East Peds: 3 Peds Cross: X							
Heavys Trucks Cars Totals	ffin St Cars Trucks Heavys Totals 504 1 0 505 251 0 0 251							
Lakeshore Dr	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$							
W	Lakeshore Dr							
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	5							
0 0 15 15 0 3 574 Tiffin St	Cars Trucks Heavys Totals 1046 3 0 1049							
	rs 6 1 2 9 Peds Cross: ⊠ ks 15 0 0 15 South Peds: 3							
	ys         0         0         0         South Entering:         24           als         21         1         2         South Leg Total:         62							
	nents							



				-		o <i>Trafi</i> ount S	-	-				
Intersection:	akesho	re Dr &				vate: 20-Feb-18		cipality: Ba	rrie			
			ach Tot			2010010				ach Tot	als	
	Include	es Cars, T	rucks, & H	eavys		eavys						
Hour Ending	Left	Thru	Right	Grand Total	Total Peds	North/South Total Approaches	Hour Ending	Left	Thru	Right	Grand Total	Total Peds
6:00:00	0	0	0	0	0	0	6:00:00	0	0	0	0	0
7:00:00	158	36	46	240	0	276	7:00:00	20	15	1	36	3
8:00:00 9:00:00	319 391	21 1	76 93	416 485	0 2	443 503	8:00:00 9:00:00	18 16	8 0	1 2	27 18	0 1
12:00:00	0	0 0	93 0	400	0	0	12:00:00	0	0	0	0	0
13:00:00	396	11	125	532	0		13:00:00	18	1	4	23	0
14:00:00	476	6	111	593	2		14:00:00	17	1	3	21	0
15:00:00	0 515	0 4	0 114	0 633	0 1		15:00:00 16:00:00	0 18	0 1	0 2	0 21	0 1
17:00:00	601	4	119	727	4	747	17:00:00	20	0	20	20	4
18:00:00	526	25	87	638	3		18:00:00	30	19	2	51	1
Totals:	3382 East	111 Approa	771 ach Tota	4264 als	12	4481		157 Wes	45 t <b>Appro</b>	15 ach Tota	217 als	10
			rucks, & H			East/West				rucks, & H	eavys	
Hour Ending	Left	Thru	Right	Grand Total	Total Peds	Total Approaches	Hour Ending	Left	Thru	Right	Grand Total	Total Peds
6:00:00	0	0	0	0	0	0	6:00:00	0	0	0	0	0
7:00:00	0	177	168	345	1	466	7:00:00	17	67	37	121	1
8:00:00	0	273	423	696	0	947	8:00:00	52	178	21	251	2
9:00:00 12:00:00	1 0	294 0	644 0	939 0	4	1254 0	9:00:00 12:00:00	94 0	216 0	5 0	315 0	0 0
13:00:00	Ő	203	379	582	Ő	-	13:00:00	126	218	3	347	Ő
14:00:00	0	208	431	639	2		14:00:00	127	216	3	346	0
15:00:00	0	0	0	0	0		15:00:00	0	0	0	0	0
16:00:00 17:00:00	0 1	246 273	418 498	664 772	2 5		16:00:00 17:00:00	143 125	315 393	3 3	461 521	1
18:00:00	3	231	490	724	7		18:00:00	137	339	27	503	1
Totals:	5	1905	3451	5361	21	8226		821	1942	102	2865	6
	diaat	7.00				or Traffic Cr	-	-		10.00		
Hours En Crossing		7:00 216	8:00 360	9:00 412	13:00 425		14:00 501	16:00 540	17:00 634	18:00 589		

# **Ontario Traffic Inc.**

Count Date: 20-Feb-18 Site #: 1807000009

Teb-10 Sile #. 1007000009

		Passeng	ger Cars -	North Ap	oproach		Trucks - North Approach						Heavys - North Approach						Pedestrians		
Interval	Lef	it	Thi	ru	Rig	Jht	Le	ft	Th	ru	Rig	ht	Le	ft	Th	ru	Rig	ight Nor		Cross	
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	
6:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	(	
6:15:00	22	22	9	9	9	9	0	0	0	0	0	0	0	0	0	0	0	0	0	(	
6:30:00	53	31	14	5	19	10	0	0	0	0	0	0	0	0	0	0	0	0	0	(	
6:45:00	106	53	33	19	29	10	1	1	0	0	0	0	0	0	0	0	0	0	0	(	
7:00:00	156	50	36	3	44	15	2	1	0	0	2	2		0	0	0	0	0	0	(	
7:15:00	229	73	51	15	56	12	2	0	0	0	2	0	-	0		0	0	0	0		
7:30:00	295	66	51	0	68	12	2	0	0	0	2	0	0	0	0	0	0	0	0	(	
7:45:00	380	85	51	0	95	27	2	0	0	0	3	1	0	0	0	0	0	0	0	(	
8:00:00	473	93	57	6	119	24	4	2	0	0	3	0	0	0	0	0	0	0	0	(	
8:15:00	585	112	57	0	139	20	5	1	0	0	3	0	-	0	0	0	0	0	1		
8:30:00	685	100	58	1	161	22	6	1	0	0	4	1	0	0	0	0	0	0	1	(	
8:45:00	774	89	58	0	183	22	8	2	0	0	4	0	-	0	0	0	0	0	1	(	
9:00:00	858	84	58	0	209	26	10	2	0	0	6	2		0	0	0	0	0	2		
9:01:55	858	0	58	0	209	0	10	0	0	0	6	0		0	0	0	0	0	2	(	
12:00:00	858	0	58	0	209	0	10	0	0	0	6	0	-	0	0	0	0	0	2	(	
12:15:00	977	119	60	2	232	23	10	0	0	0	6	0		0	0	0	0	0	2		
12:30:00	1089	112	63	3	273	41	10	0	0	0	6	0	0	0	0	0	0	0	2		
12:45:00	1179	90	65	2	303	30	10	0	0	0	6	0	0	0	0	0	0	0	2	(	
13:00:00	1254	75	69	4	334	31	10	0	0	0	6	0	-	0	0	0	0	0	2		
13:15:00	1379	125	69	0	364	30	11	1	0	0	6	0	0	0	0	0	0	0	3		
13:30:00	1507	128	71	2	399	35	14	3	0	0	6	0	0	0	0	0	0	0	3	(	
13:45:00	1613	106	73	2	419	20	14	0	0	0	7	1	0	0	0	0	0	0	3	(	
14:00:00	1725	112	75	2	442	23	15	1	0	0	9	2	0	0	0	0	0	0	4		
14:02:12	1725	0	75	0	442	0	15	0	0	0	9	0		0	0	0	0	0	4	(	
15:00:00	1725	0	75	0	442	0	15	0	0	0	9	0	-	0	0	0	0	0	4	(	
15:15:00	1859	134	76	1	469	27	16	1	0	0	9	0	-	0	0	0	0	0	4	(	
15:30:00	1965	106	76	0	495	26	16	0	0	0	10	1	0	0	0	0	0	0	5		
15:45:00	2089	124	77	1	524	29	17	1	0	0	10	0	-	0	0	0	0	0	5	(	
16:00:00	2235	146	79	2	554	30	20	3	0	0	11	1	0	0	0	0	0	0	5	(	
16:15:00	2375	140	79	0	583	29	20	0	0	0	13	2	0	0	0	0	0	0	5	(	
16:30:00	2525	150	80	1	614	31	20	0	0	0	13	0	-	0	0	0	0	0	5	(	
16:45:00	2680	155	80	0	645	31	20	0	0	0	14	1	0	0	0	0	0	0	5	(	
17:00:00	2836	156	86	6	670	25	20	0	0	0	14	0		0	0	0	0	0	9		
17:15:00	2989	153	89	3	697	27	21	1	0	0	14	0	-	0	0	0	0	0	11		
17:30:00	3150	161	101	12	721	24	21	0	0	0	14	0	-	0	0	0	0	0	12		
17:45:00	3279	129	107	6	736	15	21	0	0	0	16	2		0	-	0	0	0	12		
18:00:00	3361	82	111	4	755	19	21	0	0	0	16	0	0	0	0	0	0	0	12		
18:15:00	3361	0	111	0	755	0	21	0	0	0	16	0	0	0	0	0	0	0	12	(	
18:17:27	3361	0	111	0	755	0	21	0	0	0	16	0	0	0	0	0	0	0	12	(	

# Ontario Traffic Inc.

Count Date: 20-Feb-18 Site #: 1807000009

Passenger Cars - East Approach **Trucks - East Approach** Heavys - East Approach Pedestrians Left Interval Left Thru Right Thru Right Left Thru Right East Cross Time Cum Incr 6:00:00 6:15:00 6:30:00 6:45:00 7:00:00 7:15:00 7:30:00 7:45:00 8:00:00 8:15:00 8:30:00 8:45:00 9:00:00 9:01:55 12:00:00 12:15:00 12:30:00 12:45:00 13:00:00 13:15:00 13:30:00 13:45:00 14:00:00 14:02:12 15:00:00 15:15:00 15:30:00 15:45:00 16:00:00 16:15:00 16:30:00 16:45:00 17:00:00 17:15:00 17:30:00 17:45:00 18:00:00 18:15:00 18:17:27 

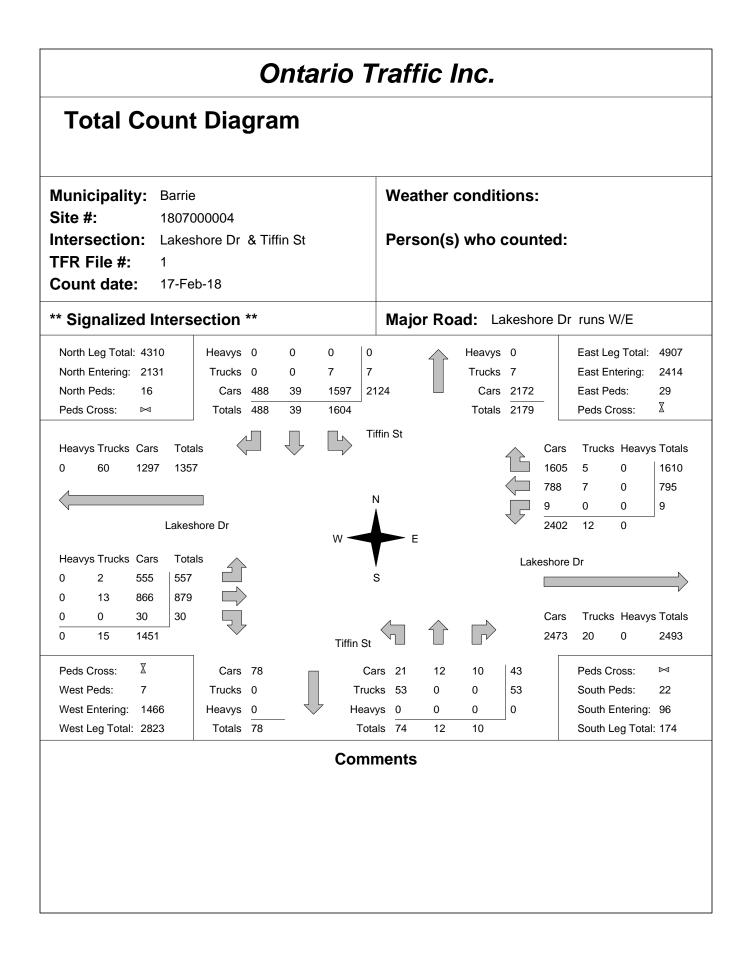
		Passeng	er Cars -	South Ap	proach			Tru	cks - Sou	th Appro	bach			Hea	avys - Sou	th Appro	ach		Pedes	strians
Interval	Le	ft	Thi	ru	Rig	ht	Le	ft	Th	ru	Rig	ht	Le	ft	Th	ru	Rig	lht	South	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Inc
6:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
6:15:00	1	1	2	2	0	0	4	4	0	0	0	0	0	0	0	0	0	0	1	
6:30:00	4	3	4	2	0	0	6	2	0	0	0	0	0	0	0	0	0	0	2	
6:45:00	6	2	12	8	0	0	9	3	0	0	0	0	0	0	0	0	0	0	3	
7:00:00	8	2	15	3	1	1	12	3	0	0	0	0	0	0	0	0	0	0	3	
7:15:00	12	4	18	3	2	1	19	7	0	0	0	0	0	0	0	0	0	0	3	
7:30:00	12	0	22	4	2	0	20	1	1	1	0	0	0	0	0	0	0	0	3	
7:45:00	12	0	22	0	2	0	23	3	1	0	0	0	0	0	0	0	0	0	3	
8:00:00	12	0	22	0	2	0	26	3	1	0	0	0	0	0	0	0	0	0	3	
8:15:00	12	0	22	0	2	0	30	4	1	0	0	0	0	0	0	0	0	0	3	
8:30:00	14	2	22	0	3	1	33	3	1	0	0	0	0	0	0	0	0	0	3	
8:45:00	15	1	22	0	4	1	38	5	1	0	0	0	0	0	0	0	0	0	4	
9:00:00	15	0	22	0	4	0	39	1	1	0	0	0	0	0	0	0	0	0	4	
9:01:55	15	0	22	0	4	0	39	0	1	0	0	0	0	0	0	0	0	0	4	
12:00:00	15	0	22	0	4	0	39	0	1	0	0	0	-	0		0	0	0	4	
12:15:00	15	0	22	0	5	1	43	4	1	0	0	0	0	0	0	0	0	0	4	
12:30:00	15	0	22	0	5	0	45	2	1	0	-	0	-	0		0	0	0	4	
12:45:00	16	1	22	0	5	0	50	5	1	0	0	0	0	0		0	0	0	4	
13:00:00	18	2	23	1	8	3	54	4	1	0	0	0	0	0	0	0	0	0	4	
13:15:00	18	0	23	0	8	0	59	5	1	0	0	0	-	0	-	0	0	0	4	
13:30:00	19	1	23	0	10	2	60	1	1	0	0	0	-	0	-	0	0	0	4	
13:45:00	21	2	23	0	11	1	64	4	1	0	0	0	-	0	-	0	0	0	4	
14:00:00	22	1	23	0	11	0	67	3	2	1	0	0	-	0	-	0	0	0	4	
14:02:12	22	0	23	0	11	0	67	0	2	0	0	0		0	-	0	0	0	4	
15:00:00	22	0	23	0	11	0	67	0	2	0	0	0		0	-	0	0	0	4	
15:15:00	24	2	23	0	11	0	70	3	3	1	0	0	-	0	-	0	0	0	4	
15:30:00	25	1	23	0	11	0	71	1	3	0	0	0	-	0	-	0	0	0	4	
15:45:00	25	0	23	0	12	1	77	6	3	0	0	0	-	0		0	0	0	5	
16:00:00	26	1	23	0	13	1	81	4	3	0		0	-	0		0	0	0	5	
16:15:00	20	1	23	0	13	0	86		3	0	0	0	-	0	-	0	0	0	5	
16:30:00	28	1	23	0	13	0	87	1	3	0	0	0	-	0	-	0	0	0	7	
16:45:00	28	0	23	0	13	0	93	6	3	0	0	0		0	-	0	0	0	7	
17:00:00	31	3	23	0	13	0	93	3	3	0	-	0		0	-	0	0	0	9	
17:15:00	33	2	23	1	15	2	100	1	3	0	0	0	-	0		0	0	0	10	
17:30:00	34	2	24	0	15	2	100	4	3	0	-	0	-	0	-	0	0	0	10	
17:45:00	41	7	38	14	15	0	102	2	3	0	-	0	-	0	-	0	0	0	10	
	41	5	42	4	15	0		2	3	0	_	0	-	0	-	0	0	0	10	
18:00:00		-				-	111			v	-	-	-		-	•		_		
18:15:00	46	0	42	0	15	0	111	0	3		0	0		0		0	0	0	10	
18:17:27	46	0	42	0	15	0	111	0	3	0	0	0	0	0	0	0	0	0	10	

Count Date: 20-Feb-18 Site #: 1807000009

1eb-10 Sile #. 1007000003

		Passen	ger Cars -	West Ap	proach			Tru	cks - Wes	t Approa	ach			Hea	avys - We	st Appro	ach		Pedes	strians
Interval	Let	ft	Thr	u	Rig	lht	Le	ft	Thr	u	Rig	ht	Le	ft	Th	ru	Rig	lht	West	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
6:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
6:15:00	0	0	13	13	8	8	0	0	0	0	0	0	0	0	0	0	0	0	0	(
6:30:00	5	5	28	15	14	6	0	0	0	0	0	0	0	0	0	0	0	0	0	(
6:45:00	11	6	48	20	28	14	0	0	0	0	0	0	0	0	0	0	0	0	1	
7:00:00	16	5	67	19	37	9	1	1	0	0	0	0	0	0	0	0	0	0	1	(
7:15:00	22	6		32	57	20	1	0	3	3	0	0	-	0	-	0	0	0	3	
7:30:00	35	13		41	58	1	1	0	6	3	0	0	-	0	0	0	0	0	3	
7:45:00	54	19		46	58	0	1	0	6	0	0	0	0	0	0	0	0	0	3	
8:00:00	68	14		51	58	0	1	0	8	2	0	0	0	0	0	0	0	0	3	(
8:15:00	94	26		55	58	0	1	0	8	0	0	0	-	0	0	0	0	0	3	
8:30:00	109	15		53	60	2	1	0	8	0	0	0	-	0	0	0	0	0	3	
8:45:00	134	25	398	53	62	2	2	1	8	0	0	0	0	0	0	0	0	0	3	(
9:00:00	161	27	453	55	63	1	2	0	8	0	0	0	-	0		0	0	0	3	
9:01:55	161	0		0	63	0	2	0	8	0	0	0		0	0	0	0	0	3	
12:00:00	161	0		0	63	0	2	0	8	0	0	0	-	0	0	0	0	0	3	
12:15:00	187	26		59	64	1	2	0	9	1	0	0	0	0	0	0	0	0	3	(
12:30:00	225	38	560	48	64	0	2	0	14	5	0	0	-	0	-	0	0	0	3	
12:45:00	257	32		56	64	0	2	0	16	2	0	0	0	0	0	0	0	0	3	
13:00:00	286	29		47	66	2	3	1	16	0	0	0		0	0	0	0	0	3	
13:15:00	312	26		52	66	0	3	0	18	2	0	0	-	0	0	0	0	0	3	
13:30:00	342	30		52	68	2	4	1	20	2	0	0	-	0	0	0	0	0	3	
13:45:00	378	36		51	69	1	4	0	21	1	0	0	-	0	0	0	0	0	3	
14:00:00	412	34		54	69	0	4	0	23	2	0	0	-	0	0	0	0	0	3	
14:02:12	412	0	-	0	69	0	4	0	23	0	0	0	-	0	0	0	0	0	3	
15:00:00	412	0	-	0	69	0	4	0	23	0	0	0		0		0	0	0	3	
15:15:00	438	26		89	70	1	4	0	23	0	0	0	-	0	0	0	0	0	3	(
15:30:00	477	39		68	71	1	4	0	25	2	0	0	-	0	0	0	0	0	4	
15:45:00	525	48		73	71	0	4	0	27	2	0	0	-	0	0	0	0	0	4	(
16:00:00	555	30		81	72	1	4	0	27	0	0	0		0	0	0	0	0	4	(
16:15:00	584	29		103	73	1	4	0	29	2	0	0	-	0	0	0	0	0	4	(
16:30:00	613	29		65	74	1	4	0	30	1	0	0	-	0	0	0	0	0	5	
16:45:00	641	28		115	75	1	4	0	31	1	0	0	-	0	0	0	0	0	5	(
17:00:00	679	38		106	75	0	5	1	31	0	0	0		0	0	0	0	0	5	(
17:15:00	710	31	1677	105	82	7	5	0	32	1	0	0		0	0	0	0	0	5	(
17:30:00	753	43		93	89	7	5	0	32	0	0	0	-	0	0	0	0	0	5	(
17:45:00	789	36		75	94	5	5	0	32	0	0	0		0	-	0	0	0	6	
18:00:00	816	27	1910	65	102	8	5	0	32	0	0	0	-	0	0	0	0	0	6	(
18:15:00	816	0		0	102	0	5	0	32	0	0	0		0	0	0	0	0	6	
18:17:27	816	0	1910	0	102	0	5	0	32	0	0	0	0	0	0	0	0	0	6	(

Mid-day Peak Diagram	Specified Period           From:         10:00:00           To:         14:00:00	One Hour PeakFrom:13:00:00To:14:00:00
Municipality:BarrieSite #:1807000004Intersection:Lakeshore Dr & Tiffin StTFR File #:1Count date:17-Feb-18	Weather conditions: Person(s) who coun	
** Signalized Intersection **	Major Road: Lakesho	ore Dr runs W/E
North Leg Total:         1236         Heavys         0         0         0           North Entering:         621         Trucks         0         0         2           North Peds:         2         Cars         145         8         466           Peds Cross:         Image: Marcine State S	0 Heavys 0 2 Trucks 2 619 Cars 613 Totals 615	East Leg Total: 1381 East Entering: 649 East Peds: 8 Peds Cross: X
Heavys Trucks Cars Totals	Tiffin St	Cars Trucks Heavys Totals 455 2 0 457
Lakeshore Dr		186     1     0     187       5     0     0     5       646     3     0
Heavys Trucks Cars Totals	Lake	eshore Dr
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	S	
	in St	Cars Trucks Heavys Totals 727 5 0 732
Peds Cross: X Cars 19	Cars 8 1 6 15	Peds Cross: 🛛 🖂
West Peds: 0 Trucks 0	Trucks 14 0 0 14	South Peds: 6
West Entering:421Heavys0West Leg Total:775Totals19	Heavys         0         0         0         0           Totals         22         1         6         6	South Entering: 29 South Leg Total: 48
-	omments	



				-		o <i>Trafi</i> ount S	-	-				
Intersection:	akesho	re Dr &				<sup>vate:</sup> 17-Feb-18		unicipality: Ba	arrie			
			ach Tot							oach Tot	als	
			rucks, & H			North/South				rucks, & H		
Hour Ending	Left	Thru	Right	Grand Total	Total Peds	Total Approaches	Hour Ending	Left	Thru	Right	Grand Total	Total Peds
10:00:00	0	0	0	0	0	0				0	0	0
11:00:00	335	15	111	461	5		11:00:0			1	27	8
12:00:00 13:00:00	384 417	10 6	107 125	501 548	4 5		12:00:0 13:00:0			2	19 21	2 6
14:00:00	468	8	145	621	2		14:00:0			6	29	6
Totals:			488 ach Tota rucks, & H	eavys	16	2227 East/West		74 Wes	t Appro	10 <b>ach Tot</b> rucks, & H	eavys	22
Hour Ending	Left	Thru	Right	Grand Total	Total Peds	Total Approaches	Hour Ending	Left	Thru	Right	Grand Total	Total Peds
10:00:00	0	0	0	0	0	0	10:00:0			0	0	0
11:00:00	Ő	180	362	542	10	-	11:00:0			12	308	2
12:00:00	3	227	380	610	4	931	12:00:0				321	3
13:00:00 14:00:00	1 5	201 187	411 457	613 649	7 8		13:00:0 14:00:0			7 6	416 421	2 0
Totals:	9	795	1610 Cala		29	3880		557		30	1466	7
Hours En		0:00	0:00	0:00	10:00	or Traffic Cr	11:0	0 12:00	13:00			
Crossing	values:	0	0	0	0		38	30 416	451	506		

		Passeng	er Cars -	North A	pproach			Tru	icks - Nor	th Appro	ach			Hea	vys - Nor	th Appro	ach		Pedes	trians
Interval	Le	ft	Thi	ru	Rig	ht	Le	ft	Th	ru	Rig	ht	Lei	ft	Th	ru	Rig	ht	North	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
10:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	C
10:15:00	75	75	5	5	32	32	1	1	0			0		0		0		0	1	1
10:30:00	149	74	6	1	59	27	1	0	0	0	0	0	0	0	0	0	0	0	3	2
10:45:00	239	90	7	1	85	26	1	0				0		0	0	0		0	4	1
11:00:00	334	95	15	8	111	26	1	0				0		0	0	0		0	5	1
11:15:00	413	79	22	7	136	25	2	1	0		-	0	-	0	0	0	-	0	5	C
11:30:00	517	104	23	1	159	23	2	0	-		-	0	-	0		0	-	0	8	3
11:45:00	612	95	24	1	182	23	3	1	0	-		0	-	0		0	-	0	8	C
12:00:00	716	104	25	1	218	36	3	0	-		-	0	-	0		0		0	9	1
12:15:00	818	102	25	0	248	30	3	0	-		-	0	-	0	0	0	-	0	10	1
12:30:00 12:45:00	915 1026	97 111	28 28	3		27 37	3	0			-	0	-	0	0	0		0	10 13	C 3
12:45:00	1131	105	31	3	312	37	5	2			-	0		0	0	0		0	13	3
13:15:00	1238	105	31	0		40	6	2	0			0	-	0	0	0	-	0	14	1
13:30:00	1361	107	34	3		40	6	0	-			0		0	0	0		0	15	C
13:45:00	1477	116	34	0		27	7	1	0		-	0		0		0	-	0	15	C
14:00:00	1597	120	39	5		36	7	0	-			0		0		0		0	16	1
14:15:00	1597	0	39	0	488	0	7	0	-		-	0	-	0	0	0	-	0	16	Ċ
14:17:14	1597	0	39	0		0	. 7	0	-			0	-	0		0		0	16	C

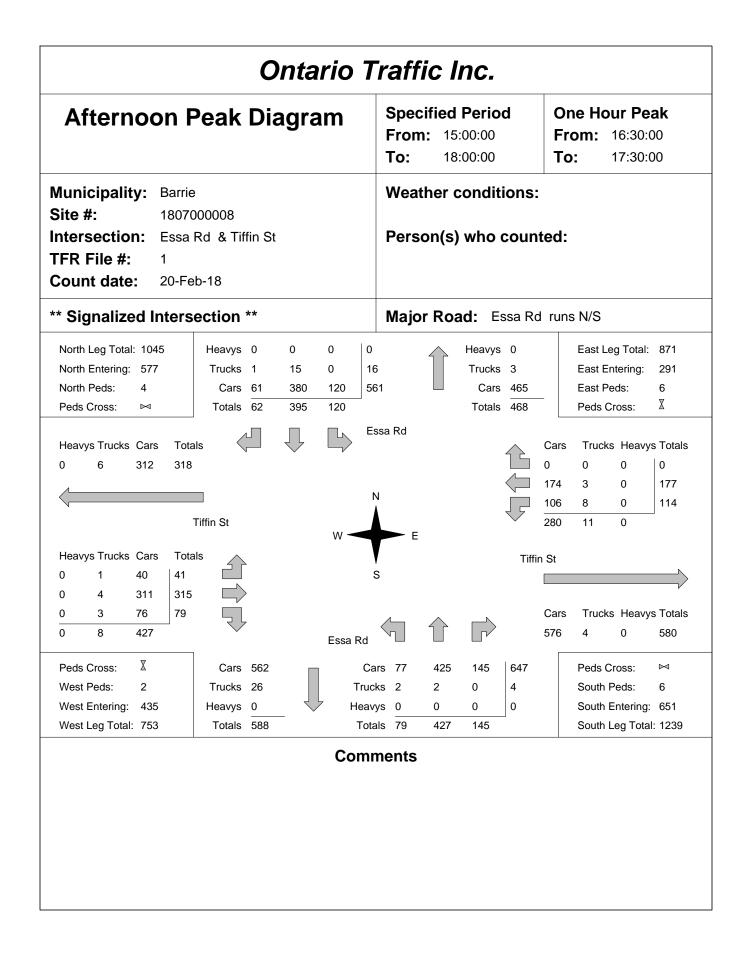
10:00:00 10:15:00	Lef Cum 0	it Incr	Thr	u	Rig	• •														
10:00:00 10:15:00	0	Incr			ing	ht	Le	ft	Th	ru	Rig	ht	Lef	it	Th	ru	Rig	ht	East C	Cross
10:15:00			Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	C
	0	0	45	45	79	79	0	0	0	0	0	0	0	0	0	0	0	0	2	2
10:30:00	0	0	92	47	166	87	0	0	-	0		1	0	0	-	0	0	0	4	2
10:45:00	0	0	138	46	256	90	0	0	-	0		1	0	0	_	0	0	0	5	1
11:00:00	0	0	180	42	360	104	0	0	-	0		0	-	0		0	0	0	10	5
11:15:00	2	2	231	51	440	80	0	0		1	2	0		0		0	0	0	10	0
11:30:00	2	0	283	52	554	114	0	0		1	2	0	-	0		0	0	0	10	0
11:45:00	3	1	342	59	645 740	91	0	0		1	2	0		0	-	0	0	0	11 14	1
12:00:00 12:15:00	3	0	404 461	62 57	835	95 95	0	0		0	2	0	-	0		0	0	0	14	3
12:15:00	4	0	401	38	956	95 121	0	0		0		1	0	0	-	0	0	0	14	2
12:45:00	4	0	499 547	48	1049	93	0	0		1	3	0	-	0	_	0	0	0	21	5
13:00:00	4	0	602	55	1150	101	0	0		1	3	0		0	-	0	0	0	21	0
13:15:00	5	1	641	39	1273	123	0	0		0	-	0	-	0	-	0	0	0	24	3
13:30:00	6	1	688	47	1372	99	0	0		0		0		0	-	0	0	0	26	2
13:45:00	7	1	742	54	1495	123	0	0		1	3	0	0	0	0	0	0	0	29	3
14:00:00	9	2	788	46	1605	110	0	0	7	0		2	0	0	0	0	0	0	29	C
14:15:00	9	0	788	0	1605	0	0	0	7	0	5	0		0	0	0	0	0	29	0
14:17:14	9	0	788	0	1605	0	0	0	7	0	5	0	0	0	0	0	0	0	29	C

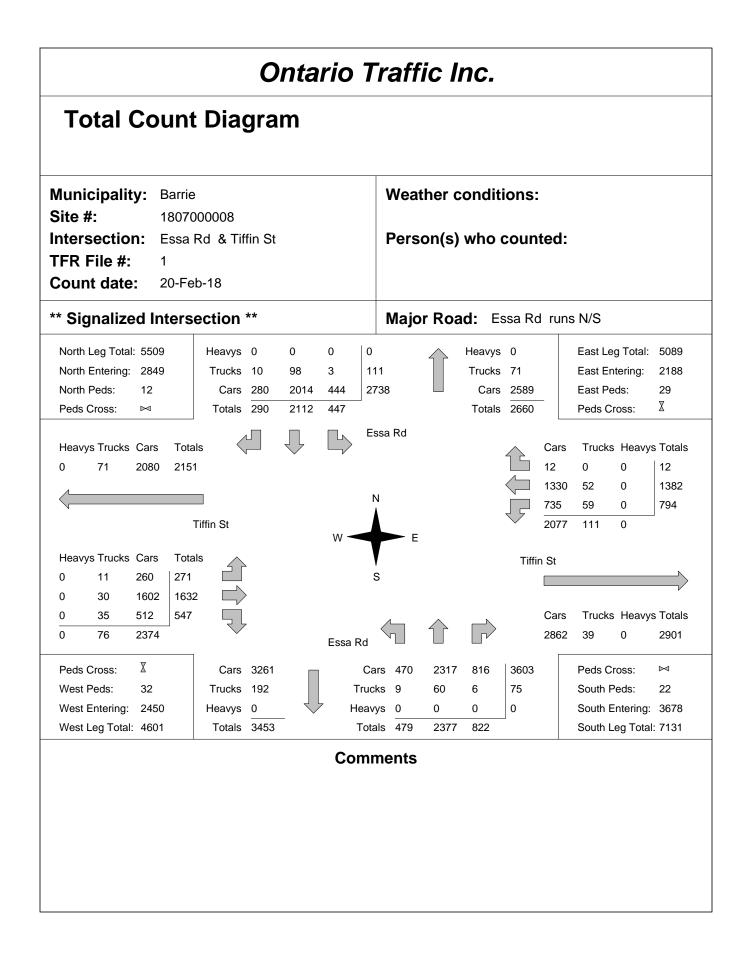
Journ	Date:		ger Cars -		nproach			True	cks - Sou	ith Appr	ach			Нор	ivys - Sou	ith Annro	ach		Pedes	triane
Interval	Le		- ger Cars Th		pproacn Rig		Le		cks - Sou Th			ght	Le		Th		Rig		South	
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
10:00:00	0	0	I			0		0						0				0	0	
10:00:00	0	0				0	4	4	0					0				0	2	
10:30:00	0	0			0	0	7	3				0		0				0	5	
10:45:00	1	1	8		-	1	11	4	0		_	0		0			-	0	6	
11:00:00	4	3				0	14	3	0	0		0		0		0		0	8	2
11:15:00	6	2			1	0	16	2	0	0	0	0		0		0		0	8	0
11:30:00	7	1	9		1	0	21	5	0	0	0	0	0	0	0	0	0	0	8	0
11:45:00	7	0	9	0	1	0	24	3	0	0	0	0	0	0	0	0	0	0	8	0
12:00:00	8	1	-	1	3	2	25	1	0	0	0	0	0	0	0	0	0	0	10	2
12:15:00	9	1	11	1	3	0	30	5	0	0	_	0	-	0	_	0	-	0	10	0
12:30:00	11	2		0	-	0	32	2	0		_	0	-	0		0	-	0	10	0
12:45:00	12	1	11	0		1	38	6	0		_	0	-	0		-		0	15	5
13:00:00	13	1		0	-	0	39	1	0	0	_	0	-	0	_	0	-	0	16	1
13:15:00	18	5	11	0		0	44	5		0		0	-	0		-		0	18	
13:30:00	19	1			7	3	47	3				0	-	0		-	-	0	20	
13:45:00	19	0			-	1	51	4	0		-	0	-	0		0		0	22	
14:00:00	21	2			-	2	53 53	2	0			0		0		0		0	22 22	
14:15:00 14:17:14	21 21	0				0	53	0		0	_	0	-	0			-	0	22	
14.17.14	21	0	12	0		U		0	0			0		0	0	0	0	0	22	0

Interval Time 10:00:00 10:15:00 10:30:00 10:45:00	Lef Cum 0	ft Incr	Thr	u	Rig	ht														
10:00:00 10:15:00 10:30:00		Incr			J	IIL	Let	ft	Thr	u	Rig	ht	Le	ft	Th	ru	Rig	ht	West	Cross
10:15:00 10:30:00	0		Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
10:30:00		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	C
	37	37	43	43	3	3	0	0	1	1	0	0	0	0	0	0	0	0	1	1
10.15.00	62	25	85	42	3	0	0	0	1	0	0	0	0	0	0	0	0	0	1	0
	94	32	127	42	4	1	1	1	2	1	0	0	0	0	0	0	0	0	1	0
11:00:00	120	26	172	45	12	8	2	1	-	0	0	0	0	0	0	0	0	0	2	
11:15:00	150	30	217	45	15	3	2	0	-	1	0	0	0	0	0	0	0	0	2	
11:30:00	179	29	265	48	15	0	2	0	-	3	0	0	0	0	0	0	0	0	5	
11:45:00	206	27	318	53	15	0	2	0		1	0	0	0	0	0	0	0	0	5	
12:00:00	238	32	364	46	17	2	2	0		1	0	0	0	0	0	0	0	0	5	
12:15:00	266	28	439	75	20	3	2	0		1	0	0	0	0	0	0	0	0	5	
12:30:00	309	43	500	61	20	0	2	0	-	0	0	0	0	0	0	0	0	0	6	
12:45:00	349	40	554	54	20	0	2	0		1	0	0	0	0	0	0	0	0	6	
13:00:00	398	49	611	57	24	4	2	0	-	0	0	0	0	0	0	0	0	0	7	
13:15:00	428	30	670	59	26 28	2	2	0		2	0	0	0	0	0	0	0	0	7	
13:30:00 13:45:00	465 511	37 46	728 786	58 58	28	2	2	0		0	0	0	0	0	0	0	0	0	7	
13:45:00	555	40	866	56 80	30	1	2	0		0	0	0	0	0 0	0	0	0	0	7	
14:15:00	555	44	866	0	30	0	2	0		0	0	0	0	0	0	0	0	0	7	
14:17:14	555	0	866	0	30	0	2	0	-	0	0	0	0	0	0	0	0	0	7	

Morning Peak Diagram	Specified Period           From:         6:00:00           To:         9:00:00	One Hour Peak         From:       8:00:00         To:       9:00:00
Municipality:BarrieSite #:1807000008Intersection:Essa Rd & Tiffin StTFR File #:1Count date:20-Feb-18	Weather conditions: Person(s) who counte	ed:
** Signalized Intersection **	Major Road: Essa Rd	runs N/S
North Leg Total:         661         Heavys         0         0         0         0           North Entering:         282         Trucks         1         16         0         17           North Peds:         2         Cars         34         198         33         26           Peds Cross:         Image: March 1         Totals         35         214         33		East Leg Total: 612 East Entering: 281 East Peds: 7 Peds Cross: X
Heavys Trucks Cars Totals		Cars Trucks Heavys Totals 1 0 0   1 182 5 0   187
Tiffin St W		34     9     0     93       267     14     0
Heavys Trucks Cars Totals 0 1 59 60 0 5 0 1 196 197 0 5	Tiffin	St
0 7 60 67 0 9 315 Essa Rd		Cars Trucks Heavys Totals 330 1 0 331
West Peds:     1     Trucks     32     Truc       West Entering:     324     Heavys     0     Heav	rs 54 309 101 464 (s 1 9 0 10 ys 0 0 0 ls 55 318 101	Peds Cross:Image: South Peds:1South Peds:474South Leg Total:848
Com	nents	

North Leg Total: 707 North Entering: 337 North Peds: 3 Peds Cross: $\bowtie$ Heavys 000014Heavys 0East Leg Total: 615 East Entering: 281 Cars 360 Totals 40East Peds: 55Peds Cross: $\bowtie$ $\bigtriangledown$ $\Box$ </th <th>Mid-day Pe</th> <th>eak Diagram</th> <th>Specifi From: To:</th> <th><b>ed Period</b> 12:00:00 14:00:00</th> <th>One Hour Peak           From:         12:15:00           To:         13:15:00</th>	Mid-day Pe	eak Diagram	Specifi From: To:	<b>ed Period</b> 12:00:00 14:00:00	One Hour Peak           From:         12:15:00           To:         13:15:00
North Leg Total: 707 North Entering: 337 North Peds: 3 Peds Cross:Heavys000141211416East Leg Total:615 East Entering:281 East Peds:5Peds Cross:Image: Solution of the second of the secon	Site #:180Intersection:EssTFR File #:1	7000008 a Rd & Tiffin St			
North Entering:       337       Trucks       1       12       1       14       Trucks       10       East Entering:       281         North Peds:       3       Peds Cross:       Image: state of the state of t	** Signalized Inter	section **	Major F	Road: Essa Ro	d runs N/S
Heavys Trucks Cars       Totals       Cars       Trucks       Heavys Trucks       Heavys Trucks       Cars       Trucks       Heavys Trucks         0       10       238       248       Image: Cars	North Entering: 337 North Peds: 3	Trucks 1 12 1 Cars <u>39 249 3</u>	14 5 323	Trucks 10	East Entering: 281 East Peds: 5
NTiffin StNTiffin St0235370618519103555801127526Cars 423Peds Cross: $\mathbb{X}$ Cars 423Trucks 21West Peds:4Cars 423Trucks 21Cars 50324105479Heavys00000South Peds:1South Peds:1South Peds:1South Peds:	-	N V	Essa Rd	Ê	1 0 0 1
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	< <u> </u>	Tiffin St		F	119 6 0 125
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	-			Tiffi	n St
0       11       275       Image: Carse of the second s			S		
West Peds:4Trucks21Trucks38213South Peds:1West Entering:286Heavys000000South Entering:492			issa Rd		,
West Entering:     286     Heavys     0     0     0     South Entering:     492	Peds Cross:	Cars 423	Cars 50 3	24 105 479	Peds Cross:
	-	·			-
Comments				-	





				-		o Traf	-		-				
Intersection:		о т: <i>tt</i> :г		Iran		ount S			<b>ary</b> <sup>ipality:</sup> Ba	rria			
				-1-		20-Feb-10							
			ach Tot rucks, & H			No with (O could					pach Tot rucks, & H		
Hour Ending	Left	Thru	Right	Grand Total	Total Peds	North/South Total Approaches	Hou Endin		Left	Thru	Right	Grand Total	Total Peds
6:00:00 7:00:00 8:00:00 9:00:00 12:00:00 13:00:00 14:00:00 15:00:00 16:00:00 17:00:00 18:00:00	0 23 30 33 0 36 54 0 69 111 91	0 161 220 214 0 277 249 0 286 356 349	0 8 30 35 0 40 36 0 41 58 42	0 192 280 282 0 353 339 0 396 525 482	0 1 0 2 0 3 2 0 0 3 1	819 0 964 1136	6:00 7:00 8:00 9:00 12:00 13:00 14:00 15:00 16:00 17:00 18:00	:00 :00 :00 :00 :00 :00 :00 :00	0 30 44 55 0 58 52 0 77 86 77	0 76 197 318 0 303 314 0 361 405 403	0 26 61 101 112 114 0 130 120 158	0 132 302 474 0 473 480 0 568 611 638	0 0 1 0 2 0 11 4 4
Totals:	447	2112	290	2849	12	6527			479	2377	822	3678	22
			ach Tota rucks, & H								ach Tota rucks, & H		
Hour				Grand	Total	East/West Total	_Hou	r				Grand	Total
Ending 6:00:00 7:00:00 8:00:00 12:00:00 13:00:00 14:00:00 15:00:00 16:00:00 17:00:00 18:00:00	Left 0 59 79 93 0 123 102 0 113 116 109	Thru 0 136 192 187 0 153 167 0 189 191 167	Right 0 1 1 0 1 2 0 4 1 1	Total 0 196 272 281 0 277 271 0 306 308 277	Peds 0 2 1 7 0 3 8 0 2 3 3 3	567 0 682 718	Endin 6:00 7:00 8:00 9:00 12:00 13:00 13:00 15:00 16:00 17:00 18:00	:00 :00 :00 :00 :00 :00 :00 :00 :00	Left 0 5 18 60 0 43 32 0 38 37 38	Thru 0 72 171 197 0 194 183 0 261 290 264	Right 0 53 62 67 0 50 81 0 77 83 74	Total 0 130 251 324 0 287 296 0 376 410 376	Peds 0 1 1 0 7 3 0 11 6 3
Totals: Hours En Crossing		1382 7:00 201	12 <b>Calc</b> 8:00 289	2188 ulated V 9:00 353	29 <b>/alues f</b> 13:00 363	4638 or Traffic Cr	14	<b>g Ma</b> :00 321	271 ajor Stre 16:00 423	1632 eet 17:00 450	18:00	2450	32

Count Date: 20-Feb-18 Site #: 1807000008

-1 eb-10 Site #. 1007000000

		Passen	ger Cars -	North Ap	proach			Tru	cks - Nor	th Appro	ach			Hea	vys - Nor	th Appro	ach		Pedes	trians
Interval	Lef	t	Thr	u	Rig	lht	Le	ft	Th	ru	Riç	ght	Le	eft	Th	ru	Ri	ght	North	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
6:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	C	0 0	0
6:15:00	5	5	24	24	1	1	0	0	2	2	0	0	0	0	0	0	0	C	0 0	0
6:30:00	9	4	69	45	4	3	0	0	4	2	0	0	0	0	0	0	0	C	0 0	0
6:45:00	19	10	114	45	4	0	0	0	6	2	0	0	0	0	0	0	0	C	) <u>1</u>	1
7:00:00	23	4	152	38	8	4	0	0	9	3	0	0	0	0	0	0	0	C	ו 1	0
7:15:00	34	11	194	42	11	3	1	1	11	2	0	0	-	0	0	0	0		1 1	0
7:30:00	36	2		40	19	8	1	0	16	5	0	0	0	0	0	0	0	C	) 1	0
7:45:00	43	7		55	23	4	1	0		1	1	1	0	0	0	0	0			0
8:00:00	52	9		71	36	13	1	0	21	4	2	1	0	0	0	0	0			0
8:15:00	58	6		67	48	12	1	0	24	3	3	1	0	0	0	0	0	C	) 2	1
8:30:00	66	8		45	55	7	1	0	29	5	3	0	-	0	0	0	0			0
8:45:00	77	11	512	40	64	9	1	0	31	2	3	0	0	0	0	0	0	C		0
9:00:00	85	8		46	70	6	1	0		6	3	0	0	0	0	0	0	C		1
9:00:46	85	0		0	70	0	1	0	37	0	3	0	-	0	0	0	0			0
12:00:00	85	0		0	70	0	1	0	37	0	3	0	-	0	0	0	0		-	0
12:15:00	99	14		69	78	8	1	0	40	3	4	1	0	0	0	0	0			0
12:30:00	103	4	690	63	92	14	2	1	44	4	4	0	-	0	0	0	0	-		0
12:45:00	111	8		68	101	9	2	0	46	2	4	0	-	0	0	0	0			2
13:00:00	120	9		64	108	7	2	0		4	5	1	0	0	0	0	0			1
13:15:00	134	14	876	54	117	9	2	0	52	2	5	0		0	0	0	0			0
13:30:00	145	11	948	72	125	8	3	1	55	3	5	0	-	0	0	0	0			2
13:45:00	153	8		65	127	2	3	0	56	1	6	1	0	0	0	0	0			0
14:00:00	173	20		47	141	14	3	0	61	5	8	2	-	0	0	0	0			0
14:01:24	173	0		0	141	0	3	0		0	8	0	-	0	0	0	0			0
15:00:00	173	0		0	141	0	3	0	61	0	8	0	-	0	0	0	0			0
15:15:00	197	24	1123	63	152	11	3	0	61	0	8	0	- v	0	0	0	0	-		0
15:30:00	205	8	-	81	162	10	3	0		4	8	0	-	0	0	0	0			0
15:45:00	227	22	1270	66	173	11	3	0	-	2	9	1	0	0	0	0	0			0
16:00:00	242	15	1335	65	181	8	3	0	72	5	9	0	· ·	0	0	0	0	-	~	0
16:15:00	274	32	1415	80	191	10	3	0		2	9	0	-	0	0	0	0			0
16:30:00	294	20	1494	79	207	16	3	0	-	2	9	0	-	0	0	0	0			0
16:45:00	325	31	1600	106	228	21	3	0	80	4	9	0		0	0	0	0			0
17:00:00	353	28	1680	80	239	11	3	0		3	9	0	-	0	0	0	0			3
17:15:00	391	38	1791	111	252	13	3	0	85	2	9	0	-	0	0	0	0			1
17:30:00	414	23	1874	83	268	16	3	0	• •	6	10	1	0	0	0	0	0	-		0
17:45:00	434	20	1941	67	274	6	3	0	95	4	10	0	-	0	0	0	0			0
18:00:00	444	10		73	280	6	3	0	98	3	10	0	-	0	0	0	0			0
18:15:00	444	0		0	280	0	3	0	98	0	-	0	-	0	0	0	0			0
18:17:24	444	0	2014	0	280	0	3	0	98	0	10	0	0	0	0	0	0	C	) 12	C

		Passen	ger Cars ·	- East Ap	proach			Tru	icks - Eas	t Approa	nch			He	avys - Eas	st Approa	ich		Pedes	trians
Interval	Lef	ft	Th	ru	Rig	ht	Le	ft	Th	ru	Rig	lht	Le	ft	Th	ru	Rig	ht	East 0	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
6:00:00	0	0	0	0	0	0	0	0	0	0	0	C	0 0	0	0	0	0	0	0	
6:15:00	11	11	16	16	0	0	2	2	0	0	0	C	0 0	0	0	0	0	0	1	
6:30:00	25	14	43	27	1	1	3	1	1	1	0	C	0 0	0	0	0	0	0	1	
6:45:00	35	10	89	46	1	0	4	1	1	0	0	C	0 0	0	0	0	0	0	2	
7:00:00	53	18	132	43	1	0	6	2	4	3	0	C	0 0	0	0	0	0	0	2	
7:15:00	68	15	161	29	1	0	10	4	5	1	0	C	0 0	0	0	0	0	0	2	
7:30:00	81	13	205	44	2	1	10	0	7	2	0	C	0 0	0	0	0	0	0	3	
7:45:00	104	23	256	51	2	0	11	1	8	1	0	C	0 0	0	0	0	0	0	3	
8:00:00	126	22	319	63	2	0	12	1	9	1	0	C	0 0	0	0	0	0	0	3	
8:15:00	144	18	370	51	2	0	14	2	10	1	0	C	0 0	0	0	0	0	0	8	
8:30:00	162	18	405	35	3	1	17	3	12	2	0	C	0 0	0	0	0	0	0	10	
8:45:00	185	23	452	47	3	0	18	1	13	1	0	C	0 0	0	0	0	0	0	10	
9:00:00	210	25	501	49	3	0	21	3	14	1	0	C	0 0	0	0	0	0	0	10	
9:00:46	210	0	501	0	3	0	21	0	14	0	0	C	0 0	0	0	0	0	0	10	
12:00:00	210	0	501	0	3	0	21	0	14	0	0	C	0 0	0	0	0	0	0	10	
12:15:00	231	21	535	34	3	0	23	2	17	3	0	C	0 0	0	0	0	0	0	10	
12:30:00	266	35	573	38	4	1	23	0	18	1	0	C	0 0	0	0	0	0	0	10	
12:45:00	297	31	615	42	4	0	25	2	20	2	0	C	0 0	0	0	0	0	0	11	
13:00:00	327	30	646	31	4	0	27	2	22	2	0	C	0 0	0	0	0	0	0	13	
13:15:00	350	23	684	38	4	0	29	2	23	1	0	C	0 0	0	0	0	0	0	15	
13:30:00	385	35	732	48	4	0	29	0	25	2	0	C	0 0	0	0	0	0	0	20	
13:45:00	397	12	768	36	6	2	31	2	28	3	0	C	0 0	0	0	0	0	0	21	
14:00:00	422	25	805	37	6	0	34	3	30	2	0	C	0 0	0	0	0	0	0	21	
14:01:24	422	0	805	0	6	0	34	0		0	0	C	0 0	0	0	0	0	0	21	
15:00:00	422	0	805	0	6	0	34	0	30	0	0	C	0 0	0	0	0	0	0	21	
15:15:00	453	31	844	39	7	1	36	2	31	1	0	C	0 0	0	0	0	0	0	21	
15:30:00	476	23	890	46	7	0	37	1	36	5	0	C	0 0	0	0	0	0	0	22	
15:45:00	501	25	933	43	9	2	39	2	39	3	0	C	0 0	0	0	0	0	0	23	
16:00:00	528	27	982	49	10	1	41	2	42	3	0	C	0 0	0	0	0	0	0	23	
16:15:00	561	33	1032	50	11	1	44	3	43	1	0	C	0 0	0	0	0	0	0	23	
16:30:00	585	24	1083	51	11	0	45	1	47	4	0	C	0 0	0	0	0	0	0	23	
16:45:00	613	28	1126	43	11	0	48	3	47	0	0	C	0 0	0	0	0	0	0	26	
17:00:00	635	22	1167	41	11	0	50	2	48	1	0	C	0 0	0	0	0	0	0	26	
17:15:00	664	29	1213	46	11	0	53	3	49	1	0	C	0 0	0	0	0	0	0	28	
17:30:00	691	27	1257	44	11	0	53	0	50	1	0	C	0 0	0	0	0	0	0	29	
17:45:00	713	22	1295	38	11	0	56	3	51	1	0	C	0 0	0	0	0	0	0	29	
18:00:00	735	22	1330	35	12	1	59	3	52	1	0	C	0 0	0	0	0	0	0	29	
18:15:00	735	0	1330	0	12	0	59	0	52	0	0	C	0 0	0	0	0	0	0	29	
18:17:24	735	0	1330	0	12	0	59	0		0	0	C		0	0	0	0	0	29	

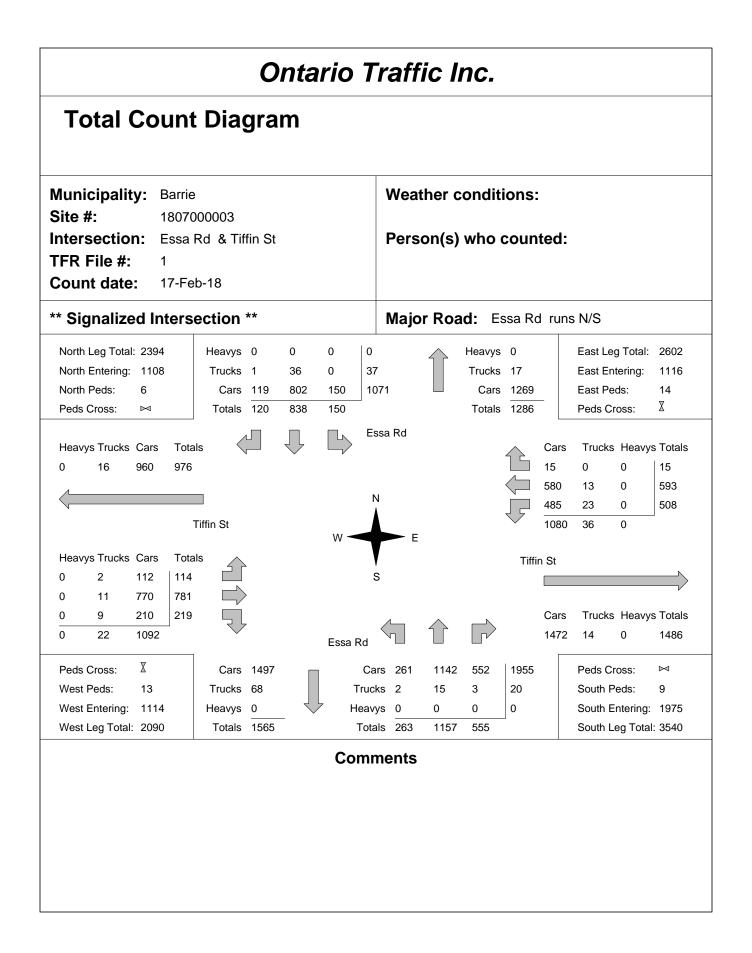
Count Date: 20-Feb-18

Site #: 180700008

Passenger Cars - South Approach **Trucks - South Approach** Heavys - South Approach Pedestrians Interval Left Thru Right Left Thru Right Left Thru Right South Cross Time Cum Incr 6:00:00 6:15:00 6:30:00 6:45:00 7:00:00 7:15:00 7:30:00 7:45:00 8:00:00 8:15:00 8:30:00 8:45:00 9:00:00 9:00:46 12:00:00 12:15:00 12:30:00 12:45:00 13:00:00 13:15:00 13:30:00 13:45:00 14:00:00 14:01:24 15:00:00 15:15:00 15:30:00 15:45:00 16:00:00 16:15:00 16:30:00 16:45:00 17:00:00 17:15:00 17:30:00 17:45:00 18:00:00 18:15:00 18:17:24 

		Passen	ger Cars -	West Ap	proach			Tru	icks - Wes	st Approa	ach			Hea	ivys - Wes	st Approa	ach		Pedes	trians
Interval	Lef	ft	Thi	ru	Rig	ht	Le	ft	Th	ru	Rig	ht	Le	eft	Th	ru	Rig	Jht	West	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
6:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
6:15:00	0	0	12	12	17	17	0	0	0	0	1	1	0	0	0	0	0	0	0	
6:30:00	0	0	29	17	31	14	1	1	0	0	1	0	0	0	0	0	0	0	0	
6:45:00	2	2		22	43	12	1	0	0	0	3	2	0	0	0	0	0	0	0	
7:00:00	4	2	71	20	49	6	1	0	1	1	4	1	0	0	0	0	0	0	0	
7:15:00	5	1	109	38	62	13	2	1	3	2	5	1	0	0	0	0	0	0	0	
7:30:00	8	3	148	39	73	11	2	0	6	3	6	1	0	0	0	0	0	0	0	
7:45:00	12	4		40	88	15	2	0	6	0	7	1	0	0	0	0	0	0	0	
8:00:00	21	9		47	107	19	2	0	8	2	8	1	0	0	0	0	0	0	1	
8:15:00	31	10		55	118	11	3	1	8	0	9	1	0	0	0	0	0	0	2	
8:30:00	47	16		40	133	15	3	0	8	0	13	4	0	0	0	0	0	0	2	
8:45:00	63	16		47	149	16	3	0	9	1	14	1	0	0	0	0	0	0	2	
9:00:00	80	17	431	54	167	18	3	0	9	0	15	1	0	0	0	0	0	0	2	
9:00:46	80	0		0	167	0	3	0	9	0	15	0	-	0	0	0	0	0	2	
12:00:00	80	0	-	0	167	0	3	0	9	0	15	0	-	0	0	0	0	0	2	
12:15:00	92	12		46	180	13	5	2	9	0	16	1	0	0	0	0	0	0	5	
12:30:00	101	9		54	192	12	6	1	13	4	16	0	-	0	0	0	0	0	6	
12:45:00	111	10		50	202	10	6	0	14	1	18	2	0	0	0	0	0	0	8	
13:00:00	119	8		39	214	12	7	1	14	0	18	0	0	0	0	0	0	0	9	
13:15:00	127	8		42	235	21	7	0	15	1	19	1	0	0	0	0	0	0	9	
13:30:00	133	6	707	45	249	14	7	0	17	2	20	1	0	0	0	0	0	0	9	
13:45:00	142	9		46	270	21	8	1	18	1	21	1	0	0	0	0	0	0	12	
14:00:00	150	8		45	289	19	8	0	19	1	24	3	0	0	0	0	0	0	12	
14:01:24	150	0		0	289	0	8	0	19	0	24	0	-	0	0	0	0	0	12	
15:00:00	150	0		0	289	0	8	0	19	0	24	0	-	0	0	0	0	0	12	
15:15:00	158	8	866	68	308	19	9	1	20	1	26	2	0	0	0	0	0	0	14	
15:30:00	169	11	924	58	321	13	9	0	22	2	27	1	0	0	0	0	0	0	17	
15:45:00	177	8		61	339	18	10	1	24	2	29	2	0	0	0	0	0	0	22	
16:00:00	186	9		69	360	21	10	0		0	30	1	0	0	0	0	0	0	23	
16:15:00	196	10		70	382	22	10	0	26	2	31	1	0	0	0	0	0	0	24	
16:30:00	206	10		50	403	21	10	0	26	0	31	0	0	0	0	0	0	0	27	
16:45:00	212	6		86	423	20	10	0	27	1	32	1	0	0	0	0	0	0	27	
17:00:00	223	11	1339	79	440	17	10	0	29	2	33	1	0	0	0	0	0	0	29	
17:15:00	237	14	1411	72	457	17	10	0	30	1	34	1	0	0	0	0	0	0	29	
17:30:00	246	9		74	479	22	11	1	30	0	34	0	0	0	0	0	0	0	29	
17:45:00	250	4	1547	62	498	19	11	0	30	0	35	1	0	0	0	0	0	0	30	
18:00:00	260	10		55	512	14	11	0	30	0	35	0	-	0	0	0	0	0	32	
18:15:00	260	0		0	512	0	11	0	30	0	35	0	0	0	0	0	0	0	32	
18:17:24	260	0	1602	0	512	0	11	0	30	0	35	0	0	0	0	0	0	0	32	

Mid-day Peak Diagram	Specified Period         One Hour Peak           From:         10:00:00         From:         13:00:00           To:         14:00:00         To:         14:00:00
Municipality:BarrieSite #:1807000003Intersection:Essa Rd & Tiffin StTFR File #:1Count date:17-Feb-18	Weather conditions: Person(s) who counted:
** Signalized Intersection **	Major Road: Essa Rd runs N/S
North Leg Total: 659       Heavys 0       0       0       0       0         North Entering: 291       Trucks 0       8       0       8         North Peds: 0       Cars       38       200       45       2         Peds Cross:       Image: 1       Totals       38       208       45	
Heavys Trucks Cars Totals 0 3 227 230	Cars Trucks Heavys Totals 4 0 0 4 131 3 0 134
Tiffin St	N $\frac{146}{281} = 0$ 134 152
Heavys Trucks Cars Totals	Tiffin St
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	S Cars Trucks Heavys Totals
0 6 282 Essa Rd	
Peds Cross:   Image: Cars 400   C     West Peds:   5   Trucks 16   Trucks 16	ars 58 335 171 564 Peds Cross: № cks 0 3 0 3 South Peds: 3
	vys         0         0         0         South Entering:         567           als         58         338         171         South Leg Total:         983
I	ments



				-		o <i>Trafi</i> count S	-	-				
Intersection:	- ssa Rd	& Tiffir		man		<sup>Pate:</sup> 17-Feb-18		nicipality: Ba	rrie			
			ach Tot	als	_	1110010				oach Tot	als	
			rucks, & H	eavys		North/South				rucks, & H	eavys	
Hour Ending	Left	Thru	Right	Grand Total	Total Peds	Total Approaches	Hour Ending	Left	Thru	Right	Grand Total	Total Peds
10:00:00	0	0	0	0	0	0			0	0	0	0
11:00:00	22 36	209 224	25 34	256 294	1 3		11:00:00 12:00:00		245 262	119 109	414 445	4 0
13:00:00	47	197	23	267	2		13:00:00		312	156	549	
14:00:00	45	208	38	291	0		14:00:00		338	171	567	2 3
Totals:	150 East	838 Approa	120 ach Tota	1108 als	6	3083		263 Wes	1157 t Appro	555 ach Tota	1975 als	9
	Include	es Cars, T	rucks, & H	eavys	-	East/West				rucks, & H	eavys	-
Hour Ending	Left	Thru	Right	Grand Total	Total Peds	Total Approaches	Hour Ending	Left	Thru	Right	Grand Total	Total Peds
10:00:00	0	0	0	0	0	0	10:00:00		0	0	0	0
11:00:00 12:00:00	124 106	134 159	1	259 268	7 1		11:00:00 12:00:00		177 181	43 70	245 285	5 2
13:00:00	126	166	7	200	3		13:00:00		217	50	205	1
14:00:00	152	134	4	290	3		14:00:00		206	56	288	5
Totals:	508	593		1116	14 /aluos f	2230 or Troffic Cr	occina 1	114	781	219	1114	13
Hours En Crossing		0:00 0	0:00 0	0:00 0:00	10:00 0	or Traffic Cr	11:00 331	12:00	13:00 376	14:00 387		

Count Date: 17-Feb-18 Site #: 180700003 Passenger Cars - North Approach Heavys - North Approach Trucks - North Approach Pedestrians Interval Left Thru Right Left Thru Right Left Thru Right North Cross Time Cum Cum Incr Incr 10:00:00 10:15:00 10:30:00 10:45:00 11:00:00 11:15:00 11:30:00 11:45:00 12:00:00 12:15:00 12:30:00 12:45:00 13:00:00 13:15:00 13:30:00 13:45:00 14:00:00 14:15:00 14:16:38 

Count Date: 17-Feb-18

Site #: 180700003

Heavys - East Approach **Passenger Cars - East Approach Trucks - East Approach** Pedestrians Interval Left Thru Right Left Thru Right Left Thru Right East Cross Time Cum Incr 10:00:00 10:15:00 10:30:00 10:45:00 11:00:00 11:15:00 11:30:00 11:45:00 12:00:00 12:15:00 12:30:00 12:45:00 13:00:00 13:15:00 13:30:00 13:45:00 14:00:00 14:15:00 14:16:38 

Count Date: 17-Feb-18

Site #: 180700003

Passenger Cars - South Approach Heavys - South Approach Trucks - South Approach Pedestrians Interval Left Thru Right Left Thru Right Left Thru Right South Cross Time Cum Cum Incr Incr 10:00:00 10:15:00 10:30:00 10:45:00 11:00:00 11:15:00 11:30:00 11:45:00 12:00:00 12:15:00 12:30:00 12:45:00 13:00:00 13:15:00 13:30:00 13:45:00 14:00:00 14:15:00 14:16:38 

Count Date: 17-Feb-18

Site #: 180700003

Passenger Cars - West Approach Heavys - West Approach **Trucks - West Approach** Pedestrians Interval Left Thru Right Left Thru Right Left Thru Right West Cross Time Cum Incr 10:00:00 10:15:00 10:30:00 10:45:00 11:00:00 11:15:00 11:30:00 11:45:00 12:00:00 12:15:00 12:30:00 12:45:00 13:00:00 13:15:00 13:30:00 13:45:00 14:00:00 14:15:00 14:16:38 

	On	tario T	raffic	Inc.			
Morning Pe	ak Diagra	ım	From:	ed Period 6:00:00 9:00:00		ne Hour Pe com: 7:45:0 c: 8:45:0	0
	00010 hore Dr & GO Ac	cess		r condition (s) who cou	_		
** Signalized Interse	ection **		Major R	oad: Lake:	shore D	r runs W/E	
						East Leg Total: East Entering: East Peds: Peds Cross:	1552 927 0 ∑
Heavys Trucks Cars Tota 0 5 922 927	ls nore Dr		N		Cars 919 3 7 922	Trucks Heavy 5 0 0 0 5 0	924 3
Heavys Trucks Cars Tota	ls	W -	E		Lakeshore	Dr	
0 10 613 623 0 0 1 1 0 10 614 Peds Cross: X		GO Access	s		Cars 615	Trucks Heavy 10 0	625
Peds Cross:ÅWest Peds:0West Entering:624West Leg Total:1551	Cars 4 Trucks 0 Heavys 0 Totals 4	Truc Heav	ars 3 ks 0 rys 0 als 3	2 5 0 0 0 0 2		Peds Cross: South Peds: South Entering: South Leg Tota	
		Com	nents				

	On	tario 7	raffic	Inc.			
Mid-day Pea	ak Diagra	m	From:	ed Period 12:00:00 14:00:00		ne Hour Po om: 13:00 o: 14:00	:00
	000010 hore Dr & GO Ad	ccess		r conditior s) who co	-		
** Signalized Inters	ection **		Major R	oad: Lake	shore Di	r runs W/E	
						East Leg Tota East Entering: East Peds: Peds Cross:	
Heavys Trucks Cars Tota 0 10 630 640	ls		N		Cars 629 0 629	Trucks Heav 10 0 0 0	639 0
Heavys Trucks Cars Tota		W	E				
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		GO Access	s		Lakeshore Cars 680	Trucks Heav	rys Totals 694
Peds Cross: X West Peds: 2 West Entering: 693 West Leg Total: 1333	Cars 0 Trucks 0 Heavys 0 Totals 0	Truc Heav	ars 1 ks 0 rys <u>0</u> als 1	1 2 0 0 0 0 1		Peds Cross: South Peds: South Entering South Leg Tot	-
		Com	ments		I		

Untario	Traffic Inc.	
Afternoon Peak Diagram	Specified Period         One Hour Peal           From:         15:00:00         From:         16:30:00           To:         18:00:00         To:         17:30:00	)
Municipality:BarrieSite #:1807000010Intersection:Lakeshore Dr & GO AccessTFR File #:1Count date:20-Feb-18	Weather conditions: Person(s) who counted:	
** Signalized Intersection **	Major Road: Lakeshore Dr runs W/E	
	East Entering: 7	1796 744 2 8
Heavys Trucks Cars Totals	Cars Trucks Heavys T	Totals
0 1 742 743	N <del>3</del> 0 0 <del>743</del> 1 0	741 3
W - Heavys Trucks Cars Totals	E Lakeshore Dr	
0 4 1044 0 0 3 0 4 1047 GO Acce	S Cars Trucks Heavys T 1048 4 0 1	Totals
		⊠ 1 6
West Leg Total: 1794 Totals 6	Totals 2 4 South Leg Total: 1	12

Ontario T	raffic Inc.
Total Count Diagram	
Municipality:BarrieSite #:1807000010Intersection:Lakeshore Dr & GO AccessTFR File #:1Count date:20-Feb-18	Weather conditions: Person(s) who counted:
** Signalized Intersection **	Major Road: Lakeshore Dr runs W/E
	East Leg Total: 10677 East Entering: 5311 East Peds: 3 Peds Cross: X
Heavys Trucks Cars Totals 0 46 5341 5387	Cars Trucks Heavys Totals 5252 46 0 5298 13 0 0 13
Lakeshore Dr W Heavys Trucks Cars Totals	E E Lakeshore Dr
0 58 5281 5339 0 0 13 13 0 58 5294 GO Access	Cars Trucks Heavys Totals 5308 58 0 5366
West Peds:     16     Trucks     0     Truck       West Entering:     5352     Heavys     0     Heavy	rs     89     27     116     Peds Cross:     ⋈       ks     0     0     0     South Peds:     2       ys     0     0     0     South Entering:     116       ks     89     27     South Leg Total:     142
Comr	nents

						o <i>Trafi</i> ount S						
Intersection:	_akesho	re Dr &				<sup>vate:</sup> 20-Feb-18		<sup>cipality:</sup> Ba	rrie			
			ach Tot							ach Tot	ale	
			rucks, & H			North/South				rucks, & H		
Hour Ending	Left	Thru	Right	Grand Total	Total Peds	Total	Hour Ending	Left	Thru	Right	Grand Total	Total Peds
6:00:00 7:00:00	0 0	0 0	0	0 0	0 0	0 43	6:00:00 7:00:00	0 37	0 0	0 6	0 43	0 0
8:00:00	0	0	0	0	0	25	8:00:00	20	0	5	25	0
9:00:00 12:00:00	0 0	0 0	0 0	0 0	0 0	6 0	9:00:00 12:00:00	4 0	0 0	2 0	6 0	0 0
13:00:00	0	0	0	0	0	6 2	13:00:00	2	0	4	6	0
14:00:00 15:00:00	0 0	0 0	0 0	0 0	0 0	20	14:00:00 15:00:00	1 0	0 0	1 0	2 0	0 0
16:00:00 17:00:00	0 0	0 0	0 0	0 0	0 0	1 5	16:00:00 17:00:00	0 3	0 0	1 2	1 5	1
18:00:00	0	0	0	0	0	28	18:00:00	22	0	6	28	0
Totals:	0	0	0	0	0	116		89	0	27	116	2
TOLAIS.	•	-	ach Tota	-	0	110			-	ach Tota		Z
			rucks, & H	eavys	_	Ea <u>s</u> t/West				rucks, & H	eavys	_
Hour Ending	Left	Thru	Right	Grand Total	Total Peds	Total Approaches	Hour Ending	Left	Thru	Right	Grand Total	Total Peds
6:00:00 7:00:00	0	0 315	0	0 318	0	0	6:00:00 7:00:00	0	0 225	0	0 226	0
8:00:00	3 0	690	0 0	690	0 0	544 1189	8:00:00	0 0	225 494	1 5	499	0
9:00:00 12:00:00	3 0	931 0	0 0	934 0	0 0	1547 0	9:00:00 12:00:00	0 0	612 0	1 0	613 0	0
13:00:00	0	581	0	581	0	1207	13:00:00	0	626	0	626	Ő
14:00:00 15:00:00	0 0	639 0	0 0	639 0	0 0	0	14:00:00 15:00:00	0 0	693 0	0 0	693 0	2 0
16:00:00 17:00:00	0 2	668 776	0 0	668 778	0 0		16:00:00 17:00:00	0 0	829 995	1	830 996	3 5
18:00:00	5	698	0	703	3		18:00:00	0	865	4	869	5
Totolo	10	FOOD		E244		10000			E220		FOFO	
Totals:	13	5298	0 <b>Calc</b>	5311 ulated V	3 alues f	10663 or Traffic Cr	ossina M	0 ajor Stre	5339 eet	13	5352	16
Hours En Crossing		7:00 38	8:00 20	9:00 4	13:00 2		14:00 3	-	17:00 8	18:00 30		

		Passen	ger Cars -	North A	pproach			Tru	cks - Nor	th Appro	ach			Hea	vys - Nor	th Appro	ach		Pedes	strians
Interval	Le	ft	Th	ru	Rig	lht	Le	ft	Th	ru	Rig	jht	Le	eft	Th	ru	Rig	ht	North	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Inc
6:00:00	0	0	0	0	0	0	0	0	0	0	0	C	0 0	0	0	0	0	0	0	i
6:15:00	0	0	0	0	0	0	0	0	0	0	0	C	0 0	0	0	0	0	0	0	,
6:30:00	0	0	0	0	0	0	0	0	0	0	0	C	0 0	0	0	0	0	0	0	i
6:45:00	0	0	0	0	0	0	0	0	0	0	0	C	0 0	0	0	0	0	0	0	i
7:00:00	0	0	0	0	0	0	0	0	0	0	0	C	0 0	0	0	0	0	0	0	i
7:15:00	0	0	0	0	0	0	0	0	0	0	0	C	0 0	0	0	0	0	0	0	1
7:30:00	0	0	0	0	0	0	0	0	0	0	0	C	0 0	0	0	0	0	0	0	1
7:45:00	0	0	0	0	0	0	0	0	0	0	0	C	0 0	0	0	0	0	0	0	i
8:00:00	0	0	0	0	0	0	0	0	0	0	0	C	0 0	0	0	0	0	0	0	1
8:15:00	0	0	0	0	0	0	0	0	0	0	0	C	0 0	0	0	0	0	0	0	1
8:30:00	0	0	0	0	0	0	0	0	0	0	0	C	0 0	0	0	0	0	0	0	ł
8:45:00	0	0	0	0	0	0	0	0	0	0	0	C	0 0	0	0	0	0	0	0	ł
9:00:00	0	0	0	0	0	0	0	0	0	0	0	C	0 0	0	0	0	0	0	0	ł
9:01:44	0	0	0	0	0	0	0	0	0	0	0	C	0 0	0	0	0	0	0	0	i
12:00:00	0	0	0	0	0	0	0	0	0	0	0	C	0 0	0	0	0	0	0	0	i
12:15:00	0	0	0	0	0	0	0	0	0	0	0	C	0 0	0	0	0	0	0	0	1
12:30:00	0	0	0	0	0	0	0	0	0	0	0	C	0 0	0	0	0	0	0	0	i
12:45:00	0	0	0	0	0	0	0	0	0	0	0	C	0 0	0	0	0	0	0	0	i
13:00:00	0	0	0	0	0	0	0	0	0	0	0	C	0 0	0	0	0	0	0	0	i
13:15:00	0	0	0	0	0	0	0	0	0	0	0	C	0 0	0	0	0	0	0	0	i
13:30:00	0	0	0	0	0	0	0	0	0	0	0	C	0 0	0	0	0	0	0	0	i
13:45:00	0	0	0	0	0	0	0	0	0	0	0	C	0 0	0	0	0	0	0	0	i
14:00:00	0	0	0	0	0	0	0	0	0	0	0	C	0 0	0	0	0	0	0	0	1
14:00:36	0	0	0	0	0	0	0	0	0	0	0	C	0 0	0	0	0	0	0	0	i
15:00:00	0	0	0	0	0	0	0	0	0	0	0	C	0 0	0	0	0	0	0	0	i
15:15:00	0	0	0	0	0	0	0	0	0	0	0	C	0 0	0	0	0	0	0	0	,
15:30:00	0	0	0	0	0	0	0	0	0	0	0	C	0 0	0	0	0	0	0	0	,
15:45:00	0	0	0	0	0	0	0	0	0	0	0	C	0 0	0	0	0	0	0	0	,
16:00:00	0	0	0	0	0	0	0	0	0	0	0	C	0 0	0	0	0	0	0	0	,
16:15:00	0	0	0	0	0	0	0	0	0	0	0	C	0 0	0	0	0	0	0	0	,
16:30:00	0	0	0	0	0	0	0	0	0	0	0	C	0 0	0	0	0	0	0	0	,
16:45:00	0	0	0	0	0	0	0	0	0	0	0	C	0 0	0	0	0	0	0	0	,
17:00:00	0	0	0	0	0	0	0	0	0	0	0	C	0 0	0	0	0	0	0	0	,
17:15:00	0	0	0	0	0	0	0	0	0	0	0	C		0	0	0	0	0	0	
17:30:00	0	0	0	0	0	0	0	0	0	0	0	C	-	0	0	0	-	0	0	
17:45:00	0	0	0	0	0	0	0	0	0	0	0	C	-	0	0	0	-	0	0	
18:00:00	0	0	0	0	0	0	0	0	0	0	0	C	-	0	0	0	0	0	0	
18:15:00	0	0	0	0	0	0	0	0	0	0	0	C		0	0	0	0	0	0	
18:15:43	0	0		0	0	0	0	0	0	0	0	C		0	0	0		0	0	

		Passer	nger Cars -	East Ap	proach			Tru	icks - Eas	st Approa	ach			Hea	avys - Eas	st Approa	ich		Pedes	trians
Interval	Let	ft	Thr	u	Rig	ht	Le	ft	Th	ru	Rig	ht	Le	ft	Th	ru	Rig	ht	East C	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
6:00:00	0	C	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
6:15:00	1	1	50	50	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
6:30:00	2	1	130	80	0	0	0	0	1	1	0	0	0	0	0	0	0	0	0	
6:45:00	3	1	227	97	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	
7:00:00	3	C	314	87	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	
7:15:00	3	C	431	117	0	0	0	0	2	1	0	0	0	0	0	0	0	0	0	
7:30:00	3	C	585	154	0	0	0	0	2	0	0	0	0	0	0	0	0	0	0	
7:45:00	3	C	775	190	0	0	0	0	3	1	0	0	0	0	0	0	0	0	0	
8:00:00	3	C	1002	227	0	0	0	0	3	0	0	0	0	0	0	0	0	0	0	
8:15:00	4	1	1237	235	0	0	0	0	5	2	0	0	0	0	0	0	0	0	0	
8:30:00	4	C	1458	221	0	0	0	0	5	0	0	0	0	0	0	0	0	0	0	
8:45:00	6	2	1694	236	0	0	0	0	8	3	0	0	0	0	0	0	0	0	0	
9:00:00	6	C	1928	234	0	0	0	0	8	0	0	0	0	0	0	0	0	0	0	
9:01:44	6	C	1928	0	0	0	0	0	8	0	0	0	0	0	0	0	0	0	0	
12:00:00	6	C	1928	0	0	0	0	0	8	0	0	0	0	0	0	0	0	0	0	
12:15:00	6	C	2074	146	0	0	0	0	9	1	0	0	0	0	0	0	0	0	0	
12:30:00	6	C	2214	140	0	0	0	0	10	1	0	0	0	0	0	0	0	0	0	
12:45:00	6	C	2358	144	0	0	0	0	11	1	0	0	0	0	0	0	0	0	0	
13:00:00	6	C	2503	145	0	0	0	0	14	3	0	0	0	0	0	0	0	0	0	
13:15:00	6	C	2652	149	0	0	0	0	15	1	0	0	0	0	0	0	0	0	0	
13:30:00	6	0	2835	183	0	0	0	0	19	4	0	0	0	0	0	0	0	0	0	
13:45:00	6	C	2997	162	0	0	0	0	23	4	0	0	0	0	0	0	0	0	0	
14:00:00	6	C	3132	135	0	0	0	0	24	1	0	0	0	0	0	0	0	0	0	
14:00:36	6	C	3132	0	0	0	0	0	24	0	0	0	0	0	0	0	0	0	0	
15:00:00	6	C	3132	0	0	0	0	0	24	0	0	0	0	0	0	0	0	0	0	
15:15:00	6	C	3276	144	0	0	0	0	27	3	0	0	0	0	0	0	0	0	0	
15:30:00	6	C	3444	168	0	0	0	0	30	3	0	0	0	0	0	0	0	0	0	
15:45:00	6	C	3618	174	0	0	0	0	33	3	0	0	0	0	0	0	0	0	0	
16:00:00	6	C	3790	172	0	0	0	0	34	1	0	0	0	0	0	0	0	0	0	
16:15:00	6	C	3991	201	0	0	0	0	35	1	0	0	0	0	0	0	0	0	0	
16:30:00	7	1	4185	194	0	0	0	0	42	7	0	0	0	0	0	0	0	0	0	
16:45:00	8	1	4368	183	0	0	0	0	42	0	0	0	0	0	0	0	0	0	0	
17:00:00	8	C	4558	190	0	0	0	0	42	0	0	0	0	0	0	0	0	0	0	
17:15:00	9	1	4749	191	0	0	0	0	43	1	0	0	0	0	0	0	0	0	0	
17:30:00	10	1	4925	176	0	0	0	0		0	0	0	0	0	0	0	0	0	2	
17:45:00	11	1	5080	155	0	0	0	0		1	0	0	0	0	0	0	0	0	2	
18:00:00	13	2		172	0	0	0	0	46	2	0	0	0	0	0	0	0	0	3	
18:15:00	13	0		0	0	0	0	0	46	0	0	0	-	0	0	0	0	0	3	
18:15:43	13	0		0	0	0	0	0		0		0		0		0	0	0	3	

		Passeng	er Cars -	South A	pproach			True	cks - Sou	th Appro	ach			Hea	vys - Sou	th Appro	ach		Pedes	trians
Interval	Le	ft	Th	ru	Rig	ht	Le	ft	Th	ru	Rig	Jht	Le	ft	Thr	ru	Rig	ht	South	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
6:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0 0	0	0	0	0	0	0	C
6:15:00	11	11	0	0	0	0	0	0	0	0	0	0	0 0	0	0	0	0	0	0	0
6:30:00	16	5	0	0	3	3	0	0	0	0	0	0	0 0	0	0	0	0	0	0	C
6:45:00	29	13	0	0	4	1	0	0	0	0	0	0	0 0	0	0	0	0	0	0	C
7:00:00	37	8	0	0	6	2	0	0	0	0	0	0	0 0	0	0	0	0	0	0	C
7:15:00	54	17	0	0	11	5	0	0	0	0	0	0	0 0	0	0	0	0	0	0	0
7:30:00	57	3	0	0	11	0	0	0	0	0	0	0	0 0	0	0	0	0	0	0	0
7:45:00	57	0	0	0	11	0	0	0	0	0	0	0	0 0	0	0	0	0	0	0	0
8:00:00	57	0	0	0	11	0	0	0	0	0	0	0	0 0	0	0	0	0	0	0	0
8:15:00	60	3	0	0	12	1	0	0	0	0	0	0	-	0	0	0	0	0	0	0
8:30:00	60	0	0	0	12	0	0	0	0	0	0	0		0	0	0	0	0	0	0
8:45:00	60	0	0	0	13	1	0	0	0	0	0	0	0 0	0	0	0	0	0	0	0
9:00:00	61	1	0	0	13	0	0	0	0	0	0	0	0 0	0	0	0	0	0	0	0
9:01:44	61	0	0	0	13	0	0	0	0	0	0	0	0 0	0	0	0	0	0	0	0
12:00:00	61	0	0	0	13	0	0	0	0	0	0	0	0 0	0	0	0	0	0	0	C
12:15:00	61	0	0	0	15	2	0	0	0	0	0	0	0 0	0	0	0	0	0	0	C
12:30:00	63	2	0	0	16	1	0	0	0	0	0	0	0 0	0	0	0	0	0	0	C
12:45:00	63	0	0	0	17	1	0	0	0	0	0	0	0 0	0	0	0	0	0	0	0
13:00:00	63	0	0	0	17	0	0	0	0	0	0	0	0 0	0	0	0	0	0	0	0
13:15:00	63	0	0	0	17	0	0	0	0	0	0	0	0 0	0	0	0	0	0	0	0
13:30:00	64	1	0	0	17	0	0	0	0	0	0	0	0 0	0	0	0	0	0	0	0
13:45:00	64	0	0	0	17	0	0	0	0	0	0	0	0 0	0	0	0	0	0	0	0
14:00:00	64	0	0	0	18	1	0	0	0	0	0	0	-	0	0	0	0	0	0	C
14:00:36	64	0	0	0	18	0	0	0	0	0	0	0	-	0	0	0	0	0	0	C
15:00:00	64	0	0	0	18	0	0	0	0	0	0	0	0 0	0	0	0	0	0	0	C
15:15:00	64	0	0	0	18	0	0	0	0	0	0	0	0 0	0	0	0	0	0	0	C
15:30:00	64	0	0	0	18	0	0	0	0	0	0	0	0 0	0	0	0	0	0	0	0
15:45:00	64	0	0	0	18	0	0	0	0	0	0	0	0 0	0	0	0	0	0	1	1
16:00:00	64	0	0	0	19	1	0	0	0	0	0	0		0	0	0	0	0	1	C
16:15:00	66	2	0	0	19	0	0	0	0	0	0	0	0 0	0	0	0	0	0	1	0
16:30:00	67	1	0	0	19	0	0	0	0	0	0	0	0 0	0	0	0	0	0	1	C
16:45:00	67	0	0	0	19	0	0	0	0	0	0	0	-	0	0	0	0	0	1	C
17:00:00	67	0	0	0	21	2	0	0	0	0	0	0		0	0	0	0	0	2	1
17:15:00	68	1	0	0	22	1	0	0	0	0	0	0		0	0	0	-	0	2	C
17:30:00	69	1	0	0	23	1	0	0	0	0	0	0	-	0	0	0	0	0	2	0
17:45:00	86	17	0	0	26	3	0	0	0	0	0	0	0 0	0	0	0	0	0	2	0
18:00:00	89	3	0	0	27	1	0	0	0	0	0	0	-	0	0	0	0	0	2	0
18:15:00	89	0	0	0	27	0	0	0	0	0	0	0	0 0	0	0	0	0	0	2	0
18:15:43	89	0	0	0	27	0	0	0	0	0	0	0	0 0	0	0	0	0	0	2	C

**Trucks - West Approach** 

Heavys - West Approach

Pedestrians

West Cross

Incr

Cum

Count Date: 20-Feb-18 Site #: 1807000010

18:15:00

18:15:43

Passenger Cars - West Approach

Interval Left		ger eure meerspreaen										noutje neet ippreuen						
		eft	t Thru		Right		Left		Thru		Right		Left		Thru		Right	
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
6:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
6:15:00	0	0	35	35	0	0	0	0	0	0	0	0	0	0	0	0	0	0
6:30:00	0	0	84	49	0	0	0	0	0	0	0	0	0	0	0	0	0	0
6:45:00	0	0	154	70	0	0	0	0	1	1	0	0	0	0	0	0	0	0
7:00:00	0	0	223	69	1	1	0	0	2	1	0	0	0	0	0	0	0	0
7:15:00	0	0	325	102	5	4	0	0	5	3	0	0	0	0	0	0	0	0
7:30:00	0	0	432	107	6	1	0	0	8	3	0	0	0	0	0	0	0	0
7:45:00	0	0	561	129	6	0	0	0	8	0	0	0	0	0	0	0	0	0
8:00:00	0	0	707	146	6	0	0	0	12	4	0	0	0	0	0	0	0	0
8:15:00	0	0	874	167	7	1	0	0	14	2	0	0	0	0	0	0	0	0
8:30:00	0	0	1026	152	7	0	0	0	16	2	0	0	0	0	0	0	0	0
8:45:00	0	0	1174	148	7	0	0	0	18	2	0	0	0	0	0	0	0	0
9:00:00	0	0	1311	137	7	0	0	0	20	2	0	0	0	0	0	0	0	0
9:01:44	0	0	1311	0	7	0	0	0	20	0	0	0	0	0	0	0	0	0
12:00:00	0	0		0	7	0	0	0		0	0	0	0	0	0	0	0	0
12:15:00	0	0	1486	175	7	0	0	0	21	1	0	0	0	0	0	0	0	0
12:30:00	0	0	1651	165	7	0	0	0	25	4	0	0	0	0	0	0	0	0
12:45:00	0	0	1799	148	7	0	0	0	27	2	0	0	0	0	0	0	0	0
13:00:00	0	0	1930	131	7	0	0	0	27	0	0	0	0	0	0	0	0	0
13:15:00	0	0	2103	173	7	0	0	0	31	4	0	0	0	0	0	0	0	0
13:30:00	0	0	2284	181	7	0	0	0	37	6	0	0	0	0	0	0	0	0
13:45:00	0	0	2442	158	7	0	0	0	38	1	0	0	0	0	0	0	0	0
14:00:00	0	0	2609	167	7	0	0	0	41	3	0	0	0	0	0	0	0	0
14:00:36	0	0	2609	0	7	0	0	0	41	0	0	0	0	0	0	0	0	0
15:00:00	0	0	2609	0	7	0	0	0	41	0	0	0	0	0	0	0	0	0
15:15:00	0	0	2831	222	8	1	0	0	43	2	0	0	0	0	0	0	0	0
15:30:00	0	0	3001	170	8	0	0	0	45	2	0	0	0	0	0	0	0	0
15:45:00	0	0	3204	203	8	0	0	0	48	3	0	0	0	0	0	0	0	0
16:00:00	0	0	3428	224	8	0	0	0	51	3	0	0	0	0	0	0	0	0
16:15:00	0	0	3677	249	8	0	0	0	53	2	0	0	0	0	0	0	0	0
16:30:00	0	0	3889	212	9	1	0	0	54	1	0	0	0	0	0	0	0	0
16:45:00	0	0	4150	261	9	0	0	0	55	1	0	0	0	0	0	0	0	0
17:00:00	0	0	4418	268	9	0	0	0	56	1	0	0	0	0	0	0	0	0
17:15:00	0	0	4676	258	12	3	0	0	58	2	0	0	0	0	0	0	0	0
17:30:00	0	0	4933	257	12	0	0	0	58	0	0	0	0	0	0	0	0	0
17:45:00	0	0		197	12	0	0	0	58	0	0	0	0	0	0	0	0	0
18:00:00	0	0	5281	151	13	1	0	0		0	0	0	0	0	0	0	0	0
															-			-

	Ontai						Dook		
Mid-day Pea	ak Diagram		Specified From: 1	0:00:00		<b>Dne Hour Peak</b> From: 13:00:00			
			<b>To:</b> 1	4:00:00	Тс	<b>):</b> 14:0	0:00		
	) 000005 hore Dr & GO Access		Weather conditions:						
TFR File #:1Count date:17-Fe	5	Person(s) who counted:							
** Signalized Inters	ection **		Major Ro	ad: Lake	eshore D	r runs W/E			
						East Leg Tot East Entering East Peds: Peds Cross:			
Heavys Trucks Cars Tota	ls				Cars	Trucks Hea	avys Totals		
0 4 632 636		N			628	4 0 0 0	632		
Lakes	hore Dr	w	E E	Z	- <del>629</del>	4 0	'		
Heavys Trucks Cars Tota	ls		Lakeshore Dr						
0 4 726 730		S							
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	G	O Access			Cars 731	Trucks Hea 4 0	avys Totals 735		
Peds Cross: X West Peds: 9	Cars 3 Trucks 0	Cars Trucks		5 9 0 0		Peds Cross: South Peds:	⊠ 1		
West Entering: 732 West Leg Total: 1368	Heavys 0 Totals 3	Heavys Totals		0 0 5		South Enterin	-		
-		Comme	onto		I				

Ontario Traffic Inc.										
Total Count Diagram										
Municipality:BarrieSite #:1807000005Intersection:Lakeshore Dr & GO AccessTFR File #:1Count date:17-Feb-18	Weather conditions: Person(s) who counted:									
** Signalized Intersection **	Major Road: Lakeshore Dr runs W/E									
	East Leg Total: 4871 East Entering: 2373 East Peds: 10 Peds Cross: <sup>X</sup>									
Heavys Trucks Cars Totals 0 13 2385 2398	Cars Trucks Heavys Totals									
Lakeshore Dr	$E = \frac{8 & 0 & 0}{2360 & 13 & 0} = 8$									
Heavys Trucks Cars Totals	Lakeshore Dr									
0 0 13 13 0 19 2466 GO Access	Cars Trucks Heavys Totals 2479 19 0 2498									
West Peds:     24     Trucks     0     Trucks       West Entering:     2485     Heavys     0     Heav	ars332659Peds Cross: $\bowtie$ iks000South Peds:3ys000South Entering:59als3326South Leg Total:80									
Comments										

						o <i>Trafi</i> count S						
Intersection:	_akesho	re Dr &				<sup>vate:</sup> 17-Feb-18		<sup>cipality:</sup> Ba	rrie			
			ach Tot							oach Tot	als	
	Include	es Cars, T	rucks, & H	eavys		North/South				rucks, & H		
Hour Ending	Left	Thru	Right	Grand Total	Total Peds	Total Approaches	Hour Ending	Left	Thru	Right	Grand Total	Total Peds
10:00:00 11:00:00	0 0	0 0	0 0	0 0	0 0	0 22	10:00:00 11:00:00	0 13	0 0	0 9	0 22	0
12:00:00 13:00:00 14:00:00	0 0 0	0 0 0	0 0 0	0 0 0	0 0 0	19 9 9	12:00:00 13:00:00 14:00:00	12 4 4	0 0 0	7 5 5	19 9 9	1 1 1
Totals:	0	0	0	0	0	59		33	0	26	. 59	3
	Last Include	es Cars. T	ach Tota rucks, & H	IIS eavvs		<b>F</b> = = (0.0/2 = 1)		Include	t <b>Appro</b> es Cars. T	ach Tota rucks, & H	ais eavvs	
Hour				Grand	Total	East/West Total	Hour				Grand	Total
Ending 10:00:00 11:00:00 12:00:00 13:00:00	Left 0 4 1 2	Thru 0 530 599 604	Right 0 0 0 0	Total 0 534 600 606	Peds 0 5 0 4	1041 1184 1268	11:00:00 12:00:00 13:00:00	Left 0 0 0 0	Thru 0 501 582 659	Right 0 6 2 3	Total 0 507 584 662	Peds 0 3 9 3
14:00:00	1	632	0	633	1	1365	14:00:00	0	730	2	732	9
Totals:	8	2365	0 Calc	2373 ulated V	10 Values f	4858 or Traffic Cr		0 aior Stre	2472	13	2485	24
Hours En Crossing		0:00 0	0:00 0	0:00 0	10:00 0		11:00 21	12:00 21	13:00 11	14:00 14		

	I	Passeng	jer Cars -	North A	oproach			Tru	cks - Nor	th Appro	ach			Hea	vys - Nort	h Appro	ach		Pedes	trians
Interval	Lef	t	Th	ru	Riç	Jht	Le	eft	Th	ru	Rig	lht	Lei	it	Thr	u	Rig	Jht	North	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
10:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
10:15:00	0	0	0	0	0	0	0	0		0		0		0	0	0	0	0	0	
10:30:00	0	0	0		0	0	0	0				0		0	0	0	0	0	0	
10:45:00	0	0	0	0	0	0	0	0		0		0		0	0	0	0	0	0	
11:00:00	0	0	0	0	0	0	0	0		0		0		0	0	0	0	0	0	
11:15:00	0	0	0	0	0	0	0	0	-	0	-	0		0	0	0	0	0	0	
11:30:00 11:45:00	0	0	0	0	0	0	0	0		0		0		0	0	0	0	0	0	
12:00:00	0	0	0	0	0	0	0	0		0		0		0	0	0	0	0	0	
12:00:00	0	0	0	0	0	0	0	0		0		0		0	0	0	0	0	0	
12:30:00	0	0	0	0	0	0	0	0		0		0		0	0	0	0	0	0	
12:45:00	0	0	0	0	0	0	0	0		0		0		0	0	0	0	0	0	
13:00:00	0	0	0	0	0	0	0	0		0		0		0	0	0	0	0	0	
13:15:00	0	0	0	0	0	0	0	0		0		0		0	0	0	0	0	0	
13:30:00	0	0	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0	
13:45:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
14:00:00	0	0	0	0	0	0	0	0	0			0		0	0	0	0	0	0	
14:15:00 14:16:49	0	0	0	0	0	0	0	0		0		0		0	0	0	0	0	0	

		Passeng	ger Cars -	East Ap	proach			Tru	cks - Eas	st Approa	ach			Неа	avys - Eas	t Approa	ach		Pedes	trians
Interval	Lei	ft	Thr	u	Rig	ht	Le	ft	Th	ru	Rig	ht	Le	ft	Thr	·u	Rig	jht	East (	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
10:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
10:15:00	2	2	118	118	0	0	0	0	0	0		0		0	0	0	0	0	0	
10:30:00	2	0	251	133	0	0	0	0	2	2		0		0	0	0	0	0	0	
10:45:00	3	1	387	136	0	0	0	0	2	0	-	0		0	0	0	0	0	3	
11:00:00	4	1	528	141	0	0	0	0	2	0		0		0	0	0	0	0	5	
11:15:00	4	0	660	132	0	0	0	0	3	1	0	0	-	0	0	0	0	0	5	
11:30:00	4	0	822	162	0	0	0	0	5	2		0		0	0	0	0	0	5	
11:45:00	4	0	966	144	0	0	0	0	5	0		0		0	0	0	0	0	5	
12:00:00	5	1	1124	158	0	0	0	0	5	0		0		0	0	0	0	0	5	
12:15:00	5	0	1280	156 151	0	0	0	0	6	1	0	0		0	0	0	0	0	9	
12:30:00	5	0	1431		0	0	0	0		1	0	0		0	0	0	0	0		
12:45:00 13:00:00	6 7	1	1578 1724	147 146	0	0	0	0	8	1	0	0		0	0	0	0	0	9	
13:15:00	7	0	1724	146	0	0	0	0	9	0	0	0	-	0	0	0	0	0	9	
13:30:00	7	0	2037	150	0	0	0	0	10	0	0	0		0	0	0	0	0	9	
13:45:00	7	0	2037	166	0	0	0	0	11	1	0	0		0	0	0	0	0	10	
14:00:00	8	1	2352	149	0	0	0	0	13	2	-	0		0	0	0	0	0	10	
14:15:00	8	0	2352	0	0	0	0	0	13	0		0		0	0	0	0	0	10	
14:16:49	8	0	2352	0	0	0	0	0	13	0	-	0	-	0	0	0	0	0	10	

		Passeng	er Cars -	South Ap	proach			Truc	cks - Sou	th Appro	ach			Hea	vys - Sou	th Appro	ach		Pedes	trians
Interval	Le	ft	Th	ru	Rig	ht	Le	ft	Th	ru	Rig	ht	Let	ft	Th	ru	Rig	lht	South	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
10:00:00	0	0	0	0	0	0	0	0		0	0	0		0	0	0	0	0	0	
10:15:00	3	3	0	0	6	6	0	0	0	0	0	0		0	0	0	0	0	0	
10:30:00	3	0	0	0	6	0	0	0	0	0	0	0		0	0	0	0	0	0	
10:45:00	4	1	0	0	8	2	0	0	0	0	0	0		0	0	0	0	0	0	
11:00:00	13	9	0	0	9	1	0	0	0	0	0	0		0	0	0	0	0	0	
11:15:00	21	8	0	0	14	5	0	0	0	0	0	0	-	0	0	0	0	0	0	
11:30:00 11:45:00	23 23	2	0	0	15 15	1	0	0	0	0	0	0		0	0	0	0	0	0	
12:00:00	23	0	0	0	15	0	0	0	0	0	0	0		0	0	0	0	0	1	
12:00:00	25	2 0	0	0 0	16	1	0	0	0	0	0	0		0	0	0	0	0	1	
12:30:00	25	1	0	0	17	0	0	0	0	0	0	0		0	0	0	0	0	2	
12:45:00	20	2	0	0	18	1	0	0	0	0	0	0		0	0	0	0	0	2	
13:00:00	20	1	0	0	21	3	0	0	0	0	0	0		0	0	0	0	0	2	
13:15:00	29	0	0	0	21	0	0	0	0	0	0	0	-	0	0	0	0	0	2	
13:30:00	31	2	0	0	22	1	0	0	0	0	0	0		0	0	0	0	0	2	
13:45:00	32	1	0	0	22	0	0	0	0	0	0	0		0	0	0	0	0	3	
14:00:00	33	1	0	0	26	4	0	0	0	0	0	0	-	0	0	0	0	0	3	
14:15:00	33	0	0	0	26	0	0	0	0	0	0	0		0	0	0	0	0	3	
14:16:49	33	0	0	0	26	0	0	0		0	0	0		0	0	0	0	0	3	

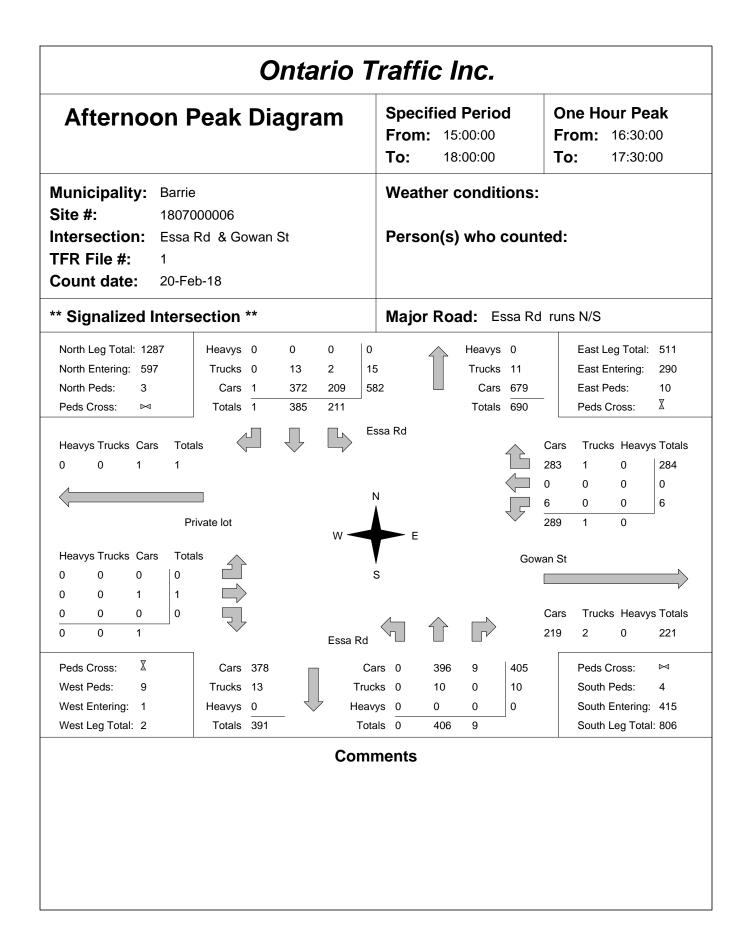
Count Date: 17-Feb-18

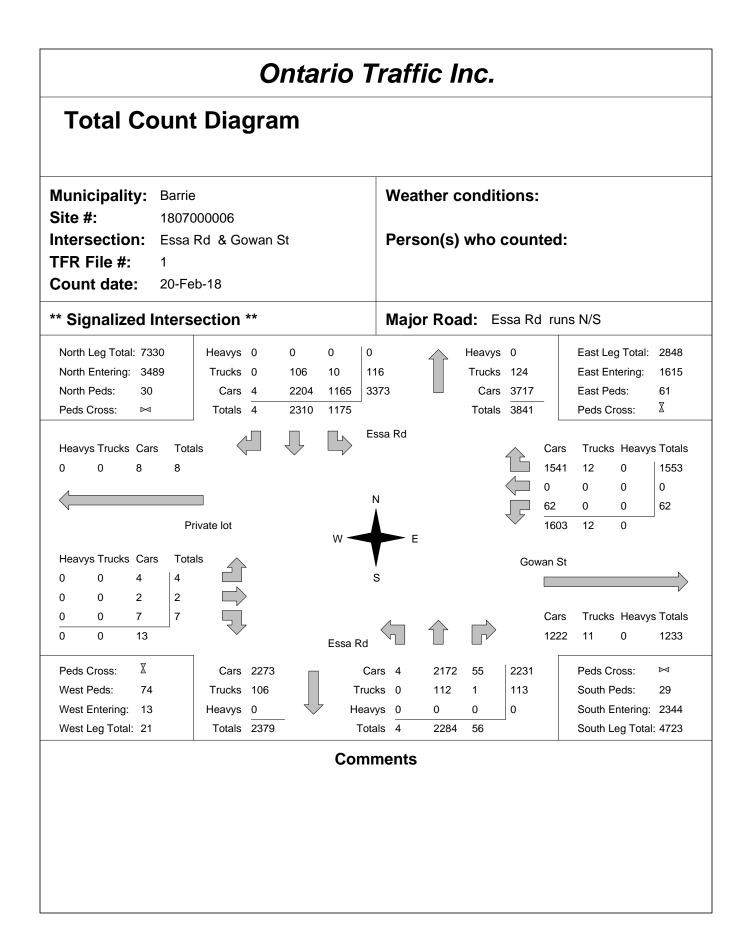
Site #: 180700005

Heavys - West Approach Passenger Cars - West Approach **Trucks - West Approach** Pedestrians Interval Left Thru Right Left Thru Right Left Thru Right West Cross Time Cum Incr 10:00:00 10:15:00 10:30:00 10:45:00 11:00:00 11:15:00 11:30:00 11:45:00 12:00:00 12:15:00 12:30:00 12:45:00 13:00:00 13:15:00 13:30:00 13:45:00 14:00:00 14:15:00 14:16:49 

Morning Peak Diagram	Specified           From:         6:           To:         9:			ne Hour Point         rom:       8:00:0         p:       9:00:0	00
Municipality:BarrieSite #:1807000006Intersection:Essa Rd & Gowan StTFR File #:1Count date:20-Feb-18	Weather Person(s				
** Signalized Intersection **	Major Ro	ad: Essa	a Rd run	s N/S	
North Leg Total:         859         Heavys         0         0         0         0           North Entering:         365         Trucks         0         20         2         2           North Peds:         3         Cars         0         207         136         3           Peds Cross:         Image: March 100         Totals         0         227         138	1 ר	Heavys 0 Trucks 2 Cars 4 Totals 4	0 74	East Leg Tota East Entering: East Peds: Peds Cross:	
Heavys Trucks Cars Totals	ssa Rd	1	Cars	Trucks Heav 3 0	rys Totals │182
Private lot	N		$\begin{array}{c} 0 \\ \hline \\ 7 \\ \hline \\ 183 \end{array}$	0 0 0 0 3 0	0 4
W -	E				
	S		Gowan St		
0 0 1 1 0 0 1 Essa Rd			Cars 140		-
West Peds: 1 Trucks 20 Truck	ars 0 295 cks 0 17	1 1	99 8	Peds Cross: South Peds:	⊠ 2
	vys 0 0 als 0 312	0 0 5		South Entering South Leg Tot	-
	ments		I	0 1	

Site #:1807000006ntersection:Essa Rd & Gowan StFR File #:1Count date:20-Feb-18	True C Tot	o count Essa Rd vys 0 cks 18 ars 498 als 516	runs N/S Eas Eas Eas	S t Leg Total: t Entering: t Peds: ls Cross:	313 188 5 ∑
North Leg Total: 968       Heavys 0       0       0       0         North Entering: 452       Trucks 0       11       1       12         North Peds: 8       Cars 0       321       119       440         Peds Cross: Im       Totals 0       332       120         Heavys Trucks Cars       Totals       0       0       Essa         0       0       0       0       N	Heav Truc C Tot	vys 0 cks 18 ars <u>498</u> als 516	Eas Eas Eas	t Leg Total: t Entering: t Peds:	188 5
North Entering:       452       Trucks       0       11       1       12         North Peds:       8       Cars       0       321       119       440         Peds Cross:       Image: Cars       Totals       0       332       120       Essa         Heavys Trucks       Cars       Totals       Image: Cars       Image: Cars <t< td=""><td>True C Tot</td><td>cks 18 ars <u>498</u> als 516</td><td>Eas Eas</td><td>t Entering: t Peds:</td><td>188 5</td></t<>	True C Tot	cks 18 ars <u>498</u> als 516	Eas Eas	t Entering: t Peds:	188 5
Heavys Trucks Cars Totals	Rd	$\wedge$			
		15	Cars Tru 178 1	ucks Heavy 0	s Totals   179
T mate lot			0 0 9 0 187 1	0 0 0	0 9
Heavys Trucks Cars Totals	E	Gowa	n Ct		
					$ \rightarrow $
0 0 2 2 0 0 2 Essa Rd			Cars Tru 124 1	ucks Heavy 0	
Peds Cross: Z Cars 332 Cars	0 320 5	325	Ped	ls Cross:	$\bowtie$
West Peds: 7 Trucks 11 Trucks		17		th Peds:	4
West Entering: 2 Heavys 0 Heavys	-	0		th Entering:	
West Leg Total:   2   Totals   343   Totals	0 337 5		Sou	th Leg Tota	: 685
Comme	ents				





				-	_		-		-				
Intersection: E	- ssa Rd	& Gow		Tran		ount S			<b>ary</b> <sup>ipality:</sup> Ba	rrie			
			ach Tot	als		2010010					ach Tot	als	
	Include	es Cars, T	rucks, & H	eavys		North/South					rucks, & H	eavys	_
Hour Ending	Left	Thru	Right	Grand Total	Total Peds	Total Approaches	Hou Endir		Left	Thru	Right	Grand Total	Total Peds
6:00:00	0	0	0	0	0	0	6:00		0	0	0	0	0
7:00:00 8:00:00	134 152	129 211	0 0	263 363	1 2	359 562	7:00 8:00		0 0	84 193	12 6	96 199	0 0
9:00:00	138	227	0	365	3	682	9:00	00:0	Ő	312	5	317	2
12:00:00	0	0	0	0	0	0	12:00		0	0	0	0	0
13:00:00 14:00:00	123 119	333 315	0 0	456 434	7 4	779 764	13:00 14:00		0 0	316 324	7 6	323 330	5 8
15:00:00	0	0	0	0	o		15:00		0	0_1	0	0	0
16:00:00	141	354	2 2	497	8	841	16:00		3	338	3	344	10
17:00:00 18:00:00	210 158	369 372	2	581 530	2 3		17:00 18:00		0	366 351	11 6	377 358	4
10.00.00	100	072	Ŭ	000	U	000	10.00			001	0	000	0
Totals:	1175 East	2310 Approa	4 ach Tota	<u>3489</u> I <b>IS</b>	30	5833			4 West	2284 t <b>Appro</b>	56 ach Tota	2344 als	29
	Include	es Cars, T	rucks, & H	eavys		East/West					rucks, & H		
Hour Ending	Left	Thru	Right	Grand Total	Total Peds	Total Approaches	Hou Endir		Left	Thru	Right	Grand Total	Total Peds
6:00:00	0	0	0	0	0	0	6:00	00:0	0	0	0	0	0
7:00:00	6	0	59	65	6	65	7:00		0	0	0	0	5
8:00:00 9:00:00	4	0 0	134 182	138 186	5 9	140 187	8:00 9:00		1 0	0 0	1	2 1	0
12:00:00	0	Ő	0	0	ŏ	0	12:00		0	0	Ö	Ö	Ö
13:00:00	8	0	172	180	4	182	13:00		0	0	2	2	5
14:00:00	11 0	0 0	191 0	202 0	10 0		14:00 15:00		0 0	0 0	1	1 0	15 0
16:00:00	6	0	254	260	5		16:00		1	0	2	3	22
17:00:00	3	0	277	280	15	281	17:00	00:0	1	0	0	1	13
18:00:00	20	0	284	304	1	307	18:00	00:00	1	2	0	3	13
Totals:	62	0	1553 Colo	1615	61 61	1628 or Troffic Cr	0001-	a M-	4 Jor Stra	2	7	13	74
Hours End Crossing		7:00 7	8:00 7	9:00 9:00	13:00 20	or Traffic Cr		<b>д Ма</b> 1:00 23	16:00 25	2 <b>et</b> 17:00 10			

		Passeng	ger Cars -	North Ap	proach			Tru	cks - Nort	h Approa	ach			Hea	vys - Nort	th Appro	ach		Pedes	trians
Interval	Lef	it	Thr	u	Rig	ht	Le	ft	Th	ru	Rig	ht	Le	ft	Thr	ru	Rig	ht	North	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
6:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	C
6:15:00	40	40	23	23	0	0	0	0	3	3	0	0	0	0	0	0	0	0	0	C
6:30:00	75	35	56	33	0	0	0	0	3	0	0	0	0	0	0	0	0	0	0	C
6:45:00	105	30	89	33	0	0	0	0	4	1	0	0	0	0	0	0	0	0	0	C
7:00:00	134	29	123	34	0	0	0	0	6	2	0	0	0	0	0	0	0	0	1	1
7:15:00	164	30	167	44	0	0	0	0	11	5	0	0	0	0	0	0	0	0	3	2
7:30:00	190	26	204	37	0	0	0	0	14	3	0	0	0	0	0	0	0	0	3	C
7:45:00	235	45	260	56	0	0	0	0	16	2	0	0	0	0	0	0	0	0	3	C
8:00:00	285	50	321	61	0	0	1	1	19	3	0	0	0	0	0	0	0	0	3	C
8:15:00	320	35	378	57	0	0	1	0	22	3	0	0	0	0	0	0	0	0	5	2
8:30:00	356	36	422	44	0	0	2	1	29	7	0	0	0	0	0	0	0	0	5	C
8:45:00	383	27	479	57	0	0	2	0	32	3	0	0	0	0	0	0	0	0	5	C
9:00:00	421	38	528	49	0	0	3	1	39	7	0	0	0	0	0	0	0	0	6	1
9:00:43	421	0	528	0	0	0	3	0	39	0	0	0	0	0	0	0	0	0	6	C
12:00:00	421	0	528	0	0	0	3	0	39	0	0	0	0	0	0	0	0	0	6	C
12:15:00	451	30	609	81	0	0	3	0	42	3	0	0	0	0	0	0	0	0	7	1
12:30:00	478	27	691	82	0	0	3	0	44	2	0	0	0	0	0	0	0	0	8	1
12:45:00	512	34	776	85	0	0	4	1	47	3	0	0	0	0	0	0	0	0	10	2
13:00:00	543	31	849	73	0	0	4	0	51	4	0	0	0	0	0	0	0	0	13	3
13:15:00	570	27	930	81	0	0	4	0	53	2	0	0	0	0	0	0	0	0	15	2
13:30:00	605	35	1007	77	0	0	4	0	55	2	0	0	0	0	0	0	0	0	15	C
13:45:00	629	24	1088	81	0	0	4	0		3	0	0	0	0	0	0	0	0	15	C
14:00:00	661	32	1150	62	0	0	5	1	65	7	0	0	0	0	0	0	0	0	17	2
14:01:50	661	0	1150	0	0	0	5	0	65	0	0	0	0	0	0	0	0	0	17	C
15:00:00	661	0	1150	0	0	0	5	0	65	0	0	0	0	0	0	0	0	0	17	C
15:15:00	702	41	1242	92	0	0	6	1	68	3	0	0	0	0	0	0	0	0	21	4
15:30:00	731	29	1331	89	2	2	6	0	71	3	0	0	0	0	0	0	0	0	23	2
15:45:00	766	35	1404	73	2	0	6	0	74	3	0	0	0	0	0	0	0	0	23	C
16:00:00	800	34	1491	87	2	0	7	1	78	4	0	0	0	0	0	0	0	0	25	2
16:15:00	851	51	1582	91	2	0	8	1	83	5	0	0	0	0	0	0	0	0	26	1
16:30:00	891	40	1679	97	3	1	8	0		1	0	0	0	0	0	0	0	0	27	1
16:45:00	955	64	1776	97	4	1	8	0	89	5	0	0	0	0	0	0	0	0	27	C
17:00:00	1009	54	1846	70	4	0	8	0		3	0	0	0	0	0	0	0	0	27	C
17:15:00	1060	51	1965	119	4	0	9	1	95	3	0	0	-	0	0	0	0	0	27	0
17:30:00	1100	40	2051	86	4	0	10	1	97	2	0	0	-	0	0	0	0	0	30	3
17:45:00	1136	36	2127	76	4	0	10	0		6	0	0	-	0	0	0	0	0	30	0
18:00:00	1165	29	2204	77	4	0	10	0		3	0	0	-	0	0	0	0	0	30	C
18:15:00	1165	0	2204	0	4	0	10	0	106	0	0	0	-	0	0	0	0	0	30	C
18:17:06	1165	0	2204	0	4	0	10	0	106	0	0	0	-	0	0	0	0	0	30	0
					· · ·					Ŭ	•		Ŭ	Ŭ	0		- <b>v</b>	Ŭ	50	

East Cross

Incr

Count Date: 20-Feb-18 Site #: 180700006 **Passenger Cars - East Approach Trucks - East Approach** Heavys - East Approach Pedestrians Left Interval Left Thru Right Thru Right Left Thru Right Time Cum Incr Cum 6:00:00 6:15:00 6:30:00 6:45:00 7:00:00 7:15:00 7:30:00 7:45:00 8:00:00 8:15:00 8:30:00 8:45:00 9:00:00 9:00:43 12:00:00 12:15:00 12:30:00 12:45:00 13:00:00 13:15:00 13:30:00 13:45:00 14:00:00 14:01:50 15:00:00 15:15:00 15:30:00 15:45:00 16:00:00 16:15:00 16:30:00 16:45:00 17:00:00 17:15:00 17:30:00 17:45:00 18:00:00 18:15:00 

18:17:06

Count Date: 20-Feb-18

Site #: 180700006

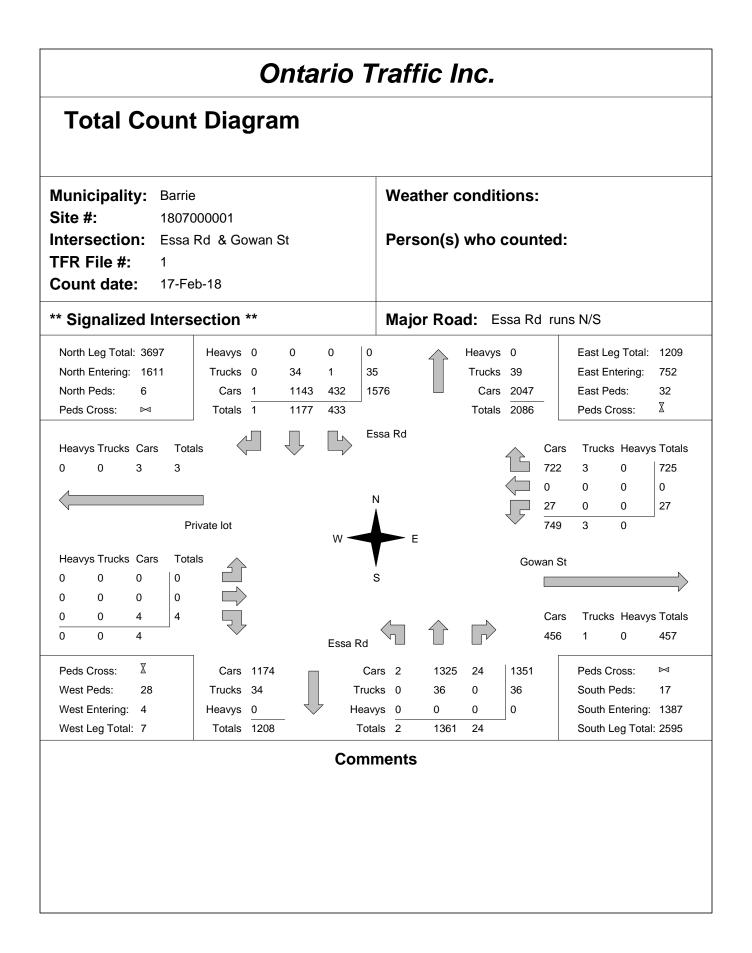
Heavys - South Approach Passenger Cars - South Approach **Trucks - South Approach** Pedestrians Left Interval Left Thru Right Thru Right Left Thru Right South Cross Time Cum Incr 6:00:00 6:15:00 6:30:00 6:45:00 7:00:00 7:15:00 7:30:00 7:45:00 8:00:00 8:15:00 8:30:00 8:45:00 9:00:00 9:00:43 12:00:00 12:15:00 12:30:00 12:45:00 13:00:00 13:15:00 13:30:00 13:45:00 14:00:00 14:01:50 15:00:00 15:15:00 15:30:00 15:45:00 16:00:00 16:15:00 16:30:00 16:45:00 17:00:00 17:15:00 17:30:00 17:45:00 18:00:00 18:15:00 18:17:06

Count Date: 20-Feb-18

Site #: 180700006

Heavys - West Approach **Passenger Cars - West Approach Trucks - West Approach** Pedestrians Interval Left Thru Right Left Thru Right Left Thru Right West Cross Time Cum Incr 6:00:00 6:15:00 6:30:00 6:45:00 7:00:00 7:15:00 7:30:00 7:45:00 8:00:00 8:15:00 8:30:00 8:45:00 9:00:00 9:00:43 12:00:00 12:15:00 12:30:00 12:45:00 13:00:00 13:15:00 13:30:00 13:45:00 14:00:00 14:01:50 15:00:00 15:15:00 15:30:00 15:45:00 16:00:00 16:15:00 16:30:00 16:45:00 17:00:00 17:15:00 17:30:00 17:45:00 18:00:00 18:15:00 18:17:06 

Mid-day Pea	k Diagram		<b>I Period</b> 0:00:00 4:00:00	One Hour PeakFrom:13:00:00To:14:00:00
Municipality:BarrieSite #:1807000Intersection:Essa RoTFR File #:1Count date:17-Feb-	d & Gowan St		conditions: ) who coun	
** Signalized Intersed	ction **	Major Ro	ad: Essa Ro	runs N/S
North Leg Total:1023North Entering:419North Peds:2Peds Cross:⋈	Heavys         0         0         0           Trucks         0         8         0           Cars         0         297         114           Totals         0         305         114		Heavys 0 Trucks 8 Cars 596 Totals 604	East Leg Total: 354 East Entering: 233 East Peds: 3 Peds Cross: X
Heavys Trucks Cars Totals		Essa Rd	^	Cars Trucks Heavys Totals
0 0 0 0			15	223 0 0 223
۸		N		0 0 0 0
Priva	ate lot	N	F	10     0     0       233     0     0
Heavys Trucks Cars Totals	vv		Cov	van St
0 0 0 0		S	Guv	
0 0 0 0				·/
0 0 2 2			N	Cars Trucks Heavys Totals
0 0 2	Ess	a Rd		121 0 0 121
Peds Cross:	Cars 309	Cars 0 373	7   380	Peds Cross:
West Peds: 7	Trucks 8	Trucks 0 8	0 8	South Peds: 4
West Entering: 2	Heavys 0	Heavys 0 0	0 0	South Entering: 388
	Totals 317	Totals 0 381	7	South Leg Total: 705
West Leg Total: 2				



						o <i>Trafi</i> ount S						
Intersection:	- ssa Rd	& Gow		IIai		vate: 17-Feb-18		nicipality: Ba	rrie			
			ach Tot	als						oach Tot	als	
	Include	es Cars, T	rucks, & H	eavys		North/South				rucks, & H		
Hour Ending	Left	Thru	Right	Grand Total	Total Peds	Total Approaches	Hour Ending	Left	Thru	Right	Grand Total	Total Peds
10:00:00	0	0	0	0	0		10:00:00		0	0	0	0
11:00:00	104 111	283 297	1 0	388 408	2 1	702 714	11:00:00 12:00:00		308 302	6 4	314 306	5 6
13:00:00	104	297	0	396	1		13:00:00		302	7	379	2
14:00:00	114	305	0	419	2	807	14:00:00		381	7	388	4
Totals:	433	1177	1 ach Tota	1611	6	2998		2	1361	24 ach Tota	1387	17
	Include	es Cars, T	rucks, & H	eavys		East/West		Include	es Cars, T	rucks, & H	eavys	
Hour Ending	Left	Thru	Right	Grand Total	Total Peds	Total	Hour Ending	Left	Thru	Right	Grand Total	Total Peds
10:00:00	0	0	0	0	0		10:00:00		0	0	0	0
11:00:00	4	0	131	135	5		11:00:00		0	1	1	10
12:00:00	8 5	0 0	158 213	166 218	12 12		12:00:00 13:00:00		0 0	0	0 1	7 4
14:00:00	10	0	223	233	3	235	14:00:00		0	2	2	7
Totals:	27	0	725	752	32	756	•••••	0	0	4	4	28
Hours End		0:00 0	0:00 0	0:00 0:00	10:00 0	or Traffic Cr	0SSING N 11:00 1	12:00		14:00 16		

Count Date: 17-Feb-18

Site #: 180700001

Passenger Cars - North Approach Heavys - North Approach Trucks - North Approach Pedestrians Interval Left Thru Right Left Thru Right Left Thru Right North Cross Time Cum Cum Incr Incr 10:00:00 10:15:00 10:30:00 10:45:00 11:00:00 11:15:00 11:30:00 11:45:00 12:00:00 12:15:00 12:30:00 12:45:00 13:00:00 13:15:00 13:30:00 13:45:00 14:00:00 14:15:00 14:17:13 

Count Date: 17-Feb-18

Site #: 180700001

**Passenger Cars - East Approach** Heavys - East Approach **Trucks - East Approach** Pedestrians Interval Left Thru Right Left Thru Right Left Thru Right East Cross Time Cum Incr 10:00:00 10:15:00 10:30:00 10:45:00 11:00:00 11:15:00 11:30:00 11:45:00 12:00:00 12:15:00 12:30:00 12:45:00 13:00:00 13:15:00 13:30:00 13:45:00 14:00:00 14:15:00 14:17:13 

Count Date: 17-Feb-18

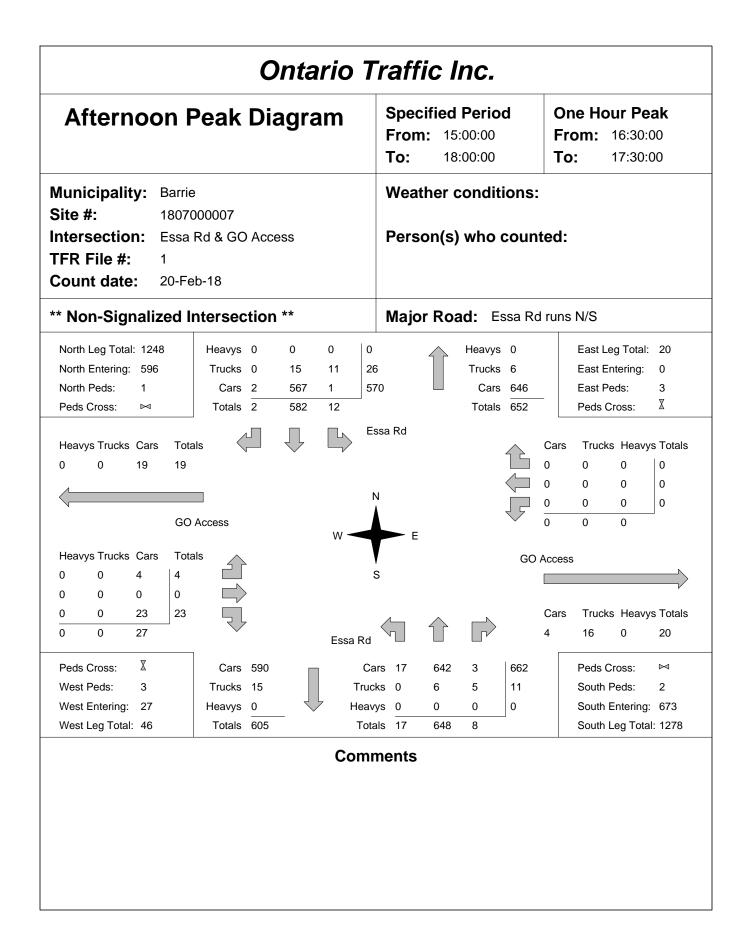
Site #: 180700001

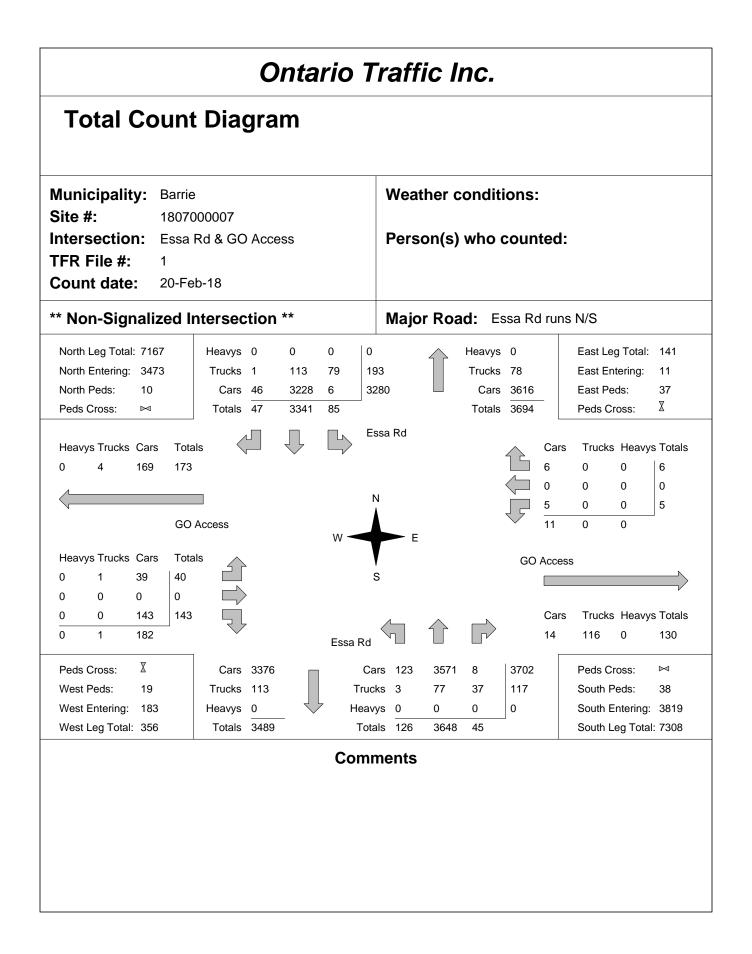
Passenger Cars - South Approach Heavys - South Approach Trucks - South Approach Pedestrians Interval Left Thru Right Left Thru Right Left Thru Right South Cross Time Cum Cum Incr Incr 10:00:00 10:15:00 10:30:00 10:45:00 11:00:00 11:15:00 11:30:00 11:45:00 12:00:00 12:15:00 12:30:00 12:45:00 13:00:00 13:15:00 13:30:00 13:45:00 14:00:00 14:15:00 14:17:13 

oount	Date: 17-I		ger Cars -		180700 pproach	0001		Tru	ucks - We	st Appro	ach			Hea	avys - We	st Appro	ach		Pedes	trians
Interval	Left		Thi		Rig	aht	Le		Th			ght	Le		Th		Rig	ht	West	
Time	Cum In	cr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
10:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:15:00	0	0	0	0	0	0	0	0	0			0	0	0	0	0	0	0	3	3
10:30:00	0	0	0	0	1	1	0	0		0	-	0	-	0		0	0	0	4	1
10:45:00	0	0	0	0	1	0	-	0	-	0		0	-	0		0	0	0	8	4
11:00:00	0	0	0	0	1	0	0	0		0		0		0		0	0	0	10	2
11:15:00	0	0	0	0	1	0	0	0		0	0	0		0		0	0	0	11	1
11:30:00	0	0	0	0	1	0	0	0		0		0		0		0	0	0	12	1
11:45:00	0	0	0	0	1	0	0	0		0		0		0		0	0	0	15	3
12:00:00	0	0	0	0	1	0	0	0	-	0		0	-	0		0	0	0	17	2
12:15:00	0	0	0	0	1	0	0	0		0		0		0		0	0	0	19	2
12:30:00 12:45:00	0	0	0	0	2	0	0	0		0	0	0		0		0	0	0	20 21	1
12:45:00	0	0	0	0	2	0	0	0	-	0	-	0	-	0	-	0	0	0	21	0
13:15:00	0	0	0	0	2	0	_	0	-	0	0	0	-	0		0	0	0	21	0
13:30:00	0	0	0	0	4	2	0	0	-	0	-	0	-	0		0	0	0	21	3
13:45:00	0	0	0	0	4	0	0	0		0		0		0	-	0	0	0	28	4
14:00:00	0	0	0	0	4	0	0	0	-	0	-	0		0		0	0	0	28	0
14:15:00	0	0	0	0	4	0	0	0		0			-	0		0	0	0	28	0
14:17:13	0	0	0	0	4	0	0	0		0		0		0		0	0	0	28	0
															-					

Morning Pe	ak Diag	ram	Spec From To:	<b>1:</b> 6:0	<b>Perioc</b> 00:00 00:00	ł		om:	<b>9:00:00</b>	0
	00007 Rd & GO Acce	ess			conditi who c					
** Non-Signalized Ir	ntersection	**	Majo	r Roa	ad: Es	sa Ro	l runs	N/S		
North Leg Total: 856 North Entering: 375 North Peds: 3 Peds Cross: ы	Heavys 0 Trucks 0 Cars 5 Totals 5	0 0 19 12 339 0 358 12	0 31 344		Heavys Trucks Cars Totals	10 471	_			18 0 9 ∑
Heavys Trucks Cars Tota 0 1 18 19	ls		Essa Rd				Cars	Truck 0	s Heavy 0	s Totals   0
<u></u>	Access		N				0 0 0	0 0	0 0 0	0
		W -					0	U	Ū	
Heavys TrucksCarsTota009000			S			GO	Access	3		
0 0 13 13 0 0 22	$\overline{\nabla}$	Ess	a Rd	$\hat{\mathbf{T}}$			Cars 2		s Heavy 0	
Peds Cross: X West Peds: 1	Cars 352 Trucks 19		Cars 13 Trucks 1	462 10	2 4	477 15		Peds C South		⊠ 1
West Entering: 22 West Leg Total: 41	Heavys 0 Totals 371	_ ~~	Heavys 0 Totals 14	0 472	0	0			Entering: Leg Tota	
			omments		-					

		am	From: To:	ed Perioo 12:00:00 14:00:00	d	One Hour Peak           From:         12:15:00           To:         13:15:00			
	000007 Rd & GO Acces	S		er conditi (s) who c		ed:			
** Non-Signalized I	ntersection *	*	Major F	Road: Es	sa Rd	runs N	N/S		
North Leg Total:944North Entering:447North Peds:2Peds Cross:🖂	Heavys 0 Trucks 1 Cars 9 Totals 10		) 22 425	Heavys Trucks Cars Totals	14 483		East Leo East Ent East Peo Peds Cro	ds:	15 0 4 ∑
Heavys Trucks Cars Tota 0 1 33 34	als		Essa Rd		1	0	Trucks 0	Heavys 0	Totals 0
GO	Access	w -	N E			0 0 0	0 0 0	0 0 0	0
Heavys Trucks Cars Tota	als		Y		GO A	Access			
0 1 9 10			S						
$\begin{array}{cccc} 0 & 0 & 0 & 0 \\ 0 & 0 & 26 & 26 \\ \hline 0 & 1 & 35 & \end{array}$	$\overline{\mathbf{v}}$	Essa Rd				Cars 0		Heavys 0	
Peds Cross:	Cars 442			74 0	498		Peds Cro	055.	
West Peds: 5	Trucks 12		cks 0 1		19		South Pe		7
West Entering: 36	Heavys 0	Hea	vys 0 0	0	0		South Er	ntering:	517
West Leg Total: 70	Totals 454	Tot	tals 24 4	87 6			South Le	eg Total:	971
		Com	ments						





				-		o <i>Trafi</i> ount S	-	-				
Intersection:	- Essa Rd	& GO A		IIai		vate: 20-Feb-18		cipality: Ba	rrie			
			ach Tot	ale		2010010				oach Tot	ale	
			rucks, & H			North/South				rucks, & H		
Hour Ending	Left	Thru	Right	Grand Total	Total Peds	Total Approaches	Hour Ending	Left	Thru	Right	Grand Total	Total Peds
6:00:00	0	0	0	0	0	0	6:00:00	0	0	0	0	0
7:00:00 8:00:00	12 11	263 350	0 2	275 363	0 0	410 675	7:00:00 8:00:00	1 7	130 300	4 5	135 312	7 4
9:00:00	12	358	2 5	375	3	867	9:00:00	14	472	6	492	4
12:00:00	0	0	0	0	Õ	0	12:00:00	0	0	Ő	0	Ö
13:00:00	8	433	6	447	4		13:00:00	21	465	5	491	7
14:00:00	11	406	15	432	1		14:00:00	31	476	5	512	5
15:00:00 16:00:00	0 10	0 464	0 9	0 483	0 1		15:00:00 16:00:00	0 21	0 565	0 6	0 592	0
17:00:00	9	404 542	9 7	403 558	0		17:00:00	21	617	7	645	1
18:00:00	12	525	3	540	1		18:00:00	10	623	7	640	6
Totals:	85	3341	47	3473	10	7292		126	3648	45	3819	38
			ach Tota rucks, & H							ach Tota rucks, & H		
Hour	melude			Grand	Total	East/West Total	Hour	menude	55 Odis, 1		Grand	Total
Ending	Left	Thru	Right	Total	Peds	Approaches	Ending	Left	Thru	Right	Total	Peds
6:00:00	0	0	0	0	0	0	6:00:00	0	0	0	0	0
7:00:00 8:00:00	2 1	0 0	1 0	3 1	0 2	5 14	7:00:00 8:00:00	1 2	0 0	1 11	2 13	0 0
9:00:00	0	0	0	0	9	22	9:00:00	9	0	13	22	1
12:00:00	Ō	0	Ō	0	Ō	0	12:00:00	Ō	Ō	0	0	0
13:00:00	0	0	0	0	3	34	13:00:00	9	0	25	34	6
14:00:00	0	0	0	0	8		14:00:00	5	0		20	2
15:00:00 16:00:00	0 2	0 0	0 0	0 2	0 2		15:00:00 16:00:00	0 7	0 0	0 31	0 38	0 3
17:00:00	0	0	0	0	10		17:00:00	2	0	28	30	4
18:00:00	0	Õ	5	5	3		18:00:00	5	Ő	19	24	3
Totals:	5	0	6	11	37	194		40	0	143	183	19
Hours En Crossing		7:00 10	<b>Calc</b> 8:00 7	ulated V 9:00 13	alues fo 13:00 20	or Traffic Cr	ossing Ma 14:00 11	ajor Stre 16:00 17	eet 17:00 3			

		Passen	ger Cars -	North Ap	proach			Tru	cks - Nori	h Appro	ach			Hea	vys - Nor	th Appro	ach		Pedes	trians
Interval	Lei	ft	Thr	u	Rig	ht	Le	ft	Th	ru	Rig	ht	Le	ft	Th	ru	Rig	ht	North	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
6:00:00	0	C	0 0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
6:15:00	1	1	52	52	0	0	3	3	2	2	0	0		0		0	0	0	0	
6:30:00	3	2	2 123	71	0	0	4	1	4	2	0	0	0	0	0	0	0	0	0	
6:45:00	3	C	191	68	0	0	7	3	6	2	0	0	0	0	0	0	0	0	0	
7:00:00	3	C	254	63	0	0	9	2	9	3	0	0	0	0	0	0	0	0	0	
7:15:00	4	1	322	68	1	1	13	4	13	4	0	0	0	0	0	0	0	0	0	
7:30:00	5	1	386	64	1	0	14	1	17	4	0	0	0	0	0	0	0	0	0	
7:45:00	5	C	479	93	1	0	16	2	19	2	0	0	0	0	0	0	0	0	0	
8:00:00	5	C	590	111	2	1	18	2	23	4	0	0	0	0	0	0	0	0	0	
8:15:00	5	C	685	95	2	0	21	3	26	3	0	0	0	0	0	0	0	0	2	
8:30:00	5	C	762	77	4	2	25	4	33	7	0	0	0	0	0	0	0	0	2	
8:45:00	5	C	841	79	5	1	27	2	36	3	0	0	0	0	0	0	0	0	2	
9:00:00	5	C	929	88	7	2	30	3	42	6	0	0	0	0	0	0	0	0	3	
9:02:20	5	C	929	0	7	0	30	0	42	0	0	0	0	0	0	0	0	0	3	
12:00:00	5	C	929	0	7	0	30	0	42	0	0	0	0	0	0	0	0	0	3	
12:15:00	5	C	1029	100	7	0	32	2	46	4	0	0	0	0	0	0	0	0	5	
12:30:00	5	C	1138	109	9	2	35	3	47	1	0	0	0	0	0	0	0	0	6	
12:45:00	5	C	1246	108	10	1	36	1	52	5	0	0	0	0	0	0	0	0	7	
13:00:00	5	C	1349	103	13	3	38	2	55	3	0	0	0	0	0	0	0	0	7	
13:15:00	5	C	1445	96	16	3	41	3	58	3	1	1	0	0	0	0	0	0	7	
13:30:00	5	C	1560	115	20	4	43	2	60	2	1	0	0	0	0	0	0	0	7	
13:45:00	5	C	1654	94	25	5	45	2	62	2	1	0	0	0	0	0	0	0	8	
14:00:00	5	C	1741	87	27	2	49	4	69	7	1	0	0	0	0	0	0	0	8	
14:01:38	5	C	1741	0	27	0	49	0	69	0	1	0	0	0	0	0	0	0	8	
15:00:00	5	C	1741	0	27	0	49	0	69	0	1	0	0	0	0	0	0	0	8	
15:15:00	5	C	1853	112	29	2	50	1	72	3	1	0	0	0	0	0	0	0	9	
15:30:00	5	C	1970	117	31	2	53	3	75	3	1	0	0	0	0	0	0	0	9	
15:45:00	5	C	2080	110	34	3	55	2	79	4	1	0	0	0	0	0	0	0	9	
16:00:00	5	C	2192	112	36	2	59	4	82	3	1	0	0	0	0	0	0	0	9	
16:15:00	5	C	2321	129	40	4	61	2	88	6	1	0	0	0	0	0	0	0	9	
16:30:00	5	C	2446	125	43	3	62	1	90	2	1	0	0	0	0	0	0	0	9	
16:45:00	5	C	2601	155	43	0	66	4	94	4	1	0	0	0	0	0	0	0	9	
17:00:00	5	C	2718	117	43	0	68	2	98	4	1	0	0	0	0	0	0	0	9	
17:15:00	6	1	2874	156	45	2	71	3	101	3	1	0	0	0	0	0	0	0	10	
17:30:00	6	C		139	45	0	73	2		4	1	0	0	0	0	0	0	0	10	
17:45:00	6	C		107	45	0	77	4	109	4	1	0	0	0		0	0	0	10	
18:00:00	6	C		108	46	1	79	2	113	4	1	0	0	0	0	0	0	0	10	
18:15:00	6	C		0	46	0	79	0	113	0	1	0	_	0	0	0	0	0	10	
18:16:07	6	C		0	46	0	79	0		0	1	0		0		0	0	0	10	

		Passen	ger Cars	- East Ap	proach			Tru	ıcks - Eas	t Approa	ch			Hea	vys - Eas	st Approa	ach		Pedes	trians
Interval	Let	ft	Th	ru	Rig	ht	Le	ft	Th	u	Rig	ght	Le	ft	Th	ru	Rig	lht	East C	ross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
6:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
6:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
6:30:00	1	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	C
6:45:00	2	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:00:00	2	0	0	0	1	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15:00	3	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1
7:30:00	3	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	1	C
7:45:00	3	0	0	0	1	0	0	0	0	0	0	0	-	0	0	0	0	0	2	1
8:00:00	3	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0
8:15:00	3	0		0	1	0	0	0	0	0	0	0	-	0	0	0	-	0	5	3
8:30:00	3	0	0	0	1	0	0	0	0	0	0	0	-	0	0	0	0	0	7	2
8:45:00	3	0	-	0	1	0	0	0	0	0	0	0		0	0	0	-	0	10	3
9:00:00	3	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	11	1
9:02:20	3	0		0	1	0		0		0	0	0	-	0	0	0	-	0	11	0
12:00:00	3	0	-	0	1	0	0	0		0	0	0	-	0	0	0	-	0	11	(
12:15:00	3	0	0	0	1	0	0	0	0	0	0	0	-	0	0	0		0	11	C
12:30:00	3	0	0	0	1	0	0	0		0	0	0		0	0	0	-	0	11	0
12:45:00	3	0		0	1	0	0	0	0	0	0	0	-	0	0	0	-	0	13	2
13:00:00	3	0	0	0	1	0	0	0	0	0	0	0	-	0	0	0	0	0	14	1
13:15:00	3	0	-	0	1	0	_	0		0	0	0	-	0	0	0	-	0	15	1
13:30:00	3	0	0	0	1	0	0	0	0	0	0	0	-	0	0	0	0	0	19	4
13:45:00	3	0	•	0	1	0	0	0		0	0	0	-	0	0	0	-	0	21	2
14:00:00	3	0	-	0	1	0	0	0		0	0	0	-	0	0	0	-	0	22	1
14:01:38	3	0	0	0	1	0	0	0	0	0	0	0	-	0	0	0	-	0	22	(
15:00:00	3	0	-	0	1	0		0		0	0	0		0	0	0	-	0	22	0
15:15:00	4	1	0	0	1	0	0	0	0	0	0	0	-	0	0	0	-	0	22	
15:30:00	5	1	0	0	1	0	0	0	0	0	0	0		0	0	0	0	0	23	1
15:45:00	5	0	0	0	1	0		0		0	0	0		0	0	0		0	24	1
16:00:00	5	0	0	0	1	0	0	0	0	0	0	0	-	0	0	0	_	0	24	(
16:15:00	5	0		0	1	0	-	0	-	0	0	0	-	0	0	0	-	0	28	4
16:30:00	5	0	•	0	1	0	0	0		0	0	0	-	0	0	0	-	0	31	3
16:45:00	5	0	0	0	1	0	0	0	0	0	0	0	-	0	0	0	-	0	32	1
17:00:00	5	0	-	0	1	0	_	0		0	0	0	-	0	0	0	-	0	34	2
17:15:00	5	0	-	0	1	0	0	0	0	0	0	0	-	0	0	0	-	0	34	0
17:30:00	5	0	-	0	1	0	0	0		0	0	0	-	0	0	0	-	0	34	C
17:45:00	5	0	-	0	4	3	0	0	0	0	0	0		0	0	0		0	37	3
18:00:00	5	0	0	0	6	2	0	0	0	0	0	0	-	0	0	0	-	0	37	C
18:15:00	5	0	0	0	6	0		0	0	0	0	0		0	0	0	0	0	37	C
18:16:07	5	0	0	0	6	0	0	0	0	0	0	0	0	0	0	0	0	0	37	0

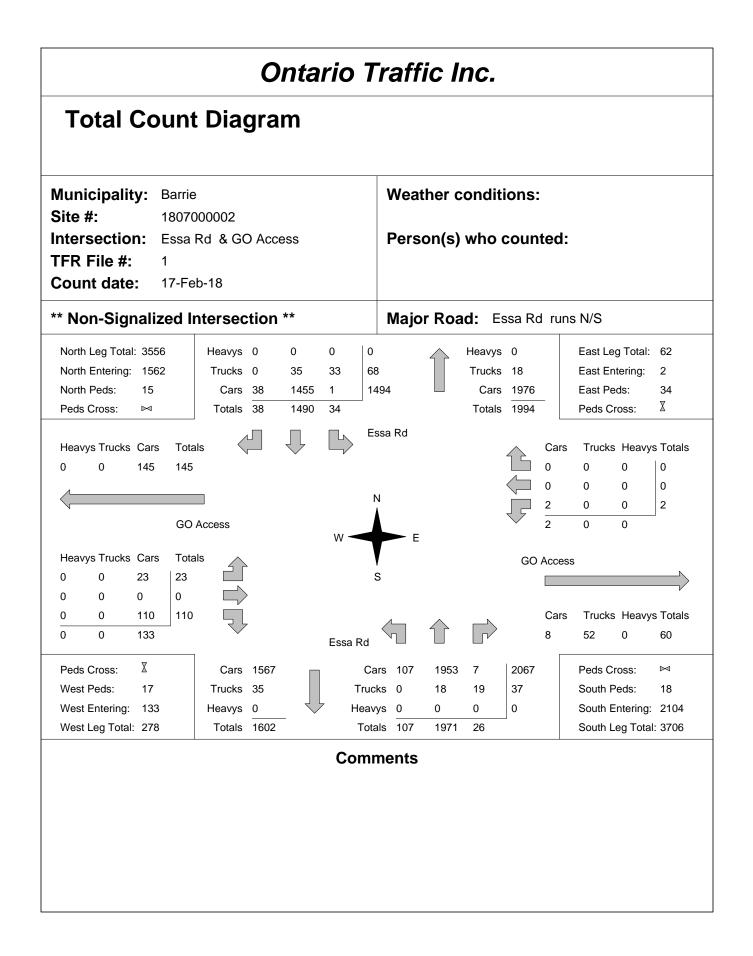
Count Date: 20-Feb-18 Site #: 1807000007

20-Feb-16 Sile #. 1807000007

		Passen	ger Cars -	South A	pproach			Tru	cks - Sou	th Appro	bach			Heav	vys - Sou	th Appro	ach		Pedes	trians
Interval	Le	eft	Thr	u	Rig	lht	Le	ft	Th	ru	Rig	jht	Le	ft	Th	ru	Rig	jht	South	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
6:00:00	0	C	0 0	0	0	0	0	0	0	0	0	C	0 0	0	0	0	0	0	0	0
6:15:00	0	C	) 14	14	1	1	0	0	3	3	1	1	0	0	0	0	0	0	1	1
6:30:00	0	C	) 39	25	1	0	0	0	8	5	1	C	0 0	0	0	0	0	0	6	5
6:45:00	1	1	78	39	1	0	0	0	10	2	3	2	2 0	0	0	0	0	0	7	1
7:00:00	1	C	118	40	1	0	0	0		2	3	C	0 0	0	0	0	0	0	7	0
7:15:00	4	3	3 173	55	1	0	1	1	13	1	5	2	-	0	0	0	0	0	9	2
7:30:00	5	1	231	58	1	0	1	0		3	-	1	0	0	0	0	0	0	9	0
7:45:00	6	1	305	74	1	0	1	0	-	2	7	1	0	0	0	0	0	0	10	1
8:00:00	7	1	409	104	1	0	1	0		3	8	1	0	0	0	0	0	0	11	1
8:15:00	8	1	519	110	2	1	1	0		1	9	1	· ·	0	0	0	0	0	11	0
8:30:00	11	3		95	3	1	1	0		2	10	1	·   ·	0	0	0	0	0	12	1
8:45:00	13	2		121	3	0	1	0		4	12	2	-	0	0	0	0	0	12	0
9:00:00	20	7	•••	136	3	0	2	1	31	3		C		0	0	0	0	0	12	0
9:02:20	20	C		0	3	0	2	0	-	0		C		0	0	0	0	0	12	0
12:00:00	20	C	0	0	3	0	2	0	• •	0		C		0	0	0	0	0	12	0
12:15:00	24	4		109	3	0	2	0	-	3		1	· ·	0	0	0	0	0	14	2
12:30:00	28	4		118	3	0	2	0		2		1	· · ·	0	0	0	0	0	16	2
12:45:00	31	3		103	3	0	2	0		4	16	2		0	0	0	0	0	16	0
13:00:00	41	10		123	3	0	2	0		3		1	·   ·	0	0	0	0	0	19	3
13:15:00	48	7		130	3	0	2	0		4	19	2		0	0	0	0	0	21	2
13:30:00	57	ç		97	3	0	2	0		3		C	-	0	0	0	0	0		0
13:45:00	65	8		112	3	0	2	0		4	21	2		0	0	0	0	0	22	1
14:00:00	72	7	1100	123	3	0	2	0	• •	3	22	1	· · ·	0	0	0	0	0	24	2
14:01:38	72	C		0	3	0	2	0		0		C	-	0	0	0	0	0	24	0
15:00:00	72	C		0	3	0	2	0	-	0		C	-	0	0	0	0	0	24	0
15:15:00	77	5	5 1924	138	3	0	2	0		2	24	2		0	0	0	0	0	25	1
15:30:00	85	8		132	4	1	2	0		3		C		0	0	0	0	0	26	1
15:45:00	89	4	2198	142	4	0	3	1	64	2	-	2		0	0	0	0	0	26	0
16:00:00	92	3	3 2342	144	4	0	3	0		2		1	·   ·	0	0	0	0	0	31	5
16:15:00	98	6	-	150	5	1	3	0		2		2		0	0	0	0	0	32	1
16:30:00	101	3	2636	144	5	0	3	0		2	-	C	-	0	0	0	0	0	32	0
16:45:00	107	6		147	6	1	3	0		2		2		0	0	0	0	0	32	0
17:00:00	113	6		169	6	0	3	0		1	32	1		0	0	0	0	0	32	0
17:15:00	115	2	3117	165	8	2	3	0		2	34	2		0	0	0	0	0	33	1
17:30:00	118	3	3278	161	8	0	3	0		1	34	C		0	0	0	0	0	34	1
17:45:00	120	2		173	8	0	3	0		1	35	1		0	0	0	0	0	37	3
18:00:00	123	3		120	8	0	3	0		0		2		0	0	0	0	0	38	1
18:15:00	123	C		0	8	0	3	0		0		C		0	0	0	0	0	38	0
18:16:07	123	C	3571	0	8	0	3	0	77	0	37	C	0 0	0	0	0	0	0	38	0

		Passen	ger Cars -	West Ap	oproach			Tru	cks - We	st Approa	ach			Hea	vys - Wes	st Approa	ach		Pedes	strians
Interval	Le	ft	Th	ru	Rig	ht	Le	ft	Th	ru	Rig	ht	Le	ft	Th	ru	Rig	ht	West	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
6:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
6:15:00	1	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
6:30:00	1	0	0	0	1	1	0	0	0	0	0	0	0	0	0	0	0	0	0	
6:45:00	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
7:00:00	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
7:15:00	2	1	0	0	3	2	0	0	0	0	0	0	0	0	0	0	0	0	0	
7:30:00	2	0	0	0	6	3	0	0	0	0	0	0	0	0	0	0	0	0	0	
7:45:00	3	1	0	0	8	2	0	0	0	0	0	0	0	0	0	0	0	0	0	
8:00:00	3	0	0	0	12	4	0	0	0	0	0	0	0	0	0	0	0	0	0	
8:15:00	6	3	0	0	14	2	0	0	0	0	0	0	0	0	0	0	0	0	1	
8:30:00	8	2	0	0	19	5	0	0	0	0	0	0	0	0	0	0	0	0	1	
8:45:00	11	3	0	0	23	4	0	0	0	0	0	0	0	0	0	0	0	0	1	
9:00:00	12	1	0	0	25	2	0	0	0	0	0	0	0	0	0	0	0	0	1	
9:02:20	12	0	0	0	25	0	0	0	0	0	0	0	0	0	0	0	0	0	1	
12:00:00	12	0	0	0	25	0	0	0	0	0	0	0	0	0	0	0	0	0	1	
12:15:00	14	2	0	0	30	5	0	0	0	0	0	0	0	0	0	0	0	0	2	
12:30:00	18	4	0	0	36	6	1	1	0	0	0	0	0	0	0	0	0	0	2	
12:45:00	19	1	0	0	43	7	1	0	0	0	0	0	0	0	0	0	0	0	5	
13:00:00	20	1	0	0	50	7	1	0	0	0	0	0	0	0	0	0	0	0	7	
13:15:00	23	3	0	0	56	6	1	0	0	0	0	0	0	0	0	0	0	0	7	
13:30:00	25	2	0	0	65	9	1	0	0	0	0	0	0	0	0	0	0	0	7	
13:45:00	25	0	0	0	65	0	1	0	0	0	0	0	0	0	0	0	0	0	7	
14:00:00	25	0	0	0	65	0	1	0	0	0	0	0	0	0	0	0	0	0	9	
14:01:38	25	0	0	0	65	0	1	0	0	0	0	0	0	0	0	0	0	0	9	
15:00:00	25	0	0	0	65	0	1	0	0	0	0	0	0	0	0	0	0	0	9	
15:15:00	28	3	0	0	70	5	1	0	0	0	0	0	0	0	0	0	0	0	9	
15:30:00	29	1	0	0	80	10	1	0	0	0	0	0	0	0	0	0	0	0	10	
15:45:00	30	1	0	0	88	8	1	0	0	0	0	0	0	0	0	0	0	0	11	
16:00:00	32	2	0	0	96	8	1	0	0	0	0	0	0	0	0	0	0	0	12	
16:15:00	33	1	0	0	100	4	1	0	0	0	0	0		0	0	0	0	0	14	
16:30:00	33	0	0	0	114	14	1	0	0	0	0	0	0	0	0	0	0	0	14	
16:45:00	33	0	0	0	123	9	1	0	0	0	0	0	0	0	0	0	0	0	15	
17:00:00	34	1	0	0	124	1	1	0	0	0	0	0	0	0	0	0	0	0	16	
17:15:00	35	1	0	0	133	9	1	0	0	0	0	0	-	0	0	0	0	0	17	
17:30:00	37	2	-	0	137	4	1	0	0	0	0	0	-	0	0	0	0	0	17	
17:45:00	37	0	0	0	139	2	1	0	0	0	0	0	-	0	0	0	0	0	17	
18:00:00	39	2	0	0	143	4	1	0	0	0	0	0	-	0	0	0	0	0	19	
18:15:00	39	0	0	0	143	0	1	0	0	0	0	0	-	0	0	0	0	0	19	
18:16:07	39	0		0	143	0	1	0	0	0	0	0		0	0	0	0	0	19	
		0			110	J			0	0	<b>J</b>	0			<b>U</b>		<b>J</b>	5	10	

Mid-day P	Peak Diagram	Specified Period           From:         10:00:00           To:         14:00:00	One Hour Peak           From:         12:45:00           To:         13:45:00
Site #:         18           Intersection:         Es           TFR File #:         1	arrie 307000002 ssa Rd & GO Access 7-Feb-18	Weather conditions Person(s) who cour	
** Non-Signalize	d Intersection **	Major Road: Essa R	d runs N/S
North Leg Total:980North Entering:388North Peds:4Peds Cross:⋈	Heavys         0         0         0           Trucks         0         10         9           Cars         12         357         0           Totals         12         367         9	0 Heavys 0 19 Trucks 3 369 Cars 589 Totals 592	East Leg Total: 15 East Entering: 0 East Peds: 8 Peds Cross: X
,	Totals	Essa Rd	Cars Trucks Heavys Totals
<			0 0 0 0 0 0 0 0
	GO Access	E	0 0 0
0 0 3	Totals 3 0	GC S	Access
0 0 33 0 0 36	33 Essa R		Cars Trucks Heavys Totals 1 14 0 15
Peds Cross:	Cars 390	Cars 27 586 1 614	Peds Cross: 🖂
West Peds: 1		ucks 0 3 5 8	South Peds: 6
West Entering: 36 West Leg Total: 75	·	avys 0 0 0 0 otals 27 589 6	South Entering: 622 South Leg Total: 1022
-	I	nments	-



				_		o <i>Trafi</i> ount S	-	-				
Intersection:	Essa Rd	& GO /		man		<sup>vate:</sup> 17-Feb-18		<sup>cipality:</sup> Ba	rrie			
	North	n Appro	ach Tot					Sout	h Appro	ach Tot		
Hour			rucks, & H	eavys Grand	Total	North/South Total	Hour			rucks, & H	Grand	Total
Ending 10:00:00 11:00:00 12:00:00	Left 0 9 9	Thru 0 356 383 250	Right 0 10 7	Total 0 375 399 276	Peds 0 5 1	817 858	11:00:00 12:00:00	Left 0 35 14 21	Thru 0 399 438	Right 0 8 7	Total 0 442 459	Peds 0 3 6
13:00:00	88	359 392	9 12	376 412	4 5		13:00:00 14:00:00	31 27	576 558	56	612 591	1 8
Totals:	34	1490	38 ach Tota	1562	15	3666		107	1971	26 ach Tota	2104	18
	Include	es Cars, T	rucks, & H	eavys		East/West		Include	es Cars, T	rucks, & H	eavys	
Hour Ending	Left	Thru	Right	Grand Total	Total Peds	Total Approaches	Hour Ending	Left	Thru	Right	Grand Total	Total Peds
10:00:00 11:00:00 12:00:00	0 2 0	0 0 0	0 0 0	0 2 0	0 8 7	28	10:00:00 11:00:00 12:00:00	0 7 7	0 0 0	0 27 21	0 34 28	0 5 6
13:00:00	00	000	00	00	11 8		13:00:00 14:00:00	4 5	000	33 29	37 34	42
Totals:	2	0	0	2	34	<u>135</u>		23	0	110	133	17
Hours En Crossing		0:00 0	0:00 0	0:00 0:00	10:00 0	or Traffic Cr	0551ng Ma 11:00 17	ajor Stre 12:00 14	13:00 9	14:00 18		

Count Date: 17-Feb-18 Site #: 180700002 Passenger Cars - North Approach Heavys - North Approach Trucks - North Approach Pedestrians Interval Left Thru Right Left Thru Right Left Thru Right North Cross Time Cum Cum Incr Incr 10:00:00 10:15:00 10:30:00 10:45:00 11:00:00 11:15:00 11:30:00 11:45:00 12:00:00 12:15:00 12:30:00 12:45:00 13:00:00 13:15:00 13:30:00 13:45:00 14:00:00 14:15:00 14:15:47 

	Date: 1		ger Cars		proach			Tru	ucks - Eas	st Approa	ach			Неа	avys - Eas	st Approa	ach		Pedes	trians
Interval	Left	t	Th	ru	Rig	Jht	Le	ft	Th	ru	Rig	ght	Le	ft	Th	ru	Rig	ht	East (	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
10:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:15:00	1	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	4	4
10:30:00	1	0		0	0	0	0	0		0		0	-	0		0		0	5	1
10:45:00	1	0	-	0	0	0	0	0		0		0		0		0		0	6	1
11:00:00	2	1	0	0	0	0	0	0		0		0	-	0		0		0	8	2 0
11:15:00	2	0	-	0	0	0	0	0		0		0		0		0		0	8	0
11:30:00	2	0	0	0	0	0	-	0		0		0		0		0		0	10	2 2 3
11:45:00	2	0	-	0	0	0	0	0		0		0		0		0		0	12	2
12:00:00	2	0	-	0	0	0		0		0		0	-	0		0		0	15	
12:15:00	2	0	-	0	0	0	0	0		0		0		0	0	0		0	16	1
12:30:00	2	0	0	0	0	0	0	0		0		0		0		0		0	16 25	0
12:45:00 13:00:00	2	0	-	0	0	0	0	0		0		0		0		0		0	25	9
13:00:00	2	0	-	0	0	0	-	0	-	0	_	0	-	0	-	0	_	0	30	4
13:30:00	2	0	0	0	0	0	-	0	-	0				0		0	-	0	31	4
13:45:00	2	0	0	0	0	0	0	0		0	-	0	-	0		0		0	33	2
14:00:00	2	0	-	0	0	0	0	0		0		0		0		0	_	0	34	1
14:15:00	2	0	-	0	0	0	0	0		0	-		-	0		0		0	34	0
14:15:47	2	0	0	0	0	0		0		0		0		0		0		0	34	0
	_																		0.	

Count Date: 17-Feb-18

Site #: 180700002

Passenger Cars - South Approach Heavys - South Approach Trucks - South Approach Pedestrians Interval Left Thru Right Left Thru Right Left Thru Right South Cross Time Cum Cum Incr Incr 10:00:00 10:15:00 10:30:00 10:45:00 11:00:00 11:15:00 11:30:00 11:45:00 12:00:00 12:15:00 12:30:00 12:45:00 13:00:00 13:15:00 13:30:00 13:45:00 14:00:00 14:15:00 14:15:47 

#### **Ontario Traffic Inc.**

Count Date: 17-Feb-18

Site #: 180700002

Heavys - West Approach **Passenger Cars - West Approach Trucks - West Approach** Pedestrians Interval Left Thru Right Left Thru Right Left Thru Right West Cross Time Cum Incr 10:00:00 10:15:00 10:30:00 10:45:00 11:00:00 11:15:00 11:30:00 11:45:00 12:00:00 12:15:00 12:30:00 12:45:00 13:00:00 13:15:00 13:30:00 13:45:00 14:00:00 14:15:00 14:15:47 

# APPENDIX

## **B** Signal Warrant Analysis

Input Da	ta She	eet		Analysis	Sheet	Results S	Sheet	Propose	d Collisio		) Justificati	on:	
What are the i	ntersecting	roadways?	E	ssa Rd. & G	O Access								-
What is the dir	ection of th	ne Main Roa	d street?	Nor	th-South	<b>T</b>	When was	the data co	llected?	2018-03-08			
Justificatio	n 1 - 4: \	/olume W	arrants										
a Number of	lanes on th	he Main Roa	ıd?	2 or more	. –								
b Number of	lanes on th	he Minor Roa	ad?	1	-								
c How many	approache	as? 4	<b>•</b>		_								
C HOW Many	approache	51 4											
d What is the	e operating	environmen	it?	Urban	•	Populati	ion >= 10,00	0 AND	Speed < 70	km/hr			
e What is the	e eight hou	ır vehicle volu	ume at the	intersection	? (Please	fill in table b	celow)						
u an Earling	Main No	orthbound A	pproach	Minor Ea	astbound A	pproach	Main Se	outhbound A	oproach	Minor W	estbound A	pproach	
Hour Ending	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	Pedestrians
7:00	1			1									Pedestrians Crossing Main Road
8:00	1	130	4	1	0	1	12	263	0	2	0	1	Crossing Main
	7	130 300	4 5	1	0	1 11	12 11	263 350	0	2	0 0	1 0	Crossing Main Road
9:00										2 1 0		·····	Crossing Main Road 7 4 4
9:00 13:00	7	300	5	2	0	11	11	350	2	1	0	0	Crossing Main Road 7 4
	7 14	300 472	5	2 9	0	11 13	11 12	350 358	2 5	1 0	0	0	Crossing Main Road 7 4 4
13:00	7 14 21	300 472 465	5 6 5	2 9 9	0 0 0 0	11 13 25	11 12 8	350 358 433	2 5 6		0 0 0	0 0 0	Crossing Main Road 7 4 4 11

3,341

18:00

Total

3,648

Analys	is Sheet		<u>I</u> np	ut Sheet	Resu	ilts Sheet	Pro	oposed Co	llision		GO TO Jus	stificatior	1:
ersection:	ection: Essa Rd. & GO Access Count Date: 2018-03-08												
	Flow Urban C	num Vehic onditions	ie volu	mes									
	Flow Urban Co			mes			Percentage	Warrant				Trees	<b>D</b> e atlian
	Flow Urban Co	onditions	ies	mes		1	Percentage Hour En					-	Section Percent

Image: Constraint of the system         Image: Consystem         Image: Constraint of the syst	
1A         480         720         600         900         415         689         889         972         964         1,115         1,233         1,209           COMPLIANCE *         46         77         99         100         100         100         100         100         72           1B         120         170         120         170         5         14         22         34         20         40         30         29	
1A         480         720         600         900         415         689         889         972         964         1,115         1,233         1,209           COMPLIANCE %         46         77         99         100         100         100         100         100         100         72           120         170         120         170         5         14         22         34         20         40         30         29	14
1A         480         720         600         900         415         689         889         972         964         1,115         1,233         1,209	
480 720 600 900 415 689 889 972 964 1,115 1,233 1,209	90

#### Justification 2: Delay to Cross Traffic

#### **Restricted Flow Urban Conditions**

lustification	Guidance Approach Lanes					Percentage Warrant								
Justineation	1 la	nes	2 or Mo	re lanes		Hour Ending								Percent
Flow Condition	FREE FLOW	RESTR. FLOW	FREE FLOW	RESTR. FLOW	7:00	8:00	9:00	13:00	14:00	16:00	17:00	18:00		
2A	480	720	600	900	410	675	867	938	944	1,075	1,203	1,180		
24		COMPLI	ANCE %		46	75	96	100	100	100	100	100	717	90
2B	50	75	50	75	10	7	13	20	11	17	3	12		
20		COMPLI	ANCE %		13	9	17	27	15	23	4	16	124	16
	Restricted Flow				Both 2A and 2	Both 2A and 2B 100% fulfilled each of 8 hours Yes N								
											V			

#### **Justification 3: Combination**

Combination Justification 1 and 2

		Justification Satisfied 80% or M		TwoJust Satisfied 80	ifications 0% or More			
Justifica 1	tion	Minimum Vehicle Volume	YES 🗆	NO 🗹	YES		NO	<b>V</b>
Justifica 2	tion	Delay Cross Traffic	YES 🗆	NO 🗹			NOT JU	stified

#### **Justification 4: Four Hour Volume**

Justification	Time Period	Total Volume of Both Approaches (Main) X	Heaviest Minor Approach Y (actual)	Required Value Y (warrant threshold)	Average % Compliance	Overall % Compliance
	14:00	944	20	215	9 %	
Justification	16:00	1,075	38	170	22 %	10.01
4	17:00	1,203	30	137	22 %	18 %
	18:00	1,180	24	142	17 %	

Analysi	is Sheet	put Sheet Resu	Ilts Sheet Pro	oposed Collision	GO TO Jus	tification:			
ntersection:	Essa Rd. & GO Access		Count Date: 2018-03	3-08					
Justification 6: Pedestrian Volume									
Pedestrian	Volume Analysis								
	8 Hour Vehicular		Net8H	our Pedestrian Volume	9				
	Volume V <sub>8</sub>	< 200	200 - 275	276 - 475	476 - 1000	>1000			
	< 1440								
Justification	1440 - 2600								
6A	2601 - 7000	Not Justified							
	> 7000								
	Delay Analysis Net Total 8 Hour Volume	Net Total 8 Hou	ur Volume of Delayed	Pedestrians	]				
	of Total Pedestrians	< 75	75 - 130	> 130					
	< 200	Not Justified							
ustification 6B	200 - 300								
	> 300								

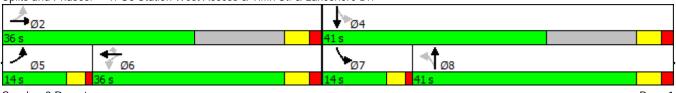
Results	Sheet	Input Sheet Analysis	Sheet	Propo	sed Collision	GO TO Justificat	ion:
Intersection: E	ssa Rd. & GO Access	Count Dat	e: 2018-03-	08			
Summary R	Results		3	1	1	1	
J	ustification	Compliance		ustified?	]		
			YES	NO			
1. Minimum Vehicular	A Total Volume	90 %		~			
Volume	B Crossing Volume	14 %					
2. Delay to Cross	A Main Road	90 %		~			
Traffic	B Crossing Road	16 %					
3. Combination	A Justificaton 1	14 %		~			
	B Justification 2	16 %					
4. 4-Hr Volume	,	18 %		~			
			8			1 8	1 1
5. Collision Expe	erience	0 %		V			
}	3	1 1	3	3			
6. Pedestrians	A Volume	Justification not met		~			
	B Delay	Justification not met		<b>N</b>			

# APPENDIX

## C Existing (2018) Synchro Reports

Existing AM

Lane Group         EBL         EBT         EBT         EBT         WBT         WBT         WBT         NBT         NBT         NBT         SBL         SBT         SBR           Lane Configurations         •		٦	-	$\mathbf{F}$	•	+	•	1	Ť	1	1	ţ	~
Traffic Volume (vph)       94       220       5       1       294       644       16       0       2       402       1       93         Future Volume (vph)       94       220       5       1       294       644       16       0       2       402       1       93         Stati. Flow (prot)       1787       1894       0       1615       0       977       1570       0         Stati. Flow (prot)       1787       1894       0       1161       3574       1599       1056       1615       0       967       102         Lane Group Flow (ph)       99       237       0       1       300       657       25       3       0       442       103       0         Tum Type       pm-ptl<       NA       Perm<       NA       Perm       NA       perm       Part       NA       Permited Phases       2       6       6       8       4       104       10       140       410       140       410       140       410       140       410       160       105       01       115       024       1050       117       154       4       4       104       163       160	Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Volume (vph)       94       220       5       1       294       644       16       0       2       402       1       93         Future Volume (vph)       94       220       5       1       294       644       16       0       2       402       1       93         Stati. Flow (prot)       1787       1894       0       1161       3574       1599       1003       1615       0       9770       1570       0         Stati. Flow (perm)       948       1894       0       1161       3574       1599       1056       1615       0       947       1570       0         Stati. Flow (perm)       94       1894       0       1161       3574       1599       1056       1615       0       442       103       0         Tum Type       pm+tl<		ሻ	4Î		۲	<b>^</b>	1	۲	f,		٦	4Î	
Satel. Flow (pron)       1787       1894       0       1805       3574       1599       1003       1615       0       1770       1570       0         FIL Permitted       0.504       0.611       5574       1599       1056       1615       0       967       1570       0         Satel. Flow (perm)       948       1944       0       1161       3574       1599       1056       1615       0       967       1570       0         Satel. Flow (perm)       948       1894       0       1161       3574       1599       1056       1615       0       967       1570       0         Satel. Flow (perm)       948       1894       0       1161       3574       1599       1056       1615       0       677       1570       0         Sate. Flow (perm)       99       237       0       1       300       657       25       3       0       442       103       0         Permitted Phases       2       6       6       6       8       4       104       104       104       104       104       104       104       104       104       15       116       162       116	Traffic Volume (vph)	94		5			644			2	402		93
FIF Permitted       0.501       0.511       0.519         Satd. Flow (perm)       948       1894       0       1161       3574       1599       1056       1615       0       967       1570       0         Lane Group Flow (uph)       99       237       0       1       300       657       25       3       0       442       103       0         Turn Type       pm+pt       NA       Perm       NA       Perm       NA       pm+pt       NA         Portleted Phases       2       6       6       8       7       4         Portletic (ls)       140       36.0       36.0       36.0       41.0       11.0       14.0       41.0         Total Lost Time (s)       4.0       6.0       6.0       6.0       6.0       6.0       4.0       6.0         Act Latel Green (s)       42.3       40.3       30.9       30.9       10.7       10.7       17.4       15.4         Act Latel Green (s)       0.42       0.0       0.0       0.0       0.0       0.0       0.0       0.0       0.0       0.0       0.0       0.0       0.0       0.0       0.0       0.0       0.0       0.0	Future Volume (vph)	94	220	5	1	294	644	16	0	2	402	1	93
Said. Flow (perm)       948       1894       0       1161       3574       1599       1056       1615       0       967       1570       0         Said. Flow (RTOR)       1       657       657       3       0       442       103       0         Lane Group Prove (rph)       99       237       0       1       300       657       25       3       0       442       103       0         Turn Type       pm+pt       NA       Perm       NA       Perm       NA       pm+pt       NA         Prolected Phases       5       2       6       6       8       4       4         Total Lost Time (s)       4.0       6.0       6.0       6.0       6.0       4.0       6.0         Actuated g/C Ratio       0.62       0.59       0.45       0.45       0.45       0.16       0.16       0.26       0.23         Vic Ratio       0.14       0.21       0.00       0.18       0.10       1.15       0.24       15       0.17       1.07       1.74       1.54         Control Delay       7.2       8.3       16.0       14.1       4.5       31.7       29.5       119.1       6.4 <td>Satd. Flow (prot)</td> <td>1787</td> <td>1894</td> <td>0</td> <td>1805</td> <td>3574</td> <td>1599</td> <td>1003</td> <td>1615</td> <td>0</td> <td>1770</td> <td>1570</td> <td>0</td>	Satd. Flow (prot)	1787	1894	0	1805	3574	1599	1003	1615	0	1770	1570	0
Satd. Flow (RTOR)       1       1       657       102         Lane Group Flow (vph)       99       237       0       1       300       657       25       3       0       442       103       0         Turn Type       pm-pt       NA       Perm       NA       Perm       NA       Perm       NA       pm-pt       NA         Protected Phases       2       6       6       8       7       4         Permitted Phases       2       6       6       8       7       4         Permitted Phases       2       6       6       8       4       7         Total Spitt(S)       14.0       36.0       36.0       36.0       40.0       6.0       40.0       6.0         Act Effct Green (S)       42.3       40.3       30.9       30.9       15.7       7.9       17.4       15.4         Actuated gC Ratio       0.62       0.59       0.45       0.45       0.45       0.10       11.5       0.24         Control Delay       7.2       8.3       16.0       14.1       4.5       31.7       29.5       119.1       6.4         Queue Delay       7.2       8.3       1	Flt Permitted	0.504			0.611						0.519		
Lane Group Flow (vph)       99       237       0       1       300       657       25       3       0       442       103       0         Turn Type       pm-pt       NA       Perm       NA       Perm       Perm       NA       pm-pt       NA         Protected Phases       5       2       6       6       8       7       4         Permited Phases       2       6       6       8       4       4         Total Split (s)       14.0       36.0       36.0       36.0       41.0       14.0       41.0       10.1         Total Lost Time (s)       4.0       6.0       10.7       17.4       15.4       7.2       8.3       16.0       14.1       4.5       31.7       29.5       119.1       6.4       10.24       10.24       10.5       0.0       0.0       0.0		948	1894	0	1161	3574	1599	1056	1615	0	967	1570	0
Turn Type         pm+pt         NA         Perm         NA         Perm         NA         pm+pt         NA           Protected Phases         2         6         6         8         7         4           Permitted Phases         2         6         6         8         4         4           Total Split (s)         14.0         36.0         36.0         36.0         41.0         41.0         14.0         41.0           Total Split (s)         4.0         6.0         6.0         6.0         6.0         40.0         60.0         40.0         60.0         40.0         60.0         40.0         60.0         40.0         60.0         40.0         60.0         40.0         60.0         40.0         60.0         40.0         60.0         60.0         40.0         60.0         60.0         40.0         60.0 </td <td>Satd. Flow (RTOR)</td> <td></td> <td>•</td> <td></td>	Satd. Flow (RTOR)		•										
Protected Phases       5       2       6       8       7       4         Permitted Phases       2       6       6       8       4         Total Split (s)       14.0       36.0       36.0       36.0       41.0       41.0       41.0       41.0         Total Lost Time (s)       4.0       6.0       6.0       6.0       6.0       6.0       6.0       4.0       6.0         Act Eft Green (s)       42.3       40.3       30.9       30.9       10.7       10.7       17.4       15.4         Actuated gC Ratio       0.62       0.59       0.45       0.45       0.45       0.16       0.16       0.26       0.23         wic Ratio       0.14       0.21       0.00       0.18       0.61       0.15       0.01       1.15       0.24         Control Delay       7.2       8.3       16.0       14.1       4.5       31.7       29.5       119.1       6.4         LOS       A       A       B       B       A       C       F       A         Approach LOS       A       A       B       B       C       7.0       -72.5       0.1         Oueue Length 50th (m)		99		0	•		657	25		0	442		0
Permitted Phases         2         6         6         8         4           Total Split (s)         14.0         36.0         36.0         36.0         41.0         40.0         6.0         6.0         6.0         6.0         6.0         40.0         6.0         40.0         6.0         40.0         6.0         40.0         6.0         40.0         6.0         40.0         6.0         40.0         6.0         40.0         6.0         6.0         40.0         6.0         6.0         40.0         6.0         6.0         40.0         6.0         6.0         40.0         6.0		pm+pt			Perm		Perm	Perm			pm+pt		
Total Split (s)       14.0       36.0       36.0       36.0       36.0       41.0       41.0       14.0       41.0         Total Lost Time (s)       4.0       6.0       6.0       6.0       6.0       6.0       6.0       40.0       6.0         Act Effed Green (s)       42.3       40.3       30.9       30.9       30.9       10.7       10.7       17.4       15.4         Actuated g/C Ratio       0.62       0.59       0.45       0.45       0.16       0.16       0.26       0.23         w/c Ratio       0.14       0.21       0.00       0.18       0.61       0.15       0.01       1.15       0.24         Control Delay       7.2       8.3       16.0       14.1       4.5       31.7       29.5       119.1       6.4         LOS       A       A       B       B       C       C       F       A         Approach LOS       A       A       B       B       C       C       F       C         Queue Length 50th (m)       3.3       9.6       0.1       10.5       0.0       2.7       0.4       -72.5       0.1         Queue Length 50th (m)       3.5.0       23.0       <			2			6			8		•	4	
Total Lost Time (s)       4.0       6.0       6.0       6.0       6.0       6.0       4.0       6.0         Act Effic Green (s)       42.3       40.3       30.9       30.9       10.7       10.7       17.4       15.4         Actuated g/C Ratio       0.62       0.59       0.45       0.45       0.16       0.16       0.26       0.23         v/c Ratio       0.14       0.21       0.00       0.18       0.61       0.15       0.01       1.15       0.24         Control Delay       7.2       8.3       16.0       14.1       4.5       31.7       29.5       119.1       6.4         Ueue Delay       7.2       8.3       16.0       14.1       4.5       31.7       29.5       119.1       6.4         LOS       A       A       B       B       A       C       C       F       A         Approach Delay       7.9       7.5       31.5       97.8       A       Approach LOS       A       A       C       F       C       0.1       0.0       0.2.7       0.4       -72.5       0.1         Oueue Length 50th (m)       3.3       9.6       0.1       10.5       0.0       2.0													
Act Effct Green (s)       42.3       40.3       30.9       30.9       30.9       10.7       10.7       17.4       15.4         Actuated g/C Ratio       0.62       0.59       0.45       0.45       0.16       0.16       0.26       0.23         v/c Ratio       0.14       0.21       0.00       0.18       0.61       0.15       0.01       1.15       0.24         Control Delay       7.2       8.3       16.0       14.1       4.5       31.7       29.5       119.1       6.4         Queue Delay       0.0													
Actuated g/C Ratio       0.62       0.59       0.45       0.45       0.45       0.16       0.16       0.26       0.23         wic Ratio       0.14       0.21       0.00       0.18       0.61       0.15       0.01       1.15       0.24         Control Delay       7.2       8.3       16.0       14.1       4.5       31.7       29.5       119.1       6.4         Queue Delay       0.0													
v/c Ratio       0.14       0.21       0.00       0.18       0.61       0.15       0.01       1.15       0.24         Control Delay       7.2       8.3       16.0       14.1       4.5       31.7       29.5       119.1       6.4         Queue Delay       0.0       0       0       0.0<	• •												
Control Delay       7.2       8.3       16.0       14.1       4.5       31.7       29.5       119.1       6.4         Queue Delay       0.0													
Queue Delay         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0           Total Delay         7.2         8.3         16.0         14.1         4.5         31.7         29.5         119.1         6.4           LOS         A         A         B         B         A         C         C         F         A           Approach Delay         7.9         7.5         31.5         97.8         Approach LOS         A         A         C         F         C         Delay         10.6         0.0         2.7         0.4         -72.5         0.1         Queue Length 95th (m)         14.0         32.2         1.2         27.1         22.0         7.6         2.0         #135.4         10.6           Internal Link Dist (m)         35.0         23.0         55.0         75.0         T         Base Capacity (vph)         716         1261         526         1622         1084         559         855         386         1190           Starvation Cap Reductn         0         0         0         0         0         0         0         0         0         0         0         0         <													
Total Delay       7.2       8.3       16.0       14.1       4.5       31.7       29.5       119.1       6.4         LOS       A       A       B       B       A       C       C       F       A         Approach Delay       7.9       7.5       31.5       97.8         Approach LOS       A       A       C       F         Oueue Length 50th (m)       3.3       9.6       0.1       10.5       0.0       2.7       0.4       -72.5       0.1         Oueue Length 95th (m)       14.0       32.2       1.2       27.1       22.0       7.6       2.0       #135.4       10.6         Internal Link Dist (m)       44.8       116.0       88.9       104.9         Turn Bay Length (m)       35.0       23.0       55.0       75.0       Base Capacity (vph)       716       1261       526       1622       1084       559       855       386       1190         Starvation Cap Reductn       0       0       0       0       0       0       0       0       0         Starvation Cap Reductn       0       0       0       0       0       0       0       0       0       0													
LOS       A       A       B       B       B       A       C       C       F       A         Approach LOS       A       A       A       C       F       A         Queue Length 50th (m)       3.3       9.6       0.1       10.5       0.0       2.7       0.4       ~72.5       0.1         Queue Length 95th (m)       14.0       32.2       1.2       27.1       22.0       7.6       2.0       #135.4       10.6         Internal Link Dist (m)       44.8       116.0       88.9       104.9       104.9         Turn Bay Length (m)       35.0       23.0       55.0       75.0       855       386       1190         Starvation Cap Reductn       0 <td></td>													
Approach Delay       7.9       7.5       31.5       97.8         Approach LOS       A       A       C       F         Queue Length 50th (m)       3.3       9.6       0.1       10.5       0.0       2.7       0.4      72.5       0.1         Queue Length 95th (m)       14.0       32.2       1.2       27.1       22.0       7.6       2.0       #135.4       10.6         Internal Link Dist (m)       44.8       116.0       88.9       104.9         Turn Bay Length (m)       35.0       23.0       55.0       75.0         Base Capacity (vph)       716       1261       526       1622       1084       559       855       386       1190         Starvation Cap Reductn       0       0       0       0       0       0       0       0         Storage Cap Reductn       0       15       0.61       1.15       0.09       1.15       0.09       1.15       <													
Approach LOS       A       A       C       F         Queue Length 50th (m)       3.3       9.6       0.1       10.5       0.0       2.7       0.4      72.5       0.1         Queue Length 50th (m)       14.0       32.2       1.2       27.1       22.0       7.6       2.0       #135.4       10.6         Internal Link Dist (m)       44.8       116.0       88.9       104.9         Turn Bay Length (m)       35.0       23.0       55.0       75.0       Base Capacity (vph)       716       1261       526       1622       1084       559       855       386       1190         Starvation Cap Reductn       0		А			В		А	С			F		
Dueue Length 50th (m)       3.3       9.6       0.1       10.5       0.0       2.7       0.4       -72.5       0.1         Queue Length 95th (m)       14.0       32.2       1.2       27.1       22.0       7.6       2.0       #135.4       10.6         Internal Link Dist (m)       44.8       116.0       88.9       104.9         Turn Bay Length (m)       35.0       23.0       55.0       75.0         Base Capacity (vph)       716       1261       526       1622       1084       559       855       386       1190         Starvation Cap Reductn       0       0       0       0       0       0       0       0       0         Starvation Cap Reductn       0       0       0       0       0       0       0       0       0         Starvation Cap Reductn       0       115       115       115       115       116<													
Queue Length 95th (m)       14.0       32.2       1.2       27.1       22.0       7.6       2.0       #135.4       10.6         Internal Link Dist (m)       44.8       116.0       88.9       104.9         Turn Bay Length (m)       35.0       23.0       55.0       75.0         Base Capacity (vph)       716       1261       526       1622       1084       559       855       386       1190         Starvation Cap Reductn       0       0       0       0       0       0       0       0       0         Spillback Cap Reductn       0													
Internal Link Dist (m)       44.8       116.0       88.9       104.9         Turn Bay Length (m)       35.0       23.0       55.0       75.0         Base Capacity (vph)       716       1261       526       1622       1084       559       855       386       1190         Starvation Cap Reductn       0       0       0       0       0       0       0       0       0         Starvation Cap Reductn       0       0       0       0       0       0       0       0       0         Starvation Cap Reductn       0													
Turn Bay Length (m)       35.0       23.0       55.0       75.0         Base Capacity (vph)       716       1261       526       1622       1084       559       855       386       1190         Starvation Cap Reductn       0       0       0       0       0       0       0       0       0       0         Spillback Cap Reductn       0       16       15       5       5       3       16		14.0			1.2		22.0	7.6			#135.4		
Base Capacity (vph)       716       1261       526       1622       1084       559       855       386       1190         Starvation Cap Reductn       0       0       0       0       0       0       0       0       0         Spillback Cap Reductn       0       0       0       0       0       0       0       0       0         Storage Cap Reductn       0       0       0       0       0       0       0       0       0       0         Storage Cap Reductn       0 </td <td>, , ,</td> <td></td> <td>44.8</td> <td></td> <td></td> <td>116.0</td> <td></td> <td></td> <td>88.9</td> <td></td> <td></td> <td>104.9</td> <td></td>	, , ,		44.8			116.0			88.9			104.9	
Starvation Cap Reductn       0 <td></td> <td></td> <td>10/1</td> <td></td> <td></td> <td>1 ( 0 0</td> <td></td> <td></td> <td>055</td> <td></td> <td></td> <td>4400</td> <td></td>			10/1			1 ( 0 0			055			4400	
Spillback Cap Reductn         0													
Storage Cap Reductin000000000Reduced v/c Ratio0.140.190.000.180.610.040.001.150.09Intersection SummaryCycle Length:105Actuated Cycle Length:68Control Type:Semi Act-UncoordMaximum v/c Ratio:1.15Intersection Signal Delay:34.3Intersection LOS:CIntersection Capacity Utilization93.9%ICU Level of Service FAnalysis Period (min)-Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles.#95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.													
Reduced v/c Ratio0.140.190.000.180.610.040.001.150.09Intersection SummaryCycle Length: 105Actuated Cycle Length: 68Control Type: Semi Act-UncoordMaximum v/c Ratio: 1.15Intersection Signal Delay: 34.3Intersection LOS: CIntersection Capacity Utilization 93.9%ICU Level of Service FAnalysis Period (min) 15- Volume exceeds capacity, queue is theoretically infinite.Queue shown is maximum after two cycles.# 95th percentile volume exceeds capacity, queue may be longer.Queue shown is maximum after two cycles.													
Intersection Summary         Cycle Length: 105         Actuated Cycle Length: 68         Control Type: Semi Act-Uncoord         Maximum v/c Ratio: 1.15         Intersection Signal Delay: 34.3         Intersection Capacity Utilization 93.9%         ICU Level of Service F         Analysis Period (min) 15         -         Volume exceeds capacity, queue is theoretically infinite.         Queue shown is maximum after two cycles.         #       95th percentile volume exceeds capacity, queue may be longer.         Queue shown is maximum after two cycles.													
Cycle Length: 105 Actuated Cycle Length: 68 Control Type: Semi Act-Uncoord Maximum v/c Ratio: 1.15 Intersection Signal Delay: 34.3 Intersection LOS: C Intersection Capacity Utilization 93.9% ICU Level of Service F Analysis Period (min) 15 ~ Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles. # 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.	Reduced V/C Rallo	0.14	0.19		0.00	0.18	0.61	0.04	0.00		1.15	0.09	
Actuated Cycle Length: 68 Control Type: Semi Act-Uncoord Maximum v/c Ratio: 1.15 Intersection Signal Delay: 34.3 Intersection LOS: C Intersection Capacity Utilization 93.9% ICU Level of Service F Analysis Period (min) 15 ~ Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles. # 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.	Intersection Summary												
Control Type: Semi Act-Uncoord Maximum v/c Ratio: 1.15 Intersection Signal Delay: 34.3 Intersection LOS: C Intersection Capacity Utilization 93.9% ICU Level of Service F Analysis Period (min) 15 ~ Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles. # 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.	Cycle Length: 105												
Maximum v/c Ratio: 1.15         Intersection Signal Delay: 34.3       Intersection LOS: C         Intersection Capacity Utilization 93.9%       ICU Level of Service F         Analysis Period (min) 15       -         ~ Volume exceeds capacity, queue is theoretically infinite.	Actuated Cycle Length: 68												
Intersection Signal Delay: 34.3       Intersection LOS: C         Intersection Capacity Utilization 93.9%       ICU Level of Service F         Analysis Period (min) 15       Volume exceeds capacity, queue is theoretically infinite.         Queue shown is maximum after two cycles.       95th percentile volume exceeds capacity, queue may be longer.         Queue shown is maximum after two cycles.       Undersection Capacity (maximum after two cycles)	Control Type: Semi Act-Un	coord											
Intersection Capacity Utilization 93.9%       ICU Level of Service F         Analysis Period (min) 15       -         Volume exceeds capacity, queue is theoretically infinite.       -         Queue shown is maximum after two cycles.       -         # 95th percentile volume exceeds capacity, queue may be longer.       -         Queue shown is maximum after two cycles.       -	Maximum v/c Ratio: 1.15												
<ul> <li>Analysis Period (min) 15</li> <li>Volume exceeds capacity, queue is theoretically infinite.</li> <li>Queue shown is maximum after two cycles.</li> <li># 95th percentile volume exceeds capacity, queue may be longer.</li> <li>Queue shown is maximum after two cycles.</li> </ul>	Intersection Signal Delay: 3	34.3			Ir	ntersection	n LOS: C						
<ul> <li>Volume exceeds capacity, queue is theoretically infinite.</li> <li>Queue shown is maximum after two cycles.</li> <li># 95th percentile volume exceeds capacity, queue may be longer.</li> <li>Queue shown is maximum after two cycles.</li> </ul>	Intersection Capacity Utiliza	ation 93.9%			IC	CU Level	of Service	e F					
Queue shown is maximum after two cycles.         #       95th percentile volume exceeds capacity, queue may be longer.         Queue shown is maximum after two cycles.													
<ul> <li>95th percentile volume exceeds capacity, queue may be longer.</li> <li>Queue shown is maximum after two cycles.</li> </ul>		<b>J</b> 1		ally infini:	te.								
Queue shown is maximum after two cycles.			,										
				eue may	be longe	r.							
Splits and Phases: 1: Go Station West Access & Tiffin St. & Lakeshore Dr.	Queue shown is maximu	um after two	o cycles.										
	Splits and Phases: 1: Go	Station We	est Access	s & Tiffin	St. & Lak	eshore Di	r.						



Synchro 9 Report

#### 2: Essa Rd./Bradford St. & Tiffin St.

Existing AM

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	٦	4Î		٦	<b>↑</b>			4î>				1
Traffic Volume (vph)	60	197	67	93	187	0	56	324	103	33	215	35
Future Volume (vph)	60	197	67	93	187	0	56	324	103	33	215	35
Satd. Flow (prot)	1770	1770	0	1641	1845	0	0	3397	0	0	3352	1568
Flt Permitted	0.628			0.581				0.858			0.815	
Satd. Flow (perm)	1170	1770	0	1004	1845	0	0	2932	0	0	2751	1568
Satd. Flow (RTOR)		20						42				44
Lane Group Flow (vph)	67	293	0	103	208	0	0	568	0	0	314	44
Turn Type	Perm	NA		Perm	NA		Perm	NA		pm+pt	NA	Perm
Protected Phases		4			8			2		1	6	
Permitted Phases	4			8			2			6	6	6
Total Split (s)	31.0	31.0		31.0	31.0		34.0	34.0		21.0	34.0	34.0
Total Lost Time (s)	6.0	6.0		6.0	6.0			6.0			6.0	6.0
Act Effct Green (s)	25.1	25.1		25.1	25.1			14.4			14.4	14.4
Actuated g/C Ratio	0.49	0.49		0.49	0.49			0.28			0.28	0.28
v/c Ratio	0.12	0.34		0.21	0.23			0.67			0.41	0.09
Control Delay	9.0	9.7		10.1	9.4			19.3			16.6	5.5
Queue Delay	0.0	0.0		0.0	0.0			0.0			0.0	0.0
Total Delay	9.0	9.7		10.1	9.4			19.3			16.6	5.5
LOS	А	А		В	А			В			В	А
Approach Delay		9.5			9.6			19.3			15.2	
Approach LOS		А			А			В			В	
Queue Length 50th (m)	3.2	14.5		5.1	10.5			23.3			12.8	0.0
Queue Length 95th (m)	10.3	34.2		15.3	25.2			34.0			18.5	4.2
Internal Link Dist (m)		153.0			41.6			55.6			43.8	
Turn Bay Length (m)	47.0			23.0								89.0
Base Capacity (vph)	569	871		488	897			1616			2606	1487
Starvation Cap Reductn	0	0		0	0			0			0	0
Spillback Cap Reductn	0	0		0	0			0			0	0
Storage Cap Reductn	0	0		0	0			0			0	0
Reduced v/c Ratio	0.12	0.34		0.21	0.23			0.35			0.12	0.03
Intersection Summary												
Cycle Length: 86												
Actuated Cycle Length: 51.6	, )											
Control Type: Semi Act-Unc	oord											
Maximum v/c Ratio: 0.67												
Intersection Signal Delay: 14					tersectior							
Intersection Capacity Utiliza	tion 82.2%			IC	CU Level	of Service	E					
Analysis Period (min) 15												

Splits and Phases: 2: Essa Rd./Bradford St. & Tiffin St.

Ø1	<b>▲</b> ¶ <sub>Ø2</sub>	<u></u> 04
21 s	34 s	31 s
∳ ø <sub>6</sub>		✓ Ø8
34 s		31 s

	-	$\mathbf{r}$	1	-	1	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	Ø4
Lane Configurations	1	1	ሻ	<b>†</b>	۲	1	
Traffic Volume (vph)	623	1	3	936	3	2	
Future Volume (vph)	623	1	3	936	3	2	
Satd. Flow (prot)	1881	1615	1805	1881	1805	1615	
Flt Permitted			0.345		0.950		
Satd. Flow (perm)	1881	1615	656	1881	1805	1615	
Satd. Flow (RTOR)						6	
Lane Group Flow (vph)	677	1	3	975	10	6	
Turn Type	NA	Perm	pm+pt	NA	Prot	Perm	
Protected Phases	2		1	6	8		4
Permitted Phases		2	6			8	
Total Split (s)	50.0	50.0	38.0	50.0	41.0	41.0	25.0
Total Lost Time (s)	6.0	6.0	4.0	6.0	5.0	5.0	
Act Effct Green (s)	61.5	61.5	60.5	63.5	10.0	10.0	
Actuated g/C Ratio	0.92	0.92	0.90	0.94	0.15	0.15	
v/c Ratio	0.39	0.00	0.00	0.55	0.04	0.02	
Control Delay	4.2	4.0	1.3	3.5	27.0	17.5	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	4.2	4.0	1.3	3.5	27.0	17.5	
LOS	А	А	A	A	С	В	
Approach Delay	4.2			3.5	23.4		
Approach LOS	А			А	С		
Queue Length 50th (m)	0.0	0.0	0.0	0.0	1.1	0.0	
Queue Length 95th (m)	94.6	0.7	0.6	109.9	1.9	0.7	
Internal Link Dist (m)	144.3			137.6	30.3		
Turn Bay Length (m)		40.0	70.0				
Base Capacity (vph)	1723	1479	1174	1881	970	871	
Starvation Cap Reductn	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	
Reduced v/c Ratio	0.39	0.00	0.00	0.52	0.01	0.01	
Intersection Summary							
Cycle Length: 129							
Actuated Cycle Length: 67.2	2						
Control Type: Semi Act-Unc	oord						
Maximum v/c Ratio: 0.55							
Intersection Signal Delay: 3.	.9			Ir	ntersectior	n LOS: A	
Intersection Capacity Utilization						of Service	С
Analysis Period (min) 15							
, , , , , , , , , , , , , , , , , , ,							

Splits and Phases: 3: Go Station East Access & Lakeshore Dr.

<b>√</b> Ø1	<b>₩</b> Ø2	₩ø4	
38 s 5	Ds	25 s	
<b>₩</b> Ø6		<b>▲</b> √Ø8	
50 s		41 s	

#### 4: Essa Rd. & Gowan St.

	4	۰.	1	1	1	Ŧ			
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT			
Lane Configurations	Υ		<b>∱1</b> ≱			- <b>€</b> †			
Traffic Volume (vph)	4	182	312	5	140	231			
Future Volume (vph)	4	182	312	5	140	231			
Satd. Flow (prot)	1616	0	3424	0	0	3342			
Flt Permitted	0.999					0.688			
Satd. Flow (perm)	1616	0	3424	0	0	2343			
Satd. Flow (RTOR)	222		2						
Lane Group Flow (vph)	227	0	382	0	0	387			
Turn Type	Prot		NA		pm+pt	NA			
Protected Phases	8		2		1	6			
Permitted Phases					6				
Total Split (s)	31.0		86.0		11.0	86.0			
Total Lost Time (s)	6.0		6.0			6.0			
Act Effct Green (s)	10.6		80.0			80.0			
Actuated g/C Ratio	0.10		0.78			0.78			
v/c Ratio	0.62		0.14			0.21			
Control Delay	14.4		3.0			3.4			
Queue Delay	0.0		0.0			0.0			
Total Delay	14.4		3.0			3.4			
LOS	В		А			А			
Approach Delay	14.4		3.0			3.4			
Approach LOS	В		A			A			
Queue Length 50th (m)	1.0		8.0			8.6			
Queue Length 95th (m)	16.7		12.3			15.0			
Internal Link Dist (m)	120.4		80.7			19.9			
Turn Bay Length (m)									
Base Capacity (vph)	561		2668			2077			
Starvation Cap Reductn	0		0			0			
Spillback Cap Reductn	0		0			0			
Storage Cap Reductn	0		0			0			
Reduced v/c Ratio	0.40		0.14			0.19			
Intersection Summary									
Cycle Length: 128									
Actuated Cycle Length: 10	27								
Control Type: Semi Act-Un									
Maximum v/c Ratio: 0.62									
Intersection Signal Delay: !	5.8			Ir	ntersection	1 OS· A			
Intersection Capacity Utiliz		/			CU Level				
Analysis Period (min) 15	ation 137.07	0							
	ssa Rd. & Go	wan St.							

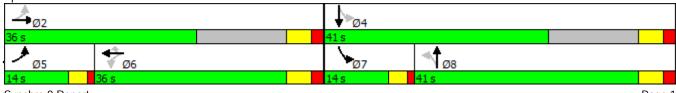


5: Property Access	ccess/Go Station South Access & Essa Rd. Existing A											
	٦	-	$\mathbf{r}$	4	-	*	•	1	1	1	ţ	~
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4						4îb			ፋጉ	
Traffic Volume (veh/h)	9	0	13	0	0	0	14	474	6	12	358	5
Future Volume (Veh/h)	9	0	13	0	0	0	14	474	6	12	358	5
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.79	0.79	0.79	1.00	1.00	1.00	0.84	0.84	0.84	0.93	0.93	0.93
Hourly flow rate (vph)	11	0	16	0	0	0	17	564	7	13	385	5
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)								44			80	
pX, platoon unblocked	0.99	0.99	0.98	0.99	0.99	0.98	0.98			0.98		
vC, conflicting volume	730	1018	195	836	1018	286	390			571		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol	500	001	104	70/	000	004	224			F1/		
vCu, unblocked vol	598	891	124	706	890	224	324			516		
tC, single (s)	7.5	6.5	6.9	7.5	6.5	6.9	4.2			6.1		
tC, 2 stage (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.3			3.2		
tF (s) p0 queue free %	5.5 97	4.0	5.5 98	3.5 100	4.0	5.5 100	2.3 99			5.2 98		
cM capacity (veh/h)	374	270	887	304	267	762	1168			572		
						702	1100			572		
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2							
Volume Total	27	299	289	206	198							
Volume Left	11	17	0	13	0							
Volume Right cSH	16	0	7	0 572	5							
Volume to Capacity	569 0.05	1168 0.01	1700 0.17	0.02	1700 0.12							
Queue Length 95th (m)	1.2	0.01	0.17	0.02	0.12							
Control Delay (s)	1.2	0.4	0.0	1.0	0.0							
Lane LOS	B	0.0 A	0.0	1.0 A	0.0							
Approach Delay (s)	11.6	0.3		0.5								
Approach LOS	B	0.5		0.5								
Intersection Summary												
Average Delay			0.7									
Intersection Capacity Utiliza	ation		33.4%	IC	Ulevelo	of Service			А			
Analysis Period (min)			15	.0	5 201010				,,			
J												

#### 5: Property Access/Go Station South Access & Essa Rd.

Existing AM

1: Go Station West Access & Tiffin St. & Lakeshore Dr. Existing Pl													
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	1	el el		ľ	<u></u>	1	1	el el		2	¢Î		
Traffic Volume (vph)	141	422	15	2	251	505	21	1	2	627	21	108	
Future Volume (vph)	141	422	15	2	251	505	21	1	2	627	21	108	
Satd. Flow (prot)	1805	1890	0	1805	3610	1599	1031	1685	0	1805	1647	0	
Flt Permitted	0.521			0.507			0.952			0.488			
Satd. Flow (perm)	990	1890	0	963	3610	1599	1034	1685	0	927	1647	0	
Satd. Flow (RTOR)		2				532					113		
Lane Group Flow (vph)	142	441	0	2	264	532	31	4	0	653	135	0	
Turn Type	pm+pt	NA		Perm	NA	Perm	Perm	NA		pm+pt	NA		
Protected Phases	5	2			6			8		7	4		
Permitted Phases	2			6		6	8			4			
Total Split (s)	14.0	36.0		36.0	36.0	36.0	41.0	41.0		14.0	41.0		
Total Lost Time (s)	4.0	6.0		6.0	6.0	6.0	6.0	6.0		4.0	6.0		
Act Effct Green (s)	45.1	43.1		30.5	30.5	30.5	10.9	10.9		17.7	15.7		
Actuated g/C Ratio	0.64	0.61		0.43	0.43	0.43	0.15	0.15		0.25	0.22		
v/c Ratio	0.20	0.38		0.00	0.17	0.54	0.20	0.02		1.75	0.30		
Control Delay	7.5	9.8		16.0	14.7	4.2	32.6	29.3		371.1	8.1		
Queue Delay	0.0	0.5		0.0	0.0	0.0	0.0	0.0		0.0	0.0		
Total Delay	7.5	10.3		16.0	14.7	4.2	32.6	29.3		371.1	8.1		
LOS	А	В		В	В	А	С	С		F	А		
Approach Delay		9.6			7.7			32.2			308.9		
Approach LOS		А			А			С			F		
Queue Length 50th (m)	4.8	20.4		0.2	9.4	0.0	3.4	0.4		~130.3	2.4		
Queue Length 95th (m)	19.8	65.5		1.6	24.6	20.2	9.2	2.4		#219.1	14.5		
Internal Link Dist (m)		44.8			116.0			88.9			104.9		
Turn Bay Length (m)	35.0			23.0		55.0				75.0			
Base Capacity (vph)	746	1190		413	1549	990	517	843		373	1188		
Starvation Cap Reductn	0	361		0	0	0	0	0		0	0		
Spillback Cap Reductn	0	0		0	0	0	0	0		0	0		
Storage Cap Reductn	0	0		0	0	0	0	0		0	0		
Reduced v/c Ratio	0.19	0.53		0.00	0.17	0.54	0.06	0.00		1.75	0.11		
Intersection Summary													
Cycle Length: 105													
Actuated Cycle Length: 71													
Control Type: Semi Act-Unc	oord												
Maximum v/c Ratio: 1.75													
Intersection Signal Delay: 1	16.3			In	tersectior	1 LOS: F							
Intersection Capacity Utiliza	tion 106.4%	6		IC	CU Level of	of Service	e G						
Analysis Period (min) 15													
<ul> <li>Volume exceeds capacity, queue is theoretically infinite.</li> </ul>													
Queue shown is maximu													
# 95th percentile volume e			eue may	be longer	r.								
Queue shown is maximu	m after two	cycles.											
Splits and Phases: 1: Go	Splits and Phases: 1: Go Station West Access & Tiffin St. & Lakeshore Dr.												
					L L								



Synchro 9 Report

#### 2: Essa Rd./Bradford St. & Tiffin St.

Existing PM

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	4Î		<u>۲</u>	<b>↑</b>			4 Þ			-4↑	1
Traffic Volume (vph)	41	315	80	116	177	0	81	438	149	120	400	62
Future Volume (vph)	41	315	80	116	177	0	81	438	149	120	400	62
Satd. Flow (prot)	1752	1814	0	1687	1863	0	0	3431	0	0	3464	1599
Flt Permitted	0.637			0.433				0.782			0.629	
Satd. Flow (perm)	1175	1814	0	769	1863	0	0	2700	0	0	2203	1599
Satd. Flow (RTOR)		15						46				70
Lane Group Flow (vph)	43	416	0	126	192	0	0	711	0	0	591	70
Turn Type	Perm	NA		Perm	NA		Perm	NA		pm+pt	NA	Perm
Protected Phases		4			8			2		1	6	
Permitted Phases	4			8			2			6	6	6
Total Split (s)	31.0	31.0		31.0	31.0		34.0	34.0		21.0	34.0	34.0
Total Lost Time (s)	6.0	6.0		6.0	6.0			6.0			6.0	6.0
Act Effct Green (s)	25.2	25.2		25.2	25.2			21.8			21.8	21.8
Actuated g/C Ratio	0.43	0.43		0.43	0.43			0.37			0.37	0.37
v/c Ratio	0.09	0.53		0.39	0.24			0.69			0.73	0.11
Control Delay	12.6	16.4		17.9	13.2			18.4			21.6	3.8
Queue Delay	0.0	0.0		0.0	0.0			0.0			0.0	0.0
Total Delay	12.6	16.4		17.9	13.2			18.4			21.6	3.8
LOS	В	В		В	В			В			С	A
Approach Delay		16.1			15.1			18.4			19.7	
Approach LOS		В			В			В			В	
Queue Length 50th (m)	2.8	32.1		9.4	13.3			32.3			29.2	0.0
Queue Length 95th (m)	9.3	65.7		26.0	29.8			48.6			43.9	5.9
Internal Link Dist (m)		153.0			41.6			55.6			43.8	
Turn Bay Length (m)	47.0			23.0								89.0
Base Capacity (vph)	500	781		327	793			1312			1839	1346
Starvation Cap Reductn	0	0		0	0			0			0	0
Spillback Cap Reductn	0	0		0	0			0			0	0
Storage Cap Reductn	0	0		0	0			0			0	0
Reduced v/c Ratio	0.09	0.53		0.39	0.24			0.54			0.32	0.05
Intersection Summary												
Cycle Length: 86												
Actuated Cycle Length: 59.1	1											
Control Type: Semi Act-Unc	coord											
Maximum v/c Ratio: 0.73												
Intersection Signal Delay: 1	7.8			In	itersectior	n LOS: B						
Intersection Capacity Utiliza	tion 95.2%			IC	CU Level o	of Service	e F					
Analysis Period (min) 15												

Splits and Phases: 2: Essa Rd./Bradford St. & Tiffin St.

Ø1	↑ ø₂	<u></u> ø4
21 s	34 s	31s
∳ ø <sub>6</sub>		₹Ø8
34 s		31 s

	-	$\mathbf{\hat{z}}$	1	←	1	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	Ø4
Lane Configurations	<b>†</b>	1	٦	<b>↑</b>	٦	1	
Traffic Volume (vph)	1048	3	3	756	2	4	
Future Volume (vph)	1048	3	3	756	2	4	
Satd. Flow (prot)	1900	1615	1805	1900	1805	1615	
Flt Permitted			0.166		0.950		
Satd. Flow (perm)	1900	1615	315	1900	1805	1615	
Satd. Flow (RTOR)		1				5	
Lane Group Flow (vph)	1069	3	3	788	3	5	
Turn Type	NA	Perm	pm+pt	NA	Prot	Perm	
Protected Phases	2		1	6	8		4
Permitted Phases		2	6			8	
Total Split (s)	50.0	50.0	38.0	50.0	41.0	41.0	25.0
Total Lost Time (s)	6.0	6.0	4.0	6.0	5.0	5.0	
Act Effct Green (s)	62.6	62.6	61.4	64.4	10.1	10.1	
Actuated g/C Ratio	0.92	0.92	0.90	0.95	0.15	0.15	
v/c Ratio	0.61	0.00	0.01	0.44	0.01	0.02	
Control Delay	7.4	3.7	1.3	2.4	28.0	18.8	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	7.4	3.7	1.3	2.4	28.0	18.8	
LOS	А	А	А	А	С	В	
Approach Delay	7.4			2.4	22.2		
Approach LOS	А			А	С		
Queue Length 50th (m)	0.0	0.0	0.0	0.0	0.4	0.0	
Queue Length 95th (m)	#242.5	1.1	0.6	68.8	2.4	2.6	
Internal Link Dist (m)	144.3			137.6	30.3		
Turn Bay Length (m)		40.0	70.0				
Base Capacity (vph)	1748	1486	1034	1900	961	862	
Starvation Cap Reductn	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	
Reduced v/c Ratio	0.61	0.00	0.00	0.41	0.00	0.01	
Intersection Summary							
Cycle Length: 129							
Actuated Cycle Length: 68							
Control Type: Semi Act-Une	coord						
Maximum v/c Ratio: 0.61							
Intersection Signal Delay: 5					itersectior		
Intersection Capacity Utiliza	ation 72.7%			IC	CU Level o	of Service	С
Analysis Period (min) 15							
# 95th percentile volume			leue may	be longe	r.		
Queue shown is maximu	um after two	cycles.					

Splits and Phases: 3: Go Station East Access & Lakeshore Dr.

<b>√</b> Ø1	<b>₩</b> Ø2	₩ø4	
38 s	50 s	25 s	
<b>★</b> Ø6		<b>▲</b> √ø8	
50 s		41 s	

#### 4: Essa Rd. & Gowan St.

	4	۰.	Ť	۲	1	ţ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		<b>≜</b> †⊅			4†
Traffic Volume (vph)	6	284	406	9	214	390
Future Volume (vph)	6	284	406	9	214	390
Satd. Flow (prot)	1632	0	3496	0	0	3344
Flt Permitted	0.999					0.661
Satd. Flow (perm)	1632	0	3496	0	0	2248
Satd. Flow (RTOR)	369	-	3	-	-	
Lane Group Flow (vph)	377	0	482	0	0	702
Turn Type	Prot	Ū	NA	0	pm+pt	NA
Protected Phases	8		2		1 1	6
Permitted Phases	U		2		6	0
Total Split (s)	31.0		86.0		11.0	86.0
					11.0	
Total Lost Time (s)	6.0		6.0			6.0
Act Effct Green (s)	11.4		80.1			80.1
Actuated g/C Ratio	0.11		0.77			0.77
v/c Ratio	0.74		0.18			0.40
Control Delay	14.7		3.4			4.9
Queue Delay	0.0		0.0			0.2
Total Delay	14.7		3.4			5.1
LOS	В		А			А
Approach Delay	14.7		3.4			5.1
Approach LOS	В		А			А
Queue Length 50th (m)	1.5		10.3			19.1
Queue Length 95th (m)	12.3		19.3			35.0
Internal Link Dist (m)	120.4		80.7			19.9
Turn Bay Length (m)	.20.1		0017			. , . ,
Base Capacity (vph)	674		2704			1977
Starvation Cap Reductn	0		0			591
Spillback Cap Reductin	0		0			0
Storage Cap Reductin	0		0			0
Reduced v/c Ratio	0.56		0.18			0.51
Intersection Summary						
Cycle Length: 128						
Actuated Cycle Length: 10	3.5					
Control Type: Semi Act-Un	coord					
Maximum v/c Ratio: 0.74						
Intersection Signal Delay:	5.9			lr	ntersection	1 LOS: A
Intersection Capacity Utiliz		6				of Service
Analysis Period (min) 15						
Splits and Phases: 4: Es	sa Rd. & Go	wan St.				



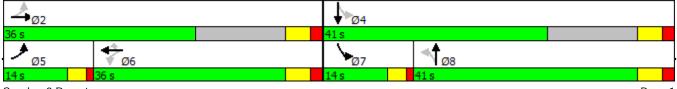
Synchro 9 Report

5: Property Access	s/Go Sta	tion S	outh A	ccess	& Essa	a Rd.	Rd. Existing Pl									
	٦	-	$\mathbf{i}$	∢	+	•	•	Ť	1	\$	ţ	~				
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR				
Lane Configurations		<b>4</b>						4î b			đ ĥ					
Traffic Volume (veh/h)	4	0	23	0	0	0	18	664	8	12	582	2				
Future Volume (Veh/h)	4	0	23	0	0	0	18	664	8	12	582	2				
Sign Control		Stop			Stop			Free			Free					
Grade		0%			0%			0%			0%					
Peak Hour Factor	0.68	0.68	0.68	1.00	1.00	1.00	0.95	0.95	0.95	0.90	0.90	0.90				
Hourly flow rate (vph)	6	0	34	0	0	0	19	699	8	13	647	2				
Pedestrians																
Lane Width (m)																
Walking Speed (m/s)																
Percent Blockage																
Right turn flare (veh)																
Median type								None			None					
Median storage veh)																
Upstream signal (m)								44			80					
pX, platoon unblocked	0.92	0.92	0.91	0.92	0.92	0.97	0.91			0.97						
vC, conflicting volume	1062	1419	324	1124	1416	354	649			707						
vC1, stage 1 conf vol																
vC2, stage 2 conf vol																
vCu, unblocked vol	757	1146	45	825	1143	279	403			643						
tC, single (s)	7.5	6.5	6.9	7.5	6.5	6.9	4.1			5.9						
tC, 2 stage (s)																
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			3.1						
p0 queue free %	98	100	96	100	100	100	98			97						
cM capacity (veh/h)	267	177	925	226	175	698	1056			507						
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2											
Volume Total	40	368	358	336	326											
Volume Left	6	19	0	13	0											
Volume Right	34	0	8	0	2											
cSH	675	1056	1700	507	1700											
Volume to Capacity	0.06	0.02	0.21	0.03	0.19											
Queue Length 95th (m)	1.5	0.4	0.0	0.6	0.0											
Control Delay (s)	10.7	0.6	0.0	0.8	0.0											
Lane LOS	В	А		А												
Approach Delay (s)	10.7	0.3		0.4												
Approach LOS	В															
Intersection Summary																
Average Delay			0.7													
Intersection Capacity Utiliz	ation		41.6%	IC	U Level o	of Service			А							
Analysis Period (min)			15													

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Image: Construction         EBL         EBR         VBL         VBL         VBR         NBL         NBT         NBR         SBL         SBR         SBR           Lane Condigurations         1	1: Go Station West	1: Go Station West Access & Tiffin St. & Lakeshore Dr. Existing Weekend Pe													
Lane Configurations       Y       A       Y        Current (ynh)       105       228       6       5       191       1466       23       9       0533       165       0       10       104       104       104       104       104       104       104       104       104       104       104       104       104		٦	-	$\mathbf{F}$	∢	-	•	1	Ť	۲	1	Ļ	~		
Traffic Volume (vph)       157       258       6       5       187       457       22       1       6       468       8       145         Future Volume (vph)       157       258       6       5       187       457       22       1       6       468       8       145         Stalt-Flow (prot)       1805       1876       0       1805       3574       1615       1101       1647       0       1787       1630       0         Stalt-Flow (perm)       1060       1876       0       119       3574       1615       1159       1647       0       965       1630       0         Stalt-Flow (perm)       1060       1876       0       119       3574       1647       0       965       1630       0         Stalt-Flow (perm)       165       278       0       5       191       466       23       9       0       503       165       0         Turn Type       pm-upt       NA       Perm       NA       Perm       NA       Permited Phases       2       6       6       8       4       1704       104       10.1       10.1       10.1       10.1       10.1       10	Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR		
Traffic Volume (vph)       157       258       6       5       187       457       22       1       6       468       8       145         Future Volume (vph)       157       258       6       5       187       457       22       1       6       468       8       145         Stalt-Flow (prot)       1805       1876       0       1805       3574       1615       1101       1647       0       1787       1630       0         Stalt-Flow (perm)       1060       1876       0       119       3574       1615       1159       1647       0       965       1630       0         Stalt-Flow (perm)       1060       1876       0       119       3574       1647       0       965       1630       0         Stalt-Flow (perm)       165       278       0       5       191       466       23       9       0       503       165       0         Turn Type       pm-upt       NA       Perm       NA       Perm       NA       Permited Phases       2       6       6       8       4       1704       104       10.1       10.1       10.1       10.1       10.1       10	Lane Configurations	1	eî		1	<b>^</b>	*	1	¢Î		۲. ۲	el el			
Satd. Flow (pent)       1805       1876       0       1805       3574       1615       1101       1647       0       1787       1630       0         FII Permitted       0.558       0.589       0.513       0.513       0       0.513       0         Satd. Flow (perm)       1000       176       0       119       3574       1615       1159       1647       0       965       1650       0         Lane Group Flow (vph)       165       278       0       5       191       466       8       7       4         Protected Phases       5       2       6       6       8       7       4         Promitted Phases       2       6       6       8       4       4       104       14.0       14.0       40.0       6.0       6.0       6.0       6.0       4.0       6.0				6						6	468		145		
Fit Permited       0.589       0.513         Satd. Flow (perm)       1060       1876       0       1119       3574       1615       1159       1647       0       965       1630       0         Satd. Flow (perm)       165       278       0       5       191       466       156       165         Lane Group Flow (vph)       165       278       0       5       191       466       23       9       0       503       165       0         Turn Type       pm+pt       NA       Perm       Perm       NA       Perm       NA       pm+pt       NA         Permited       Phases       2       6       6       8       4       Total Spit (S)       14.0       40.0       6.0       6.0       6.0       6.0       6.0       4.0       6.0       4.0       6.0       4.0       6.0       6.0       6.0       6.0       6.0       6.0       4.0       6.0       6.0       6.0       6.0       6.0       4.0       4.0       10.0       10.0       10.0       10.0       10.0       10.0       10.0       10.0       10.0       10.0       10.0       10.0       10.0       10.0       10.0	Future Volume (vph)	157	258	6	5	187	457	22	1	6	468	8	145		
Said. Flow (perm)       1060       1876       0       119       3574       1615       1159       1647       0       965       1630       0         Said. Flow (RTOR)       1       466       23       9       0       503       165       0         Lane Group Prove (vph)       165       278       0       5       191       466       23       9       0       503       165       0         Prolected Phases       5       2       6       6       8       7       4       Permited Phases       1       410       14.0       41.0 </td <td>Satd. Flow (prot)</td> <td>1805</td> <td>1876</td> <td>0</td> <td>1805</td> <td>3574</td> <td>1615</td> <td>1101</td> <td>1647</td> <td>0</td> <td>1787</td> <td>1630</td> <td>0</td>	Satd. Flow (prot)	1805	1876	0	1805	3574	1615	1101	1647	0	1787	1630	0		
Satd. Flow (PTOR)       1       466       156         Lane Group Flow (vph)       165       278       0       5       191       466       23       9       0       503       165       0         Turn Type       pm+pt       NA       Perm       NA       Perm       NA       Perm       NA       pm+pt       NA         Prodicted Phases       2       6       6       8       7       4         Permitted Phases       2       6       6       8       4       104       14.0       41.0	Flt Permitted	0.558			0.589						0.513				
Lane Group Flow (vph) 165 278 0 5 191 466 23 9 0 503 165 0 Turn Type pri-pt NA Perm NA Perm Perm NA pri-pt NA Permited Phases 5 2 6 6 8 7 4 Total Split (s) 140 36.0 36.0 36.0 36.0 41.0 41.0 14.0 41.0 Total Lost Time (s) 4.0 6.0 6.0 6.0 6.0 6.0 6.0 6.0 6.0 Act Efft Green (s) 45.3 43.2 30.4 30.4 30.4 10.4 10.4 17.3 15.3 Actuated g/C Ratio 0.64 0.61 0.43 0.43 0.43 0.43 0.15 0.15 0.24 0.22 v/C Ratio 0.21 0.24 0.01 0.12 0.49 0.14 0.04 1.36 0.35 Control Delay 7.2 8.2 15.6 14.3 3.9 31.7 30.0 204.7 6.7 Oueue Delay 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.	Satd. Flow (perm)	1060	1876	0	1119	3574	1615	1159	1647	0	965	1630	0		
Turn Type     pm+pt     NA     Perm     NA     Perm     NA     pm-pt     NA       Protected Phases     5     2     6     6     8     7     4       Protected Phases     2     6     6     8     7     4       Permitted Phases     2     6     6     8     4       Total Split (s)     14.0     36.0     36.0     36.0     41.0     41.0     14.0       At Effet Green (s)     45.3     43.2     30.4     30.4     30.4     10.4     10.4     17.3     15.3       Actuated g/C Ratio     0.44     0.61     0.43     0.43     0.14     0.04     1.36     0.35       Control Delay     7.2     8.2     15.6     14.3     3.9     31.7     30.0     204.7     6.7       Cotal Delay     7.2     8.2     15.6     14.3     3.9     31.7     30.0     204.7     6.7       LOS     A     A     B     B     A     C     C     F     A       Approach LoS     A     A     B     B     0.2.6     1.0     -91.5     1.0       Ouceu Length 95th (m)     5.7     11.6     0.3     6.8     0.0     2.6 <td< td=""><td>Satd. Flow (RTOR)</td><td></td><td>-</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></td<>	Satd. Flow (RTOR)		-												
Protected Phases       5       2       6       8       7       4         Permitted Phases       2       6       6       8       4         Total Split (s)       14.0       36.0       36.0       36.0       40.0       40.0       41.0       41.0       41.0         Total Lost Time (s)       4.0       6.0       6.0       6.0       6.0       6.0       40.0       6.0         Act Effc Green (s)       45.3       43.2       30.4       30.4       10.4       10.4       17.3       15.3         ActLated g/C Ratio       0.64       0.61       0.43       0.43       0.43       0.15       0.15       0.24       0.22         wc Ratio       0.21       0.24       0.01       0.12       0.49       0.14       0.04       1.36       0.35         Control Delay       7.2       8.2       15.6       14.3       3.9       31.7       30.0       204.7       6.7         Loes       A       A       B       B       A       C       F       A         Approach LOS       A       A       B       10.3       4.5       #15.8.6       14.3         Internal Link Dist (m)       5	Lane Group Flow (vph)	165		0	5		466	23		0	503		0		
Permitted Phases         2         6         6         8         4           Total Split (s)         14.0         36.0         36.0         36.0         41.0         11.0         14.0         41.0           Total Lost Time (s)         4.0         6.0         6.0         6.0         6.0         6.0         4.0         6.0           Actuated g/C Ratio         0.64         0.61         0.43         0.43         0.15         0.24         0.22           w/c Ratio         0.21         0.24         0.01         0.12         0.49         0.14         0.04         1.36         0.35           Control Delay         7.2         8.2         15.6         14.3         3.9         31.7         30.0         204.7         6.7           Queue Delay         0.0		pm+pt			Perm		Perm	Perm			pm+pt				
Total Split (s)       14.0       36.0       36.0       36.0       36.0       41.0       41.0       14.0       41.0         Total Lost Time (s)       4.0       6.0       6.0       6.0       6.0       6.0       6.0       4.0       6.0         Act Effed Green (s)       45.3       43.2       30.4       30.4       30.4       10.4       10.4       17.3       15.3         Actuated g/C Ratio       0.64       0.61       0.43       0.43       0.43       0.15       0.15       0.24       0.22         w/c Ratio       0.21       0.24       0.01       0.12       0.49       0.14       0.04       1.36       0.35         Control Delay       7.2       8.2       15.6       14.3       3.9       31.7       30.0       204.7       6.7         Queue Delay       7.2       8.2       15.6       14.3       3.9       31.7       30.0       204.7       6.7         Coursol Delay       7.9       7.0       31.2       155.8       Approach LOS       A       A       C       F       A         Oueue Length S0th (m)       5.7       11.6       0.3       6.8       0.0       2.6       1.0       -91.5 <td></td> <td></td> <td>2</td> <td></td> <td></td> <td>6</td> <td></td> <td></td> <td>8</td> <td></td> <td>7</td> <td>4</td> <td></td>			2			6			8		7	4			
Total Lost Time (s)       4.0       6.0       6.0       6.0       6.0       6.0       4.0       6.0         Act Efric Green (s)       45.3       43.2       30.4       30.4       10.4       10.4       17.3       15.3         Actuated g/C Ratio       0.21       0.24       0.01       0.12       0.43       0.43       0.15       0.15       0.24       0.22         w(Ratio       0.21       0.24       0.01       0.12       0.49       0.14       0.04       1.36       0.35         Control Delay       7.2       8.2       15.6       14.3       3.9       31.7       30.0       204.7       6.7         Queue Delay       0.0															
Act Effct Green (s)       45.3       43.2       30.4       30.4       30.4       10.4       10.4       17.3       15.3         Actuated g/C Ratio       0.64       0.61       0.43       0.43       0.15       0.15       0.24       0.22         v/c Ratio       0.21       0.24       0.01       0.12       0.49       0.14       0.04       1.36       0.35         Control Delay       7.2       8.2       15.6       14.3       3.9       31.7       30.0       204.7       6.7         Queue Delay       0.0															
Actuated g/C Ratio       0.64       0.61       0.43       0.43       0.15       0.15       0.24       0.22         Wic Ratio       0.21       0.24       0.01       0.12       0.49       0.14       0.04       1.36       0.35         Control Delay       7.2       8.2       15.6       14.3       3.9       31.7       30.0       204.7       6.7         Queue Delay       0.0       0       0.0       0.0       0       0       0.0       0       0       0       0.0       0       0.0       0       0.0       0       0.0       0       0.0       0       0.0       0       0.0       0															
v/c Ratio       0.21       0.24       0.01       0.12       0.49       0.14       0.04       1.36       0.35         Control Delay       7.2       8.2       15.6       14.3       3.9       31.7       30.0       204.7       6.7         Queue Delay       0.0       0       0       0       0.0 <td>( )</td> <td></td>	( )														
Control Delay       7.2       8.2       15.6       14.3       3.9       31.7       30.0       204.7       6.7         Queue Delay       0.0       0.0       0.0       0.0       0.0       0.0       0.0       0.0       0.0         Total Delay       7.2       8.2       15.6       14.3       3.9       31.7       30.0       204.7       6.7         LOS       A       A       B       B       A       C       C       F       A         Approach LOS       A       A       B       B       A       C       C       F       A         Queue Length 50th (m)       5.7       11.6       0.3       6.8       0.0       2.6       1.0       -91.5       1.0         Queue Length 95th (m)       21.4       37.2       2.7       17.9       18.8       10.3       4.5       #158.6       14.3         Internal Link Dist (m)       44.8       116.0       88.9       104.9       104.9         Turn Bay Length (m)       35.0       23.0       55.0       75.0       Base Capacity (pth)       785       118.3       481       153.6       960       581       82.6       370       1191       Starva															
Queue Delay         0.0 <th< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></th<>															
Total Delay       7.2       8.2       15.6       14.3       3.9       31.7       30.0       204.7       6.7         LOS       A       A       B       B       A       C       C       F       A         Approach LOS       A       A       B       B       A       C       C       F       A         Approach LOS       A       A       C       F       F       Oucue Length 50th (m)       5.7       11.6       0.3       6.8       0.0       2.6       1.0       -91.5       1.0         Oucue Length 95th (m)       21.4       37.2       2.7       17.9       18.8       10.3       4.5       #158.6       14.3         Internal Link Dist (m)       44.8       116.0       88.9       104.9       104.9         Turn Bay Length (m)       35.0       23.0       55.0       75.0       Base Capacity (vph)       785       1183       481       153.6       960       581       82.6       370       1191         Starvation Cap Reductn       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0	3														
LOS       A       A       B       B       A       C       C       F       A         Approach LOS       A       A       C       F       A       F         Queue Length 50th (m)       5.7       11.6       0.3       6.8       0.0       2.6       1.0      91.5       1.0         Queue Length 95th (m)       21.4       37.2       2.7       17.9       18.8       10.3       4.5       #158.6       14.3         Internal Link Dist (m)       44.8       116.0       88.9       104.9       114.3         Turn Bay Length (m)       35.0       23.0       55.0       75.0       55.0         Base Capacity (vph)       785       118.3       481       1536       960       581       826       370       1191         Starvation Cap Reductn       0 <t< td=""><td>5</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></t<>	5														
Approach Delay       7.9       7.0       31.2       155.8         Approach LOS       A       A       C       F         Queue Length 50th (m)       5.7       11.6       0.3       6.8       0.0       2.6       1.0      91.5       1.0         Queue Length 95th (m)       21.4       37.2       2.7       17.9       18.8       10.3       4.5       #158.6       14.3         Internal Link Dist (m)       44.8       116.0       88.9       104.9         Turn Bay Length (m)       35.0       23.0       55.0       75.0         Base Capacity (vph)       785       1183       481       1536       960       581       826       370       1191         Starvation Cap Reductn       0       0       0       0       0       0       0       0         Storage Cap Reductn       0       1.136       1.14													_		
Approach LOS       A       A       C       F         Queue Length 50th (m)       5.7       11.6       0.3       6.8       0.0       2.6       1.0       -91.5       1.0         Queue Length 95th (m)       21.4       37.2       2.7       17.9       18.8       10.3       4.5       #158.6       14.3         Internal Link Dist (m)       44.8       116.0       88.9       104.9         Turn Bay Length (m)       35.0       23.0       55.0       75.0       Base Capacity (vph)       785       1183       481       1536       960       581       826       370       1191         Starvation Cap Reductn       0       1.36		A			В		A	С			F				
Oueue Length 50th (m)       5.7       11.6       0.3       6.8       0.0       2.6       1.0      91.5       1.0         Queue Length 95th (m)       21.4       37.2       2.7       17.9       18.8       10.3       4.5       #158.6       14.3         Internal Link Dist (m)       44.8       116.0       88.9       104.9         Turn Bay Length (m)       35.0       23.0       55.0       75.0         Base Capacity (vph)       785       1183       481       1536       960       581       826       370       1191         Starvation Cap Reductn       0       0       0       0       0       0       0       0       0         Starvation Cap Reductn       0       1.8       0.14       1.8       1.4       1.4       1.4       1.4<													_		
Queue Length 95th (m)       21.4       37.2       2.7       17.9       18.8       10.3       4.5       #158.6       14.3         Internal Link Dist (m)       44.8       116.0       88.9       104.9         Turn Bay Length (m)       35.0       23.0       55.0       75.0         Base Capacity (vph)       785       1183       481       1536       960       581       826       370       1191         Starvation Cap Reductn       0       0       0       0       0       0       0       0       0         Spillback Cap Reductn       0       1.36       0.14       14.5       44.8       14.3       14.5       14.5       14.5       14.5       14.5       14.5       14.5       14.5       14.5       14.		5 7			0.0		0.0	o (			04 5				
Internal Link Dist (m)       44.8       116.0       88.9       104.9         Turn Bay Length (m)       35.0       23.0       55.0       75.0         Base Capacity (vph)       785       1183       481       1536       960       581       826       370       1191         Starvation Cap Reductn       0       0       0       0       0       0       0       0       0         Spillback Cap Reductn       0       0       0       0       0       0       0       0       0         Storage Cap Reductn       0       0       0       0       0       0       0       0       0         Reduced v/c Ratio       0.21       0.23       0.01       0.12       0.49       0.04       0.01       1.36       0.14         Intersection Summary         Cycle Length: 70.7         Control Type: Semi Act-Uncoord       Maximum v/c Ratio: 1.36       1       <															
Turn Bay Length (m)       35.0       23.0       55.0       75.0         Base Capacity (vph)       785       1183       481       1536       960       581       826       370       1191         Starvation Cap Reductn       0       0       0       0       0       0       0       0       0       0         Spillback Cap Reductn       0       1       3.6       0       14       1       16		21.4			2.1		18.8	10.3			#158.6				
Base Capacity (vph)       785       1183       481       1536       960       581       826       370       1191         Starvation Cap Reductn       0       0       0       0       0       0       0       0       0         Spillback Cap Reductn       0       0       0       0       0       0       0       0       0         Storage Cap Reductn       0       0       0       0       0       0       0       0       0         Storage Cap Reductn       0 <td></td> <td>25.0</td> <td>44.8</td> <td></td> <td>22.0</td> <td>116.0</td> <td></td> <td></td> <td>88.9</td> <td></td> <td></td> <td>104.9</td> <td>_</td>		25.0	44.8		22.0	116.0			88.9			104.9	_		
Starvation Cap Reductin       0 <td></td> <td></td> <td>1100</td> <td></td> <td></td> <td>150/</td> <td></td> <td>F01</td> <td>027</td> <td></td> <td></td> <td>1101</td> <td></td>			1100			150/		F01	027			1101			
Spillback Cap Reductin       0       0       0       0       0       0       0       0       0         Storage Cap Reductin       0															
Storage Cap Reductin0000000000Reduced v/c Ratio0.210.230.010.120.490.040.011.360.14Intersection SummaryCycle Length: 105Actuated Cycle Length: 70.7Control Type: Semi Act-UncoordMaximum v/c Ratio: 1.36Intersection Signal Delay: 62.7Intersection LOS: EIntersection Capacity Utilization 97.6%ICU Level of Service FAnalysis Period (min) 15Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles.##95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.															
Reduced v/c Ratio0.210.230.010.120.490.040.011.360.14Intersection SummaryCycle Length: 105Actuated Cycle Length: 70.7Control Type: Semi Act-UncoordMaximum v/c Ratio: 1.36Intersection Signal Delay: 62.7Intersection Capacity Utilization 97.6%Analysis Period (min) 15- Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles.# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.															
Intersection Summary         Cycle Length: 105         Actuated Cycle Length: 70.7         Control Type: Semi Act-Uncoord         Maximum v/c Ratio: 1.36         Intersection Signal Delay: 62.7         Intersection Capacity Utilization 97.6%         ICU Level of Service F         Analysis Period (min) 15         -         Volume exceeds capacity, queue is theoretically infinite.         Queue shown is maximum after two cycles.         #       95th percentile volume exceeds capacity, queue may be longer.         Queue shown is maximum after two cycles.															
Cycle Length: 105 Actuated Cycle Length: 70.7 Control Type: Semi Act-Uncoord Maximum v/c Ratio: 1.36 Intersection Signal Delay: 62.7 Intersection LOS: E Intersection Capacity Utilization 97.6% ICU Level of Service F Analysis Period (min) 15 ~ Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles. # 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.		0.21	0.23		0.01	0.12	0.49	0.04	0.01		1.30	0.14			
Actuated Cycle Length: 70.7         Control Type: Semi Act-Uncoord         Maximum v/c Ratio: 1.36         Intersection Signal Delay: 62.7         Intersection Capacity Utilization 97.6%         ICU Level of Service F         Analysis Period (min) 15         ~ Volume exceeds capacity, queue is theoretically infinite.         Queue shown is maximum after two cycles.         # 95th percentile volume exceeds capacity, queue may be longer.         Queue shown is maximum after two cycles.	Intersection Summary														
Control Type: Semi Act-Uncoord Maximum v/c Ratio: 1.36 Intersection Signal Delay: 62.7 Intersection LOS: E Intersection Capacity Utilization 97.6% ICU Level of Service F Analysis Period (min) 15 ~ Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles. # 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.	Cycle Length: 105														
Maximum v/c Ratio: 1.36         Intersection Signal Delay: 62.7       Intersection LOS: E         Intersection Capacity Utilization 97.6%       ICU Level of Service F         Analysis Period (min) 15       -         ~ Volume exceeds capacity, queue is theoretically infinite.	Actuated Cycle Length: 70.	7													
Intersection Signal Delay: 62.7       Intersection LOS: E         Intersection Capacity Utilization 97.6%       ICU Level of Service F         Analysis Period (min) 15       Volume exceeds capacity, queue is theoretically infinite.         Queue shown is maximum after two cycles.       95th percentile volume exceeds capacity, queue may be longer.         Queue shown is maximum after two cycles.       Queue shown is maximum after two cycles.	Control Type: Semi Act-Une	coord													
Intersection Capacity Utilization 97.6%       ICU Level of Service F         Analysis Period (min) 15       -         Volume exceeds capacity, queue is theoretically infinite.       -         Queue shown is maximum after two cycles.       -         # 95th percentile volume exceeds capacity, queue may be longer.       -         Queue shown is maximum after two cycles.       -	Maximum v/c Ratio: 1.36														
<ul> <li>Analysis Period (min) 15</li> <li>Volume exceeds capacity, queue is theoretically infinite.</li> <li>Queue shown is maximum after two cycles.</li> <li># 95th percentile volume exceeds capacity, queue may be longer.</li> <li>Queue shown is maximum after two cycles.</li> </ul>															
<ul> <li>Volume exceeds capacity, queue is theoretically infinite.</li> <li>Queue shown is maximum after two cycles.</li> <li># 95th percentile volume exceeds capacity, queue may be longer.</li> <li>Queue shown is maximum after two cycles.</li> </ul>		ation 97.6%			IC	U Level	of Service	F							
Queue shown is maximum after two cycles.         # 95th percentile volume exceeds capacity, queue may be longer.         Queue shown is maximum after two cycles.															
<ul> <li>95th percentile volume exceeds capacity, queue may be longer.</li> <li>Queue shown is maximum after two cycles.</li> </ul>				ally infini	te.										
Queue shown is maximum after two cycles.															
	•			eue may	be longe	ſ.									
Splits and Phases: 1: Go Station West Access & Tiffin St. & Lakeshore Dr.	Queue shown is maximu	um atter two	cycles.												
	Splits and Dhasaes 1. Co. Station Wast Accors 8. Tiffin St. 8. Lakashara Dr.														
			31 76653												



Synchro 9 Report

#### 2: Essa Rd./Bradford St. & Tiffin St.

Existing Weekend Peak

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	٦	4Î		٦	<b>↑</b>			4î»				1
Traffic Volume (vph)	26	206	56	152	134	0	61	357	180	45	208	38
Future Volume (vph)	26	206	56	152	134	0	61	357	180	45	208	38
Satd. Flow (prot)	1736	1810	0	1736	1863	0	0	3410	0	0	3464	1615
Flt Permitted	0.657			0.582				0.875			0.738	
Satd. Flow (perm)	1200	1810	0	1063	1863	0	0	2999	0	0	2579	1615
Satd. Flow (RTOR)		16						83				42
Lane Group Flow (vph)	29	291	0	179	158	0	0	665	0	0	281	42
Turn Type	Perm	NA		Perm	NA		Perm	NA		pm+pt	NA	Perm
Protected Phases		4			8			2		1	6	
Permitted Phases	4			8			2			6	6	6
Total Split (s)	31.0	31.0		31.0	31.0		34.0	34.0		21.0	34.0	34.0
Total Lost Time (s)	6.0	6.0		6.0	6.0			6.0			6.0	6.0
Act Effct Green (s)	25.1	25.1		25.1	25.1			15.7			15.7	15.7
Actuated g/C Ratio	0.47	0.47		0.47	0.47			0.30			0.30	0.30
v/c Ratio	0.05	0.34		0.36	0.18			0.70			0.37	0.08
Control Delay	9.3	10.4		12.6	9.7			18.5			15.8	5.2
Queue Delay	0.0	0.0		0.0	0.0			0.0			0.0	0.0
Total Delay	9.3	10.4		12.6	9.7			18.5			15.8	5.2
LOS	А	В		В	А			В			В	А
Approach Delay		10.3			11.2			18.5			14.4	
Approach LOS		В			В			В			В	
Queue Length 50th (m)	1.4	15.4		10.2	8.2			26.4			11.4	0.0
Queue Length 95th (m)	5.8	36.0		25.3	19.3			41.4			19.6	5.1
Internal Link Dist (m)		153.0			41.6			55.6			43.8	
Turn Bay Length (m)	47.0			23.0								89.0
Base Capacity (vph)	569	867		504	884			1632			2390	1499
Starvation Cap Reductn	0	0		0	0			0			0	0
Spillback Cap Reductn	0	0		0	0			0			0	0
Storage Cap Reductn	0	0		0	0			0			0	0
Reduced v/c Ratio	0.05	0.34		0.36	0.18			0.41			0.12	0.03
Intersection Summary												
Cycle Length: 86												
Actuated Cycle Length: 52.9	7											
Control Type: Semi Act-Unc												
Maximum v/c Ratio: 0.70												
Intersection Signal Delay: 1	4.6			In	itersection	ו LOS: B						
Intersection Capacity Utiliza				IC	CU Level	of Service	Ε					
Analysis Period (min) 15												

Splits and Phases: 2: Essa Rd./Bradford St. & Tiffin St.

Ø1	<b>▲</b> ¶ <sub>Ø2</sub>	<u></u> 04
21 s	34 s	31 s
∳ ø <sub>6</sub>		✓ Ø8
34 s		31 s

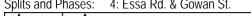
	-	$\mathbf{r}$	1	-	1	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	Ø4
Lane Configurations	1	1	ሻ	<b>†</b>	7	1	
Traffic Volume (vph)	730	2	1	645	4	6	
Future Volume (vph)	730	2	1	645	4	6	
Satd. Flow (prot)	1881	1615	1805	1881	1805	1615	
Flt Permitted			0.288		0.950		
Satd. Flow (perm)	1881	1615	547	1881	1805	1615	
Satd. Flow (RTOR)		1				10	
Lane Group Flow (vph)	793	2	1	672	7	10	
Turn Type	NA	Perm	pm+pt	NA	Prot	Perm	
Protected Phases	2		1	6	8		4
Permitted Phases		2	6			8	
Total Split (s)	50.0	50.0	38.0	50.0	41.0	41.0	25.0
Total Lost Time (s)	6.0	6.0	4.0	6.0	5.0	5.0	
Act Effct Green (s)	61.5	61.5	60.5	63.5	10.0	10.0	
Actuated g/C Ratio	0.92	0.92	0.90	0.94	0.15	0.15	
v/c Ratio	0.46	0.00	0.00	0.38	0.03	0.04	
Control Delay	4.9	3.5	2.0	2.1	26.8	16.0	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	4.9	3.5	2.0	2.1	26.8	16.0	
LOS	А	А	А	А	С	В	
Approach Delay	4.9			2.1	20.4		
Approach LOS	А			А	С		
Queue Length 50th (m)	0.0	0.0	0.0	0.0	0.8	0.0	
Queue Length 95th (m)	123.8	0.9	0.3	54.0	3.1	2.4	
Internal Link Dist (m)	144.3			137.6	30.3		
Turn Bay Length (m)		40.0	70.0				
Base Capacity (vph)	1723	1479	1131	1881	970	872	
Starvation Cap Reductn	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	
Reduced v/c Ratio	0.46	0.00	0.00	0.36	0.01	0.01	
Intersection Summary							
Cycle Length: 129							
Actuated Cycle Length: 67.2							
Control Type: Semi Act-Unc	oord						
Maximum v/c Ratio: 0.46							
Intersection Signal Delay: 3.					itersectior		
Intersection Capacity Utiliza	tion 55.9%			IC	CU Level o	of Service	В
Analysis Period (min) 15							

Splits and Phases: 3: Go Station East Access & Lakeshore Dr.

<b>√</b> Ø1	<u>→</u> Ø2	₩ <b>k</b> ø4	
38 s	50 s	25 s	
<b>★</b> Ø6		<b>▲</b> √Ø8	
50 s		41 s	

#### 4: Essa Rd. & Gowan St.

	1	*	Ť	۲	1	Ļ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		<b>∱1</b> ≱			4ħ
Traffic Volume (vph)	4	230	392	7	116	310
Future Volume (vph)	4	230	392	7	116	310
Satd. Flow (prot)	1646	0	3530	0	0	3487
Flt Permitted	0.999					0.703
Satd. Flow (perm)	1646	0	3530	0	0	2484
Satd. Flow (RTOR)	280		2			
Lane Group Flow (vph)	285	0	480	0	0	489
Turn Type	Prot	Ŭ	NA	Ŭ	pm+pt	NA
Protected Phases	8		2		1	6
Permitted Phases	Ū		-		6	Ū
Total Split (s)	31.0		86.0		11.0	86.0
Total Lost Time (s)	6.0		6.0		11.0	6.0
Act Effct Green (s)	10.9		80.0			80.0
Actuated g/C Ratio	0.11		0.78			0.78
v/c Ratio	0.11		0.78			0.78
Control Delay	14.2		3.2			3.7
Queue Delay	0.0		3.2 0.0			3.7 0.2
3	14.2		3.2			3.8
Total Delay LOS	14.2 B		3.2 A			3.0 A
	в 14.2		A 3.2			3.8
Approach Delay						
Approach LOS	B		A			A
Queue Length 50th (m)	1.0		10.3			11.4
Queue Length 95th (m)	17.4		16.4			19.4
Internal Link Dist (m)	120.4		80.7			19.9
Turn Bay Length (m)			0745			0107
Base Capacity (vph)	611		2745			2197
Starvation Cap Reductn	0		0			848
Spillback Cap Reductn	0		0			0
Storage Cap Reductn	0		0			0
Reduced v/c Ratio	0.47		0.17			0.36
Intersection Summary						
Cycle Length: 128						
Actuated Cycle Length: 10	2.9					
Control Type: Semi Act-Un						
Maximum v/c Ratio: 0.67						
Intersection Signal Delay: !	59			Ir	ntersection	110 <u>S</u> ·A
Intersection Capacity Utiliz		<b>/</b>				of Service
Analysis Period (min) 15	122.07					
Splits and Phases: 4: Es	sa Rd. & Go	wan St				
		mun Jt.				





5: Property Access/	Go Sta	tion So	outh A	ccess	& Essa	a Rd.		Existing Weekend Pea				
	۶	-	$\mathbf{i}$	4	←	•	1	Ť	1	1	ţ	~
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4						ፋጉ			đ þ	
Traffic Volume (veh/h)	9	0	33	0	0	0	27	589	6	10	393	13
Future Volume (Veh/h)	9	0	33	0	0	0	27	589	6	10	393	13
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.82	0.82	0.82	1.00	1.00	1.00	0.84	0.84	0.84	0.86	0.86	0.86
Hourly flow rate (vph)	11	0	40	0	0	0	32	701	7	12	457	15
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)								44			80	
pX, platoon unblocked	0.98	0.98	0.98	0.98	0.98	0.97	0.98			0.97		
vC, conflicting volume	903	1260	236	1061	1264	354	472			708		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												_
vCu, unblocked vol	752	1117	169	913	1121	268	411			634		
tC, single (s)	7.5	6.5	6.9	7.5	6.5	6.9	4.1			6.1		_
tC, 2 stage (s)	0.5	4.0	0.0	0.5	1.0	0.0	0.0			0.0		
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			3.2		
p0 queue free %	96	100	95	100	100	100	97			98		
cM capacity (veh/h)	285	194	832	205	190	707	1132			487		
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2							
Volume Total	51	382	358	240	244							
Volume Left	11	32	0	12	0							
Volume Right	40	0	7	0	15							
cSH	588	1132	1700	487	1700							
Volume to Capacity	0.09	0.03	0.21	0.02	0.14							_
Queue Length 95th (m)	2.3	0.7	0.0	0.6	0.0							
Control Delay (s)	11.7	1.0	0.0	1.0	0.0							_
Lane LOS	В	A		A								
Approach Delay (s)	11.7	0.5		0.5								_
Approach LOS	В											
Intersection Summary												
Average Delay			0.9									
Intersection Capacity Utilizat	ion		42.2%	IC	U Level o	of Service			А			
Analysis Period (min)			15									

# APPENDIX

### **D** Site Traffic Estimates

## vsp

#### **Allandale Mobility Hub Study**

#### Road Network Modifications & Volume Assumptions

#### Growth Rate and Opening Year:

- Opening year assumed to be 2024 to coincide with the introduction of all-day rail service
- 2% annual compound growth rate

#### Peak Periods:

AM Peak – 8:00-9:00 PM Peak – 16:30-17:30 Weekend Peak – 13:00-14:00

#### Bus Volumes:

All routes are assumed to run twice per hour in both directions. Route 90 has been considered under the category of 'other services' because of its unusual headway and the possibility that it will eventually be taken over by Simcoe County. The current schedule of Route 90 does not overlap with any of the peak traffic periods identified.

Intersection Movement	Routes	Hourly Bus Volume
Bradford/Essa Rd & Tiffin St NBR	5	10
Bradford/Essa Rd & Tiffin St WBL	5	10
Bradford/Essa Rd & Tiffin St WBTH	6	12
Bradford/Essa Rd & Tiffin St EBTH	6	12
Bradford/Essa Rd & Tiffin St WBR	5	10
Bradford/Essa Rd & Tiffin St SBL	5	10
Lakeshore-Tiffin-Station Access EBR	7 Through (x4) 2 Terminating (x2)	32
Lakeshore-Tiffin-Station Access NBL	7 Through (x4) 2 Terminating (x2)	32

#### Barrie Transit Services (Scenario 1):

#### Barrie Transit Services (Scenario 2):

Intersection Movement	Routes	Hourly Bus Volume
Bradford/Essa Rd & Tiffin St NBR	5	10
Bradford/Essa Rd & Tiffin St WBL	5	10
Bradford/Essa Rd & Tiffin St WBTH	2	4
Bradford/Essa Rd & Tiffin St EBTH	2	4
Bradford/Essa Rd & Tiffin St WBR	5	10
Bradford/Essa Rd & Tiffin St SBL	5	10
Lakeshore-Tiffin-Station Access EBR	6 (bidirectional)	24
Lakeshore-Tiffin-Station Access NBL	6 (bidirectional)	24

## vsp

#### **Other Services:**

Other Services are assumed to access the site via the 400 interchange at Essa Rd

Intersection Movement	Hourly Bus Volume
Bradford/Essa Rd & Tiffin St NBR	AM Peak: 2
Bradford/Essa Rd & Tiffin St WBL	PM Peak: Weekend: 1
Lakeshore-Tiffin-Station Access EBR	Weekend. I
Lakeshore-Tiffin-Station Access NBL	
Barrie Transit Route 90	

#### Future Passenger Demand:

No GO Rail ridership estimates have been published for the year 2024 when RER is scheduled to begin operation. Forecasts from the GO Rail Station Access Strategy (2016) and the RER Initial Business Case (2014) are available that show current ridership as well as future ridership in 2031. Predicting rail ridership from Allandale Waterfront in the early years of RER is somewhat difficult as there is currently no comparable service from which to draw inferences. It is reasonable to assume that ridership will take several years to fully grow out as more people become aware of the service.

In order to estimate future ridership, current GO Rail annual ridership growth rates of 2.8% were extrapolated to 2024 and coupled with an additional factor to account for the start of all day service. The RER Initial Business Case, published in 2014, estimated that Allandale Waterfront would have about 40% of 2031 total ridership coming from off-peak trips. Accordingly 40% was added to the extrapolated 2024 ridership number to account for additional ridership coming from the introduction of a new type of service. For the AM and PM peak hours in Barrie it is reasonable to assume two train arrivals and departures. For the weekend peak period one rail arrival and departure should be expected. Both sources of ridership estimates predicted very few trips using the station as destination rather than origin. It is therefore reasonable to assume that pick-up and drop-off patterns are focused in one direction.

In order to estimate future passengers using specific modes, current ridership was applied the mode splits published in the GO Rail Station Access Strategy. Because no further parking expansion is anticipated at the station and parking utilization is currently full, all future growth was assumed to be allocated proportionally to walking, local transit and pick-up and drop-off. Of the current alternative forms of station access pick-up and drop-off represents about 50% while walking and local transit represent each about 25%.



Mode	Today	2024 Extrapolated	2024 + All Day Factor	Peak Traffic Hour Estimate
Walk	29.25	20	40	n/a
Local Transit	22.75	20	40	n/a
Micro-Transit	-	-	-	n/a
Cycling	-	-	-	n/a
Pick Up/Drop Off	65	40	82	25
Drive and Park	208	-	-	n/a
Carpool	0	-	-	n/a
Subtotal	325	80	162	50
Total		405	567	n/a

#### Daily Weekday Ridership Estimates for Traffic Analysis

#### Peak Hour Demand:

Passenger volumes during the peak traffic hour should be fairly low accessing the GO Station as most trains depart before the local AM peak and arrive after the local PM Peak. Only one train currently arrives during the PM peak hour at 17:26. Travel to the station is likely to be skewed to morning arrivals and evening departures from the station. RER ridership forecasts shows that almost all riders are expected to use Allandale Waterfront as their home station and long travel times to Union Station mean that trips are less likely to begin in the afternoon or evening. It is therefore assumed that traffic during the traffic peak our represents approximately 30% of the ridership added by the all-day factor.

Weekend ridership projections for RER are not publically available. Metrolinx has released figures for the existing summer weekend services on the Barrie corridor which began operating in 2012. The data shows that these trips average about 1,300 trips daily on the entire corridor. Allandale is assumed to represent about 10% of line ridership, more than its weekday share as weekend travel patterns differ from work day commutes. Of this ridership roughly 20% is estimated to take place during the peak hour as most trips are assumed to occur in the morning or early afternoon. The future estimate was derived by extrapolating the 2.8% annual growth rate of GO Rail and adding an additional 10% growth to account for the increased service frequency following RER implementation.

	Weekend Peak Ho	ur Rider Estimate	
	Total Riders (Daily)	Peak Traffic Hour Riders (1-2pm)	Pick-Up / Drop-Off Trips (20%)
Estimate for Existing	130	26	5
Estimate for Opening Day	178	35	7

All future demand projections are on the high end of the likely range in order to provide a conservative estimate for the purposes of informing the traffic impact study.

# APPENDIX

### E Synchro Reports for Future (2024) Background Traffic Analysis

1: Go Station West Access & Tiffin St. & Lakeshore Dr. Future (2024) Background Traffic-AM

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	eî 👘		ሻ	- <b>†</b> †	1	٦	eî 👘		- ሻ	eî 👘	
Traffic Volume (vph)	106	248	19	1	331	725	26	5	2	453	5	105
Future Volume (vph)	106	248	19	1	331	725	26	5	2	453	5	105
Satd. Flow (prot)	1787	1879	0	1805	3574	1599	1805	1822	0	1770	1581	0
Flt Permitted	0.457			0.587			0.680			0.659		
Satd. Flow (perm)	860	1879	0	1115	3574	1599	1292	1822	0	1228	1581	0
Satd. Flow (RTOR)		5				740					115	
Lane Group Flow (vph)	112	281	0	1	338	740	41	11	0	498	120	0
Turn Type	pm+pt	NA		Perm	NA	Perm	Perm	NA		pm+pt	NA	
Protected Phases	5	2			6			8		7	4	
Permitted Phases	2			6		6	8			4		
Total Split (s)	11.0	47.0		36.0	36.0	36.0	35.0	35.0		18.0	53.0	
Total Lost Time (s)	4.0	6.0		6.0	6.0	6.0	6.0	6.0		4.0	6.0	
Act Effct Green (s)	43.0	41.0		30.0	30.0	30.0	29.0	29.0		49.0	47.0	
Actuated g/C Ratio	0.43	0.41		0.30	0.30	0.30	0.29	0.29		0.49	0.47	
v/c Ratio	0.26	0.36		0.00	0.32	0.74	0.11	0.02		0.74	0.15	
Control Delay	19.1	21.8		25.0	28.1	7.4	27.2	25.7		26.5	3.7	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	19.1	21.8		25.0	28.1	7.4	27.2	25.7		26.5	3.7	
LOS	В	С		С	С	А	С	С		С	А	
Approach Delay		21.0			13.9			26.8			22.1	
Approach LOS		С			В			С			С	
Queue Length 50th (m)	13.6	38.2		0.2	27.9	0.0	6.1	1.6		69.4	0.5	
Queue Length 95th (m)	24.7	59.3		1.4	40.2	32.3	10.2	4.1		101.9	10.0	
Internal Link Dist (m)		44.8			116.0			88.9			104.9	
Turn Bay Length (m)	35.0			23.0		55.0				75.0		
Base Capacity (vph)	434	773		334	1072	997	374	528		677	804	
Starvation Cap Reductn	0	0		0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0		0	0	
Storage Cap Reductn	0	0		0	0	0	0	0		0	0	
Reduced v/c Ratio	0.26	0.36		0.00	0.32	0.74	0.11	0.02		0.74	0.15	
Intersection Summary												
Cycle Length: 100												
Actuated Cycle Length: 100												
Control Type: Semi Act-Unc	coord											
Maximum v/c Ratio: 0.74												
Intersection Signal Delay: 1					itersection							
Intersection Capacity Utiliza	tion 96.8%			IC	CU Level	of Service	e F					
Analysis Period (min) 15												
	<u>.</u>		• <b>T</b> I(1	0								

Splits and Phases: 1: Go Station West Access & Tiffin St. & Lakeshore Dr.

		<b>₽</b> Ø4	
47 s		53 s	
	<b>∮</b> Ø6	Ø7	<b>™</b> ø8
11 s	36 s	18 s	35 s

#### 2: Essa Rd./Bradford St. & Tiffin St.

Future (2024) Background Traffic-AM

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	٦	ef 🕺		٦	<b>↑</b>			4 î b				1
Traffic Volume (vph)	68	233	75	104	216	0	63	365	116	39	241	39
Future Volume (vph)	68	233	75	104	216	0	63	365	116	39	241	39
Satd. Flow (prot)	1770	1775	0	1641	1845	0	0	3397	0	0	3354	1568
Flt Permitted	0.610			0.535				0.847			0.747	
Satd. Flow (perm)	1136	1775	0	924	1845	0	0	2895	0	0	2523	1568
Satd. Flow (RTOR)		21						43				49
Lane Group Flow (vph)	76	342	0	116	240	0	0	639	0	0	354	49
Turn Type	Perm	NA		Perm	NA		Perm	NA		pm+pt	NA	Perm
Protected Phases		4			8			2		1	6	
Permitted Phases	4			8			2			6	6	6
Total Split (s)	41.0	41.0		41.0	41.0		39.0	39.0		10.0	49.0	49.0
Total Lost Time (s)	6.0	6.0		6.0	6.0			6.0			6.0	6.0
Act Effct Green (s)	35.2	35.2		35.2	35.2			19.0			19.0	19.0
Actuated g/C Ratio	0.53	0.53		0.53	0.53			0.29			0.29	0.29
v/c Ratio	0.13	0.36		0.24	0.24			0.74			0.49	0.10
Control Delay	9.9	10.6		11.3	10.2			25.5			21.8	6.0
Queue Delay	0.0	0.0		0.0	0.0			0.0			0.0	0.0
Total Delay	9.9	10.6		11.3	10.2			25.5			21.8	6.0
LOS	А	В		В	В			С			С	A
Approach Delay		10.5			10.5			25.5			19.8	
Approach LOS		В			В			С			В	
Queue Length 50th (m)	4.4	21.4		7.2	15.0			36.3			19.7	0.0
Queue Length 95th (m)	13.1	46.9		20.1	33.6			49.0			26.4	5.1
Internal Link Dist (m)		153.0			41.6			55.6			43.8	
Turn Bay Length (m)	47.0			23.0								89.0
Base Capacity (vph)	603	953		490	980			1471			1646	1040
Starvation Cap Reductn	0	0		0	0			0			0	0
Spillback Cap Reductn	0	0		0	0			0			0	0
Storage Cap Reductn	0	0		0	0			0			0	0
Reduced v/c Ratio	0.13	0.36		0.24	0.24			0.43			0.22	0.05
Intersection Summary												
Cycle Length: 90												
Actuated Cycle Length: 66.2	2											
Control Type: Semi Act-Unc	coord											
Maximum v/c Ratio: 0.74												
Intersection Signal Delay: 1	7.9			In	tersectior	n LOS: B						
Intersection Capacity Utiliza				IC	CU Level o	of Service	E					
Analysis Period (min) 15												

Splits and Phases: 2: Essa Rd./Bradford St. & Tiffin St.

Ø1	<↑ ø₂	<u></u> ø4
10 s	39 s	41 s
\$ ø6		₩ Ø8
49 s		41 s

	-	$\mathbf{r}$	1	-	1	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	Ø4
Lane Configurations	1	1	٦	<b>^</b>	٦	1	
Traffic Volume (vph)	702	1	12	1054	3	12	
Future Volume (vph)	702	1	12	1054	3	12	
Satd. Flow (prot)	1881	1615	1805	1881	1805	1615	
Flt Permitted			0.292		0.950		
Satd. Flow (perm)	1881	1615	555	1881	1805	1615	
Satd. Flow (RTOR)		1				39	
Lane Group Flow (vph)	763	1	13	1098	10	39	
Turn Type	NA	Perm	pm+pt	NA	Prot	Perm	
Protected Phases	2		1	6	8		4
Permitted Phases		2	6			8	
Total Split (s)	53.0	53.0	11.0	64.0	31.0	31.0	31.0
Total Lost Time (s)	6.0	6.0	4.0	6.0	5.0	5.0	
Act Effct Green (s)	64.8	64.8	66.6	67.1	10.0	10.0	
Actuated g/C Ratio	0.81	0.81	0.84	0.84	0.13	0.13	
v/c Ratio	0.50	0.00	0.02	0.69	0.04	0.16	
Control Delay	6.9	4.0	2.4	8.2	31.3	12.8	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	6.9	4.0	2.4	8.2	31.3	12.8	
LOS	А	А	А	А	С	В	
Approach Delay	6.9			8.2	16.5		
Approach LOS	А			А	В		
Queue Length 50th (m)	43.0	0.0	0.4	88.2	1.5	0.0	
Queue Length 95th (m)	114.2	0.6	1.4	147.6	2.0	0.0	
Internal Link Dist (m)	144.3			137.6	30.3		
Turn Bay Length (m)		40.0	70.0				
Base Capacity (vph)	1532	1315	574	1584	590	553	
Starvation Cap Reductn	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	
Reduced v/c Ratio	0.50	0.00	0.02	0.69	0.02	0.07	
Intersection Summary							
Cycle Length: 95							
Actuated Cycle Length: 79.6							
Control Type: Semi Act-Unc	coord						
Maximum v/c Ratio: 0.69							
Intersection Signal Delay: 7.					itersectior		
Intersection Capacity Utiliza	tion 73.0%			IC	CU Level of	of Service	С
Analysis Period (min) 15							

Splits and Phases: 3: Go Station East Access & Lakeshore Dr.

Ø1	₩ Ø2	<b>Å</b> ₽ <sub>Ø4</sub>
11 s	53 s	31 s
₹ø6		<b>√</b> Ø8
64 s		31 s

#### 4: Essa Rd. & Gowan St.

	4	۰.	Ť	1	1	Ļ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		<b>≜</b> †⊳			41
Traffic Volume (vph)	5	205	350	6	158	259
Future Volume (vph)	5	205	350	6	158	259
Satd. Flow (prot)	1616	0	3423	0	0	3342
Flt Permitted	0.999					0.668
Satd. Flow (perm)	1616	0	3423	0	0	2276
Satd. Flow (RTOR)	250		2			
Lane Group Flow (vph)	256	0	429	0	0	435
Turn Type	Prot	Ŭ	NA	Ŭ	pm+pt	NA
Protected Phases	8		2		phi pt 1	6
Permitted Phases	0		2		6	0
Total Split (s)	32.0		87.0		11.0	98.0
Total Lost Time (s)	6.0		6.0		11.0	90.0 6.0
Act Effct Green (s)	0.0 11.0		92.0			92.0
. ,						
Actuated g/C Ratio	0.10		0.80			0.80
v/c Ratio	0.67		0.16			0.24
Control Delay	16.1		2.9			3.3
Queue Delay	0.0		0.0			0.0
Total Delay	16.1		2.9			3.3
LOS	В		А			А
Approach Delay	16.1		2.9			3.3
Approach LOS	В		А			А
Queue Length 50th (m)	1.3		9.1			10.0
Queue Length 95th (m)	18.1		14.7			18.4
Internal Link Dist (m)	120.4		80.7			19.9
Turn Bay Length (m)						
Base Capacity (vph)	558		2738			1820
Starvation Cap Reductn	0		0			0
Spillback Cap Reductn	0		0			0
Storage Cap Reductn	0		0			0
Reduced v/c Ratio	0.46		0.16			0.24
Intersection Summary						
Cycle Length: 130	-					
Actuated Cycle Length: 11						
Control Type: Semi Act-Un	coord					
Maximum v/c Ratio: 0.67						
Intersection Signal Delay: 6					ntersection	
Intersection Capacity Utiliz	ation 156.5%	0		10	CU Level	of Service
Analysis Period (min) 15						
Splits and Dhasos, A: Es	sa Rd. & Go	wan St				
Splits and Phases: 4: Es	isa πu. α GU	iwan St.				



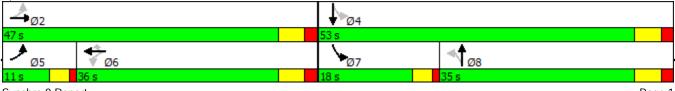
5: Property Access/Go Station South Access & Essa Rd. Future (2024) Background Traffic-AM
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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		÷						4î b			4î b	
Traffic Volume (veh/h)	10	0	15	0	0	0	16	534	6	12	402	6
Future Volume (Veh/h)	10	0	15	0	0	0	16	534	6	12	402	6
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.79	0.92	0.79	0.92	0.92	0.92	0.84	0.84	0.92	0.92	0.93	0.93
Hourly flow rate (vph)	13	0	19	0	0	0	19	636	7	13	432	6
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)								44			80	
pX, platoon unblocked	0.96	0.96	0.95	0.96	0.96	0.98	0.95			0.98		
vC, conflicting volume	817	1142	219	938	1142	322	438			643		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	624	961	81	750	961	255	311			585		
tC, single (s)	7.5	6.5	6.9	7.5	6.5	6.9	4.2			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.3			2.2		_
p0 queue free %	96	100	98	100	100	100	98			99		
cM capacity (veh/h)	352	242	924	277	239	726	1154			976		
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2							
Volume Total	32	337	325	229	222							
Volume Left	13	19	0	13	0							
Volume Right	19	0	7	0	6							
cSH	557	1154	1700	976	1700							
Volume to Capacity	0.06	0.02	0.19	0.01	0.13							
Queue Length 95th (m)	1.5	0.4	0.0	0.3	0.0							
Control Delay (s)	11.9	0.6	0.0	0.6	0.0							
Lane LOS	В	А		А								
Approach Delay (s)	11.9	0.3		0.3								
Approach LOS	В											
Intersection Summary												
Average Delay			0.6									
Intersection Capacity Utiliza	ition		36.5%	IC	U Level o	of Service			А			
Analysis Period (min)			15									

1: Go Station West Access & Tiffin St. & Lakeshore Dr. Future (2024) Background Traffic-PM

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	<u> </u>	4		ሻ	<u></u>	1	- ሽ	€Î →			4î 🚽	
Traffic Volume (vph)	159	475	27	2	283	569	34	5	2	706	37	122
Future Volume (vph)	159	475	27	2	283	569	34	5	2	706	37	122
Satd. Flow (prot)	1805	1885	0	1805	3610	1599	1031	1814	0	1805	1669	0
Flt Permitted	0.491	100E	0	0.380	2410	1500	0.652	1014	0	0.660	1440	0
Satd. Flow (perm) Satd. Flow (RTOR)	933	1885 3	0	722	3610	1599 599	708	1814	0	1254	1669 127	U
Lane Group Flow (vph)	161	5 507	0	2	298	599	51	10	0	735	127	0
Turn Type	pm+pt	NA	U	Perm	NA	Perm	Perm	NA	0	pm+pt	NA	0
Protected Phases	5	2		I CIIII	6	1 cmi	I CIIII	8		рш+рt 7	4	
Permitted Phases	2	L		6	0	6	8	U		4	т	
Total Split (s)	11.0	47.0		36.0	36.0	36.0	35.0	35.0		18.0	53.0	
Total Lost Time (s)	4.0	6.0		6.0	6.0	6.0	6.0	6.0		4.0	6.0	
Act Effct Green (s)	43.0	41.0		30.0	30.0	30.0	29.0	29.0		49.0	47.0	
Actuated g/C Ratio	0.43	0.41		0.30	0.30	0.30	0.29	0.29		0.49	0.47	
v/c Ratio	0.35	0.66		0.01	0.28	0.67	0.25	0.02		1.06	0.20	
Control Delay	20.3	28.6		25.0	27.6	6.5	31.2	25.6		77.6	5.2	
Queue Delay	0.0	6.5		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	20.3	35.1		25.0	27.6	6.5	31.2	25.6		77.6	5.2	
LOS	С	D		С	С	А	С	С		E	А	
Approach Delay		31.5			13.5			30.3			64.3	
Approach LOS	20.0	C		0.0	В	0.0	0.0	C		100 5	E	
Queue Length 50th (m)	20.0	81.1		0.3	24.3	0.0	8.0	1.5		~138.5	4.2	
Queue Length 95th (m)	34.0	117.9 44.8		2.0	35.6 116.0	27.7	13.2	4.0 88.9		#237.9	15.6 104.9	
Internal Link Dist (m) Turn Bay Length (m)	35.0	44.8		23.0	110.0	55.0		88.9		75.0	104.9	
Base Capacity (vph)	462	774		23.0	1083	899	205	526		691	851	
Starvation Cap Reductn	402	214		0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0		0	0	
Storage Cap Reductn	0	0		0	0	0	0	0		0	0	
Reduced v/c Ratio	0.35	0.91		0.01	0.28	0.67	0.25	0.02		1.06	0.20	
Intersection Summary												
Cycle Length: 100												_
Actuated Cycle Length: 100												
Control Type: Semi Act-Uno	coord											_
Maximum v/c Ratio: 1.06	4.0			In	torcostio							
Intersection Signal Delay: 3 Intersection Capacity Utiliza		0/			tersection	of Service	ц					
Analysis Period (min) 15		/0		IC			: П					
<ul> <li>Volume exceeds capac</li> </ul>	ity <u>aueue</u> is	s theoretic	ally infini	te								
Queue shown is maximu			any mini	10.								
<ul><li># 95th percentile volume</li></ul>			eue mav	be longe	r.							
Queue shown is maximu												
Solits and Phases: 1. Go	Station We	oct Accord	. 9. Tiffin	Ct & Lok	oshoro D	r						

Splits and Phases: 1: Go Station West Access & Tiffin St. & Lakeshore Dr.



Synchro 9 Report

#### 2: Essa Rd./Bradford St. & Tiffin St.

Future (2024) Background Traffic-PM

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	۳	4Î		٦.	<b>↑</b>			4 î b				1
Traffic Volume (vph)	46	362	90	130	207	0	91	493	168	138	449	70
Future Volume (vph)	46	362	90	130	207	0	91	493	168	138	449	70
Satd. Flow (prot)	1752	1814	0	1687	1863	0	0	3431	0	0	3461	1599
Flt Permitted	0.614			0.353				0.718			0.581	
Satd. Flow (perm)	1133	1814	0	627	1863	0	0	2479	0	0	2035	1599
Satd. Flow (RTOR)		16						48				80
Lane Group Flow (vph)	48	476	0	141	225	0	0	800	0	0	667	80
Turn Type	Perm	NA		Perm	NA		Perm	NA		pm+pt	NA	Perm
Protected Phases		4			8			2		1	6	
Permitted Phases	4			8			2			6	6	6
Total Split (s)	40.0	40.0		40.0	40.0		40.0	40.0		10.0	50.0	50.0
Total Lost Time (s)	6.0	6.0		6.0	6.0			6.0			6.0	6.0
Act Effct Green (s)	34.4	34.4		34.4	34.4			31.6			31.6	31.6
Actuated g/C Ratio	0.44	0.44		0.44	0.44			0.40			0.40	0.40
v/c Ratio	0.10	0.59		0.51	0.27			0.78			0.81	0.12
Control Delay	16.5	21.6		27.2	17.1			24.2			28.9	3.7
Queue Delay	0.0	0.0		0.0	0.0			0.2			0.0	0.0
Total Delay	16.5	21.6		27.2	17.1			24.4			28.9	3.7
LOS	В	С		С	В			С			С	А
Approach Delay		21.1			21.0			24.4			26.2	
Approach LOS		С			С			С			С	
Queue Length 50th (m)	4.3	52.5		15.4	21.7			51.5			46.4	0.0
Queue Length 95th (m)	13.3	105.0		#43.6	46.8			72.2			64.8	6.7
Internal Link Dist (m)		153.0			41.6			55.6			43.8	
Turn Bay Length (m)	47.0			23.0								89.0
Base Capacity (vph)	499	807		275	820			1192			1159	945
Starvation Cap Reductn	0	0		0	0			49			0	0
Spillback Cap Reductn	0	0		0	0			0			0	0
Storage Cap Reductn	0	0		0	0			0			0	0
Reduced v/c Ratio	0.10	0.59		0.51	0.27			0.70			0.58	0.08
Intersection Summary												
Cycle Length: 90												
Actuated Cycle Length: 78.1	1											
Control Type: Semi Act-Unc	coord											
Maximum v/c Ratio: 0.81												
Intersection Signal Delay: 2	3.7			In	tersectior	n LOS: C						
Intersection Capacity Utiliza	ition 102.69	%		IC	CU Level	of Service	G					
Analysis Period (min) 15												
# 95th percentile volume e	exceeds ca	pacity, qu	eue may	be longer	r.							
Queue shown is maximu	im after two	o cycles.										
Splits and Phases: 2: Ess	sa Rd./Brac	lford St Ø	Tiffin St									
	a nu./Dial		TIIIII JI.			1.4						

Ø1	dØ2	A <sub>04</sub>
10 s	40 s	40 s
\$ <u>06</u>		₹Ø8
50 s		40 s

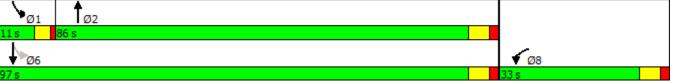
	-	$\mathbf{r}$	1	-	1	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	Ø4
Lane Configurations	<b>^</b>	1	۲	<u></u>	1	1	
Traffic Volume (vph)	1180	3	6	851	2	15	
Future Volume (vph)	1180	3	6	851	2	15	
Satd. Flow (prot)	1900	1615	1805	1900	1805	1615	
Flt Permitted			0.106		0.950		
Satd. Flow (perm)	1900	1615	201	1900	1805	1615	
Satd. Flow (RTOR)		1				20	
Lane Group Flow (vph)	1204	3	6	886	3	20	
Turn Type	NA	Perm	pm+pt	NA	Prot	Perm	
Protected Phases	2		1	6	8		4
Permitted Phases		2	6			8	
Total Split (s)	53.0	53.0	11.0	64.0	31.0	31.0	31.0
Total Lost Time (s)	6.0	6.0	4.0	6.0	5.0	5.0	
Act Effct Green (s)	69.9	69.9	70.4	72.1	10.0	10.0	
Actuated g/C Ratio	0.87	0.87	0.88	0.90	0.12	0.12	
v/c Ratio	0.73	0.00	0.02	0.52	0.01	0.09	
Control Delay	11.4	4.3	2.2	4.1	31.5	15.3	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	11.4	4.3	2.2	4.1	31.5	15.3	
LOS	В	А	А	А	С	В	
Approach Delay	11.4			4.1	17.4		
Approach LOS	В			А	В		
Queue Length 50th (m)	0.0	0.0	0.1	0.0	0.4	0.0	
Queue Length 95th (m)	#288.5	1.1	0.9	86.5	2.4	4.7	
Internal Link Dist (m)	144.3			137.6	30.3		
Turn Bay Length (m)		40.0	70.0				
Base Capacity (vph)	1653	1405	315	1706	585	536	
Starvation Cap Reductn	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	
Reduced v/c Ratio	0.73	0.00	0.02	0.52	0.01	0.04	
Intersection Summary							
Cycle Length: 95							
Actuated Cycle Length: 80.	3						
Control Type: Semi Act-Un	coord						
Maximum v/c Ratio: 0.73							
Intersection Signal Delay: 8					tersection		_
Intersection Capacity Utiliza	ation 79.6%			IC	CU Level	of Service	D
Analysis Period (min) 15							
# 95th percentile volume			leue may	be longe	r.		
Queue shown is maximi	um after two	cycles.					

Splits and Phases: 3: Go Station East Access & Lakeshore Dr.

Ø1	₩ Ø2	
11 s	53 s	31 s
₩ø6		<b>1</b> ï8
64 s		31 s

#### 4: Essa Rd. & Gowan St.

	<	*	1	1	1	Ŧ		
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT		
Lane Configurations	Y		<b>∱</b> ⊅					
Traffic Volume (vph)	7	320	456	10	241	438		
Future Volume (vph)	7	320	456	10	241	438		
Satd. Flow (prot)	1632	0	3497	0	0	3344		
Flt Permitted	0.999					0.642		
Satd. Flow (perm)	1632	0	3497	0	0	2184		
Satd. Flow (RTOR)	416		3					
Lane Group Flow (vph)	425	0	542	0	0	789		
Turn Type	Prot		NA		pm+pt	NA		
Protected Phases	8		2		1	6		
Permitted Phases				6				
Total Split (s)	33.0		86.0		11.0	97.0		
Total Lost Time (s)	6.0		6.0			6.0		
Act Effct Green (s)	12.2		91.1			91.1		
Actuated g/C Ratio	0.11		0.79			0.79		
v/c Ratio	0.78		0.20			0.46		
Control Delay	16.0		3.4			5.3		
Queue Delay	0.0		0.0			0.8		
Total Delay	16.0		3.4			6.2		
LOS	В		А			A		
Approach Delay	16.0		3.4			6.2		
Approach LOS	В		A			A		
Queue Length 50th (m)	2.0		11.9			23.2		
Queue Length 95th (m)	11.6		24.0			46.2		
Internal Link Dist (m) 120.4			80.7			19.9		
Turn Bay Length (m)		27/2			1705			
Base Capacity (vph)	700		2762			1725		
Starvation Cap Reductn	0		0			590		
Spillback Cap Reductn Storage Cap Reductn	0 0		0			0 0		
Reduced v/c Ratio	0.61		0.20			0.70		
	0.01		0.20			0.70		
Intersection Summary								
Cycle Length: 130								
Actuated Cycle Length: 115.3								
Control Type: Semi Act-Uno	coord							
Maximum v/c Ratio: 0.78								
Intersection Signal Delay: 7.7					tersectior			
Intersection Capacity Utiliza		10	CU Level o	of Service	H			
Analysis Period (min) 15								
Splits and Phases: 4: Essa Rd. & Gowan St.								



	5: Property Access/Go	Station South Access	& Essa Rd. Future	(2024) Background Traffic-PM
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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$						ፋጉ			4î»	
Traffic Volume (veh/h)	5	0	26	0	0	0	20	748	8	12	654	2
Future Volume (Veh/h)	5	0	26	0	0	0	20	748	8	12	654	2
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.68	0.68	0.68	1.00	1.00	1.00	0.95	0.95	0.95	0.90	0.90	0.90
Hourly flow rate (vph)	7	0	38	0	0	0	21	787	8	13	727	2
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)								44			80	
pX, platoon unblocked	0.90	0.90	0.88	0.90	0.90	0.97	0.88			0.97		
vC, conflicting volume	1190	1591	364	1260	1588	398	729			795		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	834	1280	21	913	1277	314	433			725		
tC, single (s)	7.5	6.5	6.9	7.5	6.5	6.9	4.1			4.1		_
tC, 2 stage (s)	0.5											
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		_
p0 queue free %	97	100	96	100	100	100	98			98		
cM capacity (veh/h)	231	145	936	195	146	666	1006			860		
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2							
Volume Total	45	414	402	376	366							
Volume Left	7	21	0	13	0							
Volume Right	38	0	8	0	2							
cSH	635	1006	1700	860	1700							
Volume to Capacity	0.07	0.02	0.24	0.02	0.21							
Queue Length 95th (m)	1.8	0.5	0.0	0.4	0.0							
Control Delay (s)	11.1	0.7	0.0	0.5	0.0							
Lane LOS	В	А		А								
Approach Delay (s)	11.1	0.3		0.2								
Approach LOS	В											
Intersection Summary												
Average Delay			0.6									
	Intersection Capacity Utilization		45.3%	ICU Level of Service A								
Analysis Period (min)			15									

1: Go Station West Access & Tiffin St. & Lakeshore Parture (2024) Background Traffic-Weekend

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	eî 👘		<u>۲</u>	- <b>†</b> †	1	ሻ	eî 👘		٦	eî 👘	
Traffic Volume (vph)	177	291	9	6	211	515	26	2	7	527	12	163
Future Volume (vph)	177	291	9	6	211	515	26	2	7	527	12	163
Satd. Flow (prot)	1805	1874	0	1805	3574	1615	1101	1682	0	1787	1634	0
Flt Permitted	0.545			0.569			0.639		-	0.658		
Satd. Flow (perm)	1036	1874	0	1081	3574	1615	740	1682	0	1238	1634	0
Satd. Flow (RTOR)	10/	2	0	1	015	526	07	10	0	F / 7	175	0
Lane Group Flow (vph)	186	315	0	6	215	526	27 Derre	13	0	567	188	0
Turn Type	pm+pt	NA		Perm	NA	Perm	Perm	NA		pm+pt	NA	
Protected Phases Permitted Phases	5 2	2		L	6	4	8	8		7	4	
Total Split (s)	2 11.0	47.0		6 36.0	36.0	6 36.0	35.0	35.0		4	53.0	
Total Lost Time (s)	4.0	47.0 6.0		50.0 6.0	50.0 6.0	6.0	6.0	6.0		4.0	6.0	
Act Effct Green (s)	4.0	41.0		30.0	30.0	30.0	29.0	29.0		4.0	47.0	
Actuated g/C Ratio	0.43	0.41		0.30	0.30	0.30	0.29	0.29		0.49	0.47	
v/c Ratio	0.37	0.41		0.02	0.20	0.62	0.13	0.03		0.83	0.22	
Control Delay	20.7	22.8		25.0	26.7	6.0	28.2	25.8		32.7	3.6	
Queue Delay	0.0	1.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	20.7	23.8		25.0	26.7	6.0	28.2	25.8		32.7	3.6	
LOS	С	С		С	С	А	С	С		С	А	
Approach Delay		22.7			12.1			27.4			25.4	
Approach LOS		С			В			С			С	
Queue Length 50th (m)	23.5	44.3		0.9	17.1	0.0	4.1	1.9		83.1	1.4	
Queue Length 95th (m)	39.0	67.6		3.9	26.6	24.9	11.2	5.3		#133.9	13.0	
Internal Link Dist (m)		44.8			116.0			88.9			104.9	
Turn Bay Length (m)	35.0			23.0		55.0				75.0		
Base Capacity (vph)	499	769		324	1072	852	214	487		683	860	
Starvation Cap Reductn	0	242		0	0	0	0	0		0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0		0	0	_
Storage Cap Reductn	0	0		0	0	0	0	0		0	0	
Reduced v/c Ratio	0.37	0.60		0.02	0.20	0.62	0.13	0.03		0.83	0.22	
Intersection Summary												
Cycle Length: 100												
Actuated Cycle Length: 100												
Control Type: Semi Act-Unc	coord											_
Maximum v/c Ratio: 0.83	0.0			ما	toroootion							
Intersection Signal Delay: 1		1			tersection							
Intersection Capacity Utiliza Analysis Period (min) 15	111011 100.97	0		IC	O Level (		G					
# 95th percentile volume	avraads ca	nacity ou	elle mav	he longo	r –							
Queue shown is maximu			cue may	be lunger								

Splits and Phases: 1: Go Station West Access & Tiffin St. & Lakeshore Dr.

ø₂	▼Ø4
47 s	53 s
▶ <sub>Ø5</sub> ♥ <sub>Ø6</sub>	▶ø7 <b>1</b> ø8
11 s 36 s	18 s 35 s

Future (2024) Background Traffic-Weekend

	≯	→	$\mathbf{\hat{z}}$	4	←	•	•	Ť	1	1	Ļ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	4		ሻ	<b>↑</b>			ፋት				1
Traffic Volume (vph)	29	234	63	170	153	0	69	402	203	51	233	43
Future Volume (vph)	29	234	63	170	153	0	69	402	203	51	233	43
Satd. Flow (prot)	1736	1810	0	1736	1863	0	0	3410	0	0	3464	1615
Flt Permitted	0.644			0.538				0.864			0.665	
Satd. Flow (perm)	1177	1810	0	983	1863	0	0	2961	0	0	2324	1615
Satd. Flow (RTOR)		18						83				48
Lane Group Flow (vph)	32	330	0	200	180	0	0	750	0	0	316	48
Turn Type	Perm	NA		Perm	NA		Perm	NA		pm+pt	NA	Perm
Protected Phases		4			8			2		1	6	
Permitted Phases	4			8			2			6	6	6
Total Split (s)	42.0	42.0		42.0	42.0		38.0	38.0		10.0	48.0	48.0
Total Lost Time (s)	6.0	6.0		6.0	6.0			6.0			6.0	6.0
Act Effct Green (s)	36.2	36.2		36.2	36.2			21.5			21.5	21.5
Actuated g/C Ratio	0.52	0.52		0.52	0.52			0.31			0.31	0.31
v/c Ratio	0.05	0.35		0.39	0.19			0.77			0.44	0.09
Control Delay	10.6	11.7		14.6	10.9			25.1			21.0	5.7
Queue Delay	0.0	0.0		0.0	0.0			0.0			0.0	0.0
Total Delay	10.6	11.7		14.6	10.9			25.1			21.0	5.7
LOS	В	В		В	В			С			С	A
Approach Delay		11.6			12.9			25.1			19.0	
Approach LOS		В			В			С			В	
Queue Length 50th (m)	2.0	23.3		15.5	12.3			43.1			18.0	0.0
Queue Length 95th (m)	7.5	50.1		35.4	26.6			61.8			28.4	6.3
Internal Link Dist (m)		153.0			41.6			55.6			43.8	
Turn Bay Length (m)	47.0			23.0								89.0
Base Capacity (vph)	610	947		509	966			1409			1405	995
Starvation Cap Reductn	0	0		0	0			7			0	0
Spillback Cap Reductn	0	0		0	0			0			0	0
Storage Cap Reductn	0	0		0	0			0			0	0
Reduced v/c Ratio	0.05	0.35		0.39	0.19			0.53			0.22	0.05
Intersection Summary												
Cycle Length: 90												
Actuated Cycle Length: 69.8	3											
Control Type: Semi Act-Unc	coord											
Maximum v/c Ratio: 0.77												
Intersection Signal Delay: 18	8.8			In	tersectior	n LOS: B						
Intersection Capacity Utiliza	tion 87.9%			IC	CU Level of	of Service	E					
Analysis Period (min) 15												

Splits and Phases: 2: Essa Rd./Bradford St. & Tiffin St.

Ø1	↑ ø 2	<u></u> ø₄	
10 s	38 s	42 s	
\$ ø6		₩ Ø8	
48 s		42 s	

	-	$\mathbf{r}$	1	-	1	1		
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	Ø4	
Lane Configurations	<b>†</b>	1	٦	<b>↑</b>	7	1		
Traffic Volume (vph)	822	2	3	726	5	9		
Future Volume (vph)	822	2	3	726	5	9		
Satd. Flow (prot)	1881	1615	1805	1881	1805	1615		
Flt Permitted			0.246		0.950			
Satd. Flow (perm)	1881	1615	467	1881	1805	1615		
Satd. Flow (RTOR)		1				15		
Lane Group Flow (vph)	893	2	3	756	8	15		
Turn Type	NA	Perm	pm+pt	NA	Prot	Perm		
Protected Phases	2		1	6	8		4	
Permitted Phases		2	6			8		
Total Split (s)	53.0	53.0	11.0	64.0	31.0	31.0	31.0	
Total Lost Time (s)	6.0	6.0	4.0	6.0	5.0	5.0		
Act Effct Green (s)	70.3	70.3	70.9	72.5	10.0	10.0		
Actuated g/C Ratio	0.87	0.87	0.88	0.90	0.12	0.12		
v/c Ratio	0.55	0.00	0.01	0.45	0.04	0.07		
Control Delay	6.9	4.5	2.0	3.4	32.2	16.8		
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0		
Total Delay	6.9	4.5	2.0	3.4	32.2	16.8		
LOS	А	А	А	А	С	В		
Approach Delay	6.9			3.4	22.1			
Approach LOS	А			А	С			
Queue Length 50th (m)	0.0	0.0	0.0	0.0	1.1	0.0		
Queue Length 95th (m)	154.3	0.9	0.6	64.6	3.4	2.8		
Internal Link Dist (m)	144.3			137.6	30.3			
Turn Bay Length (m)		40.0	70.0					
Base Capacity (vph)	1638	1407	526	1691	582	531		
Starvation Cap Reductn	0	0	0	0	0	0		
Spillback Cap Reductn	0	0	0	0	0	0		
Storage Cap Reductn	0	0	0	0	0	0		
Reduced v/c Ratio	0.55	0.00	0.01	0.45	0.01	0.03		
Intersection Summary								
Cycle Length: 95								
Actuated Cycle Length: 80.7								
Control Type: Semi Act-Unc	coord							
Maximum v/c Ratio: 0.55								
Intersection Signal Delay: 5.				In	ntersectior	n LOS: A		
Intersection Capacity Utiliza	tion 60.8%			IC	CU Level of	of Service	В	
Analysis Period (min) 15								

Splits and Phases: 3: Go Station East Access & Lakeshore Dr.

Ø1	₩ Ø2	<b>Å</b> ₽ <sub>Ø4</sub>
11 s	53 s	31 s
₹ø6		<b>√</b> Ø8
64 s		31 s

## 4: Essa Rd. & Gowan St.

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		<b>≜</b> †⊅			4†
Traffic Volume (vph)	11	259	440	8	131	348
Future Volume (vph)	11	259	440	8	131	348
Satd. Flow (prot)	1650	0	3530	0	0	3484
Flt Permitted	0.998					0.674
Satd. Flow (perm)	1650	0	3530	0	0	2381
Satd. Flow (RTOR)	316	-	3	-	-	
Lane Group Flow (vph)	329	0	540	0	0	551
Turn Type	Prot	U	NA	U	pm+pt	NA
Protected Phases	8		2		- pm+pt 1	6
Permitted Phases	U		2		6	U
Total Split (s)	31.0		88.0		11.0	99.0
					11.0	
Total Lost Time (s)	6.0		6.0			6.0
Act Effct Green (s)	11.6		93.1			93.1
Actuated g/C Ratio	0.10		0.80			0.80
v/c Ratio	0.73		0.19			0.29
Control Delay	16.8		3.1			3.8
Queue Delay	0.0		0.0			0.5
Total Delay	16.8		3.1			4.2
LOS	В		А			А
Approach Delay	16.8		3.1			4.2
Approach LOS	В		А			А
Queue Length 50th (m)	2.9		11.8			13.4
Queue Length 95th (m)	20.2		20.6			25.2
Internal Link Dist (m)	120.4		80.7			19.9
Turn Bay Length (m)						
Base Capacity (vph)	602		2816			1899
Starvation Cap Reductn	0		0			858
Spillback Cap Reductn	0		0			0
Storage Cap Reductn	0		0			0
Reduced v/c Ratio	0.55		0.19			0.53
	0.00		0.17			0.00
Intersection Summary						
Cycle Length: 130						
Actuated Cycle Length: 11						
Control Type: Semi Act-Un	coord					
Maximum v/c Ratio: 0.73						
Intersection Signal Delay: 6	6.7			Ir	ntersection	n LOS: A
Intersection Capacity Utiliz	ation 136.8%	6		10	CU Level	of Service
Analysis Period (min) 15						
Splits and Phases: 4: Es	sa Rd. & Go	wan St.				



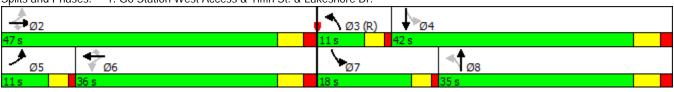
5: Property Access/Go Station South Access & Essa Rdre (2024) Background Traffic-Weekend

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$						4î b			đ ĥ	
Traffic Volume (veh/h)	10	0	37	0	0	0	30	663	6	10	442	15
Future Volume (Veh/h)	10	0	37	0	0	0	30	663	6	10	442	15
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.82	0.82	0.82	1.00	1.00	1.00	0.84	0.84	0.84	0.86	0.86	0.86
Hourly flow rate (vph)	12	0	45	0	0	0	36	789	7	12	514	17
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)								44			80	
pX, platoon unblocked	0.97	0.97	0.96	0.97	0.97	0.97	0.96			0.97		
vC, conflicting volume	1013	1414	266	1190	1420	398	531			796		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												_
vCu, unblocked vol	802	1214	140	984	1220	307	418			719		
tC, single (s)	7.5	6.5	6.9	7.5	6.5	6.9	4.1			4.1		_
tC, 2 stage (s)	0.5											
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	95	100	95	100	100	100	97			99		
cM capacity (veh/h)	262	170	850	183	169	671	1102			862		
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2							
Volume Total	57	430	402	269	274							
Volume Left	12	36	0	12	0							
Volume Right	45	0	7	0	17							
cSH	577	1102	1700	862	1700							
Volume to Capacity	0.10	0.03	0.24	0.01	0.16							
Queue Length 95th (m)	2.6	0.8	0.0	0.3	0.0							
Control Delay (s)	11.9	1.0	0.0	0.6	0.0							
Lane LOS	В	А		А								
Approach Delay (s)	11.9	0.5		0.3								
Approach LOS	В											
Intersection Summary												
Average Delay			0.9									
Intersection Capacity Utilizat	on		45.7%	IC	U Level o	of Service			А			
Analysis Period (min)			15									

# APPENDIX

# F Synchro Reports for Future (2024) Total Traffic Analysis

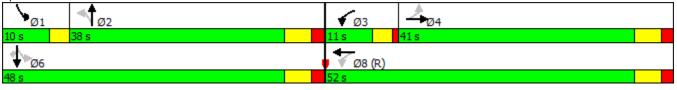
1: Go Station West Access & Tiffin St. & Lakeshore Dr. Future (2024) Total Traf											l Traffie	c-AM
	٦	-	$\mathbf{F}$	∢	←	•	•	Ť	۲	1	ŧ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ľ	•	1	ľ	•	1	ľ	el el		ľ	et.	
Traffic Volume (vph)	106	267	34	0	344	730	34	0	0	458	0	105
Future Volume (vph)	106	267	34	0	344	730	34	0	0	458	0	105
Satd. Flow (prot)	1787	1900	808	950	1881	1599	902	950	0	1770	1568	0
Flt Permitted	0.308						0.683			0.665		
Satd. Flow (perm)	579	1900	808	950	1881	1599	649	950	0	1239	1568	0
Satd. Flow (RTOR)			120			745					473	
Lane Group Flow (vph)	112	281	36	0	351	745	53	0	0	503	115	0
Turn Type	pm+pt	NA	Perm	Perm	NA	Perm	pm+pt			pm+pt	NA	
Protected Phases	5	2			6		3	8		7	4	
Permitted Phases	2		2	6		6	8			4		
Total Split (s)	11.0	47.0	47.0	36.0	36.0	36.0	11.0	35.0		18.0	42.0	
Total Lost Time (s)	4.0	6.0	6.0	6.0	6.0	6.0	4.0	6.0		4.0	6.0	
Act Effct Green (s)	43.0	41.0	41.0		30.0	30.0	38.0			49.0	36.0	
Actuated g/C Ratio	0.43	0.41	0.41		0.30	0.30	0.38			0.49	0.36	
v/c Ratio	0.34	0.36	0.09		0.62	0.74	0.20			0.74	0.13	
Control Delay	30.5	36.4	1.6		35.9	7.5	16.7			26.7	0.3	
Queue Delay	0.0	0.0	0.0		0.0	0.0	0.0			0.0	0.0	
Total Delay	30.5	36.4	1.6		35.9	7.5	16.7			26.7	0.3	
LOS	С	D	А		D	А	В			С	А	
Approach Delay		31.9			16.6			16.7			21.8	
Approach LOS	00 5	С			В		- /	В		70.4	С	
Queue Length 50th (m)	20.5	59.3	0.0		61.4	0.0	5.6			70.4	0.0	_
Queue Length 95th (m)	m33.6	m85.6	m0.3		92.3	32.7	8.8	00.0		103.4	0.0	
Internal Link Dist (m)	25.0	44.8			116.0			88.9			104.9	_
Turn Bay Length (m)	35.0	770	25.0		F ( 4	55.0	274			75.0	0/7	
Base Capacity (vph)	333	779	402		564	1001	264			681	867	
Starvation Cap Reductn	0	0 0	0 0		0	0 0	0 0			0	0 0	
Spillback Cap Reductn Storage Cap Reductn	0	0	0		0 0	0	0			0 0	0	
Reduced v/c Ratio	0.34	0.36	0.09		0.62	0.74	0.20			0.74	0.13	
Intersection Summary												
Cycle Length: 100												
Actuated Cycle Length: 100												
Offset: 28 (28%), Reference		3 NBL S	Start of Gr	een								
Control Type: Actuated-Coo				0011								
Maximum v/c Ratio: 0.74	. an latou											
Intersection Signal Delay: 2	1.0			In	tersection	n LOS: C						
Intersection Capacity Utiliza		)			U Level							
Analysis Period (min) 15												
m Volume for 95th percen	tile queue	is metere	d by upsti	ream sign	al.							
Solits and Phases 1. Co.	Station W/	ost Arros	s & Tiffin	St & Lak	oshora D	r						
	Splits and Phases: 1: Go Station West Access & Tiffin St. & Lakeshore Dr.											



Future (2024) Total Traffic-AM

	≯	-	$\mathbf{i}$	4	←	•	1	Ť	1	1	Ŧ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	۲	et 🗧		۲	<b>†</b>			4î»				1
Traffic Volume (vph)	68	245	73	110	226	0	63	363	128	49	229	39
Future Volume (vph)	68	245	73	110	226	0	63	363	128	49	229	39
Satd. Flow (prot)	1770	1781	0	1641	1845	0	0	3388	0	0	3356	1568
Flt Permitted	0.603			0.354				0.806			0.620	
Satd. Flow (perm)	1123	1781	0	611	1845	0	0	2748	0	0	2100	1568
Satd. Flow (RTOR)		16						42				76
Lane Group Flow (vph)	76	353	0	122	251	0	0	652	0	0	352	49
Turn Type	Perm	NA		pm+pt	NA		Perm	NA		pm+pt	NA	Perm
Protected Phases		4		3	8			2		1	6	
Permitted Phases	4			8			2			6	6	6
Total Split (s)	41.0	41.0		11.0	52.0		38.0	38.0		10.0	48.0	48.0
Total Lost Time (s)	6.0	6.0		4.0	6.0			6.0			6.0	6.0
Act Effct Green (s)	35.0	35.0		63.2	61.2			26.8			26.8	26.8
Actuated g/C Ratio	0.35	0.35		0.63	0.61			0.27			0.27	0.27
v/c Ratio	0.19	0.56		0.20	0.22			0.85			0.63	0.10
Control Delay	24.4	29.1		3.5	3.4			43.3			36.7	2.9
Queue Delay	0.0	0.0		0.0	0.0			0.1			0.0	0.0
Total Delay	24.4	29.1		3.5	3.4			43.4			36.7	2.9
LOS	С	С		А	A			D			D	А
Approach Delay		28.2			3.4			43.4			32.6	
Approach LOS	10 7	С		0.1	A			D			С	0.0
Queue Length 50th (m)	10.7	54.9		3.1	6.8			62.5			33.3	0.0
Queue Length 95th (m)	22.0	84.0		m6.2	11.8			71.0			37.6	2.3
Internal Link Dist (m)	47.0	153.0		23.0	41.6			55.6			43.8	00.0
Turn Bay Length (m)	47.0 393	633		23.0 614	1129			915			882	89.0 702
Base Capacity (vph)	393 0	033		014	0			12			0	
Starvation Cap Reductn Spillback Cap Reductn	0	0			0			0				0
Storage Cap Reductin	0	0		0	0			0			0 0	0 0
Reduced v/c Ratio	0.19	0.56		0.20	0.22			0.72			0.40	0.07
	0.19	0.50		0.20	0.22			0.72			0.40	0.07
Intersection Summary												
Cycle Length: 100												
Actuated Cycle Length: 100			ort of Cr	oon Moot	or Intoroa	ation						
Offset: 0 (0%), Referenced to		WBIL, SI	artorg	een, wasi	er merse	ection						
Control Type: Actuated-Coo Maximum v/c Ratio: 0.85	rumateu											
	0 5			In	torcoction							
Intersection Signal Delay: 29 Intersection Capacity Utiliza					tersection	of Service	F					
Analysis Period (min) 15						JI Selvice	E					
m Volume for 95th percentile queue is metered by upstream signal.												

Splits and Phases: 2: Essa Rd./Bradford St. & Tiffin St.



	-	$\mathbf{r}$	1	+	1	1			
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	Ø4		
Lane Configurations	<b>†</b>	1	٦	1	7	1			
Traffic Volume (vph)	700	25	13	1053	21	14			
Future Volume (vph)	700	25	13	1053	21	14			
Satd. Flow (prot)	1881	1615	1805	1881	1805	1615			
Flt Permitted			0.280		0.950				
Satd. Flow (perm)	1881	1615	532	1881	1805	1615			
Satd. Flow (RTOR)		16				45			
Lane Group Flow (vph)	761	27	14	1097	68	45			
Turn Type	NA	Perm	pm+pt	NA	Prot	Perm			
Protected Phases	2		1	6	8		4		
Permitted Phases		2	6			8			
Total Split (s)	53.0	53.0	11.0	64.0	31.0	31.0	31.0		
Total Lost Time (s)	6.0	6.0	4.0	6.0	5.0	5.0			
Act Effct Green (s)	60.0	60.0	63.0	62.2	10.2	10.2			
Actuated g/C Ratio	0.76	0.76	0.80	0.79	0.13	0.13			
v/c Ratio	0.53	0.02	0.03	0.74	0.29	0.18			
Control Delay	8.1	3.4	2.5	10.5	34.9	12.2			
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0			
Total Delay	8.1	3.4	2.5	10.5	34.9	12.2			
LOS	А	А	А	В	С	В			
Approach Delay	7.9			10.4	25.9				
Approach LOS	А			В	С				
Queue Length 50th (m)	42.7	0.4	0.4	87.9	9.9	0.0			
Queue Length 95th (m)	118.0	3.8	1.6	157.2	7.1	0.0			
Internal Link Dist (m)	144.3			137.6	30.3				
Turn Bay Length (m)		40.0	70.0						
Base Capacity (vph)	1425	1227	535	1477	592	560			
Starvation Cap Reductn	0	0	0	0	0	0			
Spillback Cap Reductn	0	0	0	0	0	0			
Storage Cap Reductn	0	0	0	0	0	0			
Reduced v/c Ratio	0.53	0.02	0.03	0.74	0.11	0.08			
Intersection Summary									
Cycle Length: 95									
Actuated Cycle Length: 79.2	2								
Control Type: Semi Act-Unc									
Maximum v/c Ratio: 0.74									
Intersection Signal Delay: 1	0.3			In	itersectior	LOS: B			
Intersection Capacity Utiliza	tion 72.9%			IC	CU Level o	of Service	С		
Analysis Period (min) 15									

Splits and Phases: 3: Go Station East Access & Lakeshore Dr.

Ø1	₩ Ø2	<b>Å</b> ₽ <sub>Ø4</sub>
11 s	53 s	31 s
₹ø6		<b>√</b> Ø8
64 s		31 s

## 4: Essa Rd. & Gowan St.

	1	•	Ť	۲	1	Ļ			
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT			
Lane Configurations	Υ		<b>∱î</b> ≽			4ħ			
Traffic Volume (vph)	5	205	354	6	158	263			
Future Volume (vph)	5	205	354	6	158	263			
Satd. Flow (prot)	1616	0	3423	0	0	3345			
Flt Permitted	0.999					0.667			
Satd. Flow (perm)	1616	0	3423	0	0	2272			
Satd. Flow (RTOR)	250		2						
Lane Group Flow (vph)	256	0	434	0	0	439			
Turn Type	Prot		NA		pm+pt	NA			
Protected Phases	8		2		1	6			
Permitted Phases	Ŭ		_		6				
Total Split (s)	32.0		87.0		11.0	98.0			
Total Lost Time (s)	6.0		6.0		11.5	6.0			
Act Effct Green (s)	11.0		92.0			92.0			
Actuated g/C Ratio	0.10		0.80			0.80			
v/c Ratio	0.10		0.00			0.24			
Control Delay	16.1		2.9			3.3			
Queue Delay	0.0		0.0			0.0			
Total Delay	16.1		2.9			3.3			
LOS	B		2.9 A			3.3 A			
	ы 16.1		2.9			3.3			
Approach Delay	10.1 B								
Approach LOS			A			A			
Queue Length 50th (m)	1.3		9.2			10.2			
Queue Length 95th (m)	18.1		14.9			18.6			
Internal Link Dist (m)	120.4		80.7			19.9			
Turn Bay Length (m)									
Base Capacity (vph)	558		2738			1817			
Starvation Cap Reductn	0		0			0			
Spillback Cap Reductn	0		0			0			
Storage Cap Reductn	0		0			0			
Reduced v/c Ratio	0.46		0.16			0.24			
Intersection Summary									
Cycle Length: 130									
Actuated Cycle Length: 11	5								
Control Type: Semi Act-Un									
Maximum v/c Ratio: 0.67	00010								
Intersection Signal Delay: 6.1 Intersection LOS: A									
Intersection Capacity Utiliz		6				of Service			
Analysis Period (min) 15	uu011100.07	0							
Anarysis i enou (min) 15									
Splits and Phases: 4: Es	sa Rd. & Go	wan St							
Splits and Filases. 4: ES	sa ru. a Gl	wan St.							

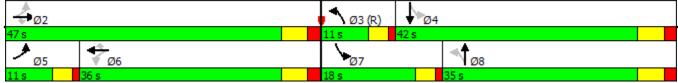


# 5: Property Access & Essa Rd.

	≯	*	•	t	Ļ	∢
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			-4↑	At≱	
Traffic Volume (veh/h)	10	15	16	544	406	6
Future Volume (Veh/h)	10	15	16	544	406	6
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.79	0.79	0.84	0.84	0.93	0.93
Hourly flow rate (vph)	13	19	19	648	437	6
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (m)				44	80	
pX, platoon unblocked	0.95	0.94	0.94		00	
vC, conflicting volume	802	222	443			
vC1, stage 1 conf vol	002		110			
vC2, stage 2 conf vol						
vCu, unblocked vol	579	48	284			
tC, single (s)	6.8	6.9	4.2			
tC, 2 stage (s)	0.0	0.7	1.2			
tF (s)	3.5	3.3	2.3			
p0 queue free %	97	98	98			
cM capacity (veh/h)	423	957	1167			
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	32	235	432	291	152	
Volume Left	13	19	0	0	0	
Volume Right	19	0	0	0	6	
cSH	632	1167	1700	1700	1700	
Volume to Capacity	0.05	0.02	0.25	0.17	0.09	
Queue Length 95th (m)	1.3	0.4	0.0	0.0	0.0	
Control Delay (s)	11.0	0.8	0.0	0.0	0.0	
Lane LOS	В	А				
Approach Delay (s)	11.0	0.3		0.0		
Approach LOS	В					
Intersection Summary						
Average Delay			0.5			
Intersection Capacity Utiliza	ation		36.6%	IC	CU Level d	f Service
Analysis Period (min)	-		15		,	
			10			

1: Go Station West Access & Tiffin St. & Lakeshore Dr. Future (2024) Total Traffic-P											c-PM	
	٦	→	$\mathbf{F}$	4	+	•	•	Ť	1	1	ŧ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	•	1	٦	•	1	ľ	el 🗧		<u>۲</u>	ef 👘	
Traffic Volume (vph)	159	502	32	0	302	574	32	0	0	743	0	122
Future Volume (vph)	159	502	32	0	302	574	32	0	0	743	0	122
Satd. Flow (prot)	1805	1900	808	950	1900	1599	902	950	0	1805	1599	0
Flt Permitted	0.363						0.671			0.665		
Satd. Flow (perm)	690	1900	808	950	1900	1599	637	950	0	1264	1599	0
Satd. Flow (RTOR)			120			586					509	-
Lane Group Flow (vph)	167	528	34	0	308	586	50	0	0	816	134	0
Turn Type	pm+pt	NA	Perm	Perm	NA	Perm	pm+pt	Ū	Ū	pm+pt	NA	U
Protected Phases	5	2	1 onn	1 onn	6	1 01111	3	8		7	4	
Permitted Phases	2	2	2	6	U	6	8	U		4	т	
Total Split (s)	11.0	47.0	47.0	36.0	36.0	36.0	11.0	35.0		18.0	42.0	
Total Lost Time (s)	4.0	47.0 6.0	6.0	6.0	6.0	6.0	4.0	6.0		4.0	6.0	
Act Effct Green (s)	4.0	41.0	41.0	0.0	30.0	30.0	38.0	0.0		4.0	36.0	
、 <i>,</i>	0.43	0.41	0.41		0.30	0.30	0.38			49.0 0.49	0.36	
Actuated g/C Ratio	0.45	0.41	0.41		0.50	0.30				1.17	0.30	
v/c Ratio							0.19					
Control Delay	28.2	38.1	0.5		33.5	6.4	16.5			118.1	0.4	
Queue Delay	0.0	30.1	0.0		0.0	0.0	0.0			0.0	0.0	
Total Delay	28.2	68.2	0.5		33.5	6.4	16.5			118.1	0.4	
LOS	С	E	А		C	А	В	4/ 5		F	A	
Approach Delay		55.9			15.7			16.5			101.5	
Approach LOS		E			В			В			F	
Queue Length 50th (m)	28.1	107.7	0.0		52.3	0.0	5.3			~200.0	0.0	
Queue Length 95th (m)	m29.8	m124.5	m0.0		79.7	27.2	8.4			#273.4	0.0	
Internal Link Dist (m)		44.8			116.0			88.9			104.9	
Turn Bay Length (m)	35.0		25.0			55.0				75.0		
Base Capacity (vph)	374	779	402		570	889	260			695	901	
Starvation Cap Reductn	0	269	0		0	0	0			0	0	
Spillback Cap Reductn	0	0	0		0	0	0			0	0	
Storage Cap Reductn	0	0	0		0	0	0			0	0	
Reduced v/c Ratio	0.45	1.04	0.08		0.54	0.66	0.19			1.17	0.15	
Intersection Summary												
Cycle Length: 100												
Actuated Cycle Length: 100												
		e 3:NBL. S	Start of Gr	reen								
	8.0			In	ntersection	1LOS E						
<b>3</b>		%										
						0.00110						
	tv allelle	is theoretic	ally infini	ite								
<ul> <li>Volume exceeds capacity, queue is theoretically infinite.</li> <li>Queue shown is maximum after two cycles.</li> </ul>												
	Queue shown is maximum after two cycles.											
	m Volume for 95th percentile queue is metered by upstream signal.											
Queue shown is maximu # 95th percentile volume e Queue shown is maximu	ordinated 8.0 tion 114.3 ty, queue m after tw exceeds ca m after tw	% is theoretic ro cycles. apacity, qu ro cycles.	cally infini ieue may	In IC ite. be longe								

#### Splits and Phases: 1: Go Station West Access & Tiffin St. & Lakeshore Dr.



Future (2024) Total Traffic-PM

	٦	-	$\mathbf{k}$	4	-	*	1	Ť	1	1	Ļ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	eî		٢	<b>†</b>			र्स कि			-4↑	1
Traffic Volume (vph)	46	374	88	132	217	0	91	491	178	148	437	70
Future Volume (vph)	46	374	88	132	217	0	91	491	178	148	437	70
Satd. Flow (prot)	1752	1816	0	1687	1863	0	0	3428	0	0	3463	1599
Flt Permitted	0.609			0.118				0.675			0.549	
Satd. Flow (perm)	1123	1816	0	210	1863	0	0	2328	0	0	1924	1599
Satd. Flow (RTOR)		12				-		42	-	-		89
Lane Group Flow (vph)	51	514	0	147	241	0	0	894	0	0	740	89
Turn Type	Perm	NA		pm+pt	NA		Perm	NA		pm+pt	NA	Perm
Protected Phases	4	4		3	8		2	2		1	6	
Permitted Phases	4 36.0	36.0		8	17.0		2	38.0		6 15.0	6 53.0	6 52.0
Total Split (s) Total Lost Time (s)	36.0 6.0	30.0 6.0		11.0 4.0	47.0 6.0		38.0	38.0 6.0		15.0	53.0 6.0	53.0 6.0
Act Effct Green (s)	30.0	30.0		4.0	42.2			45.8			45.8	45.8
Actuated g/C Ratio	0.30	0.30		0.44	42.2 0.42			43.0 0.46			45.0 0.46	0.46
v/c Ratio	0.30	0.93		0.69	0.42			0.40			0.95dl	0.40
Control Delay	27.3	58.8		43.4	10.3			30.0			34.3	3.7
Queue Delay	0.0	0.2		0.0	0.0			5.7			8.9	0.0
Total Delay	27.3	58.9		43.4	10.3			35.7			43.2	3.7
LOS	С	E		D	В			D			D	А
Approach Delay		56.1			22.8			35.7			38.9	
Approach LOS		E			С			D			D	
Queue Length 50th (m)	7.6	99.4		11.9	13.6			80.3			70.2	0.0
Queue Length 95th (m)	17.2	#164.5		#43.8	21.0			93.7			74.7	6.0
Internal Link Dist (m)		153.0			41.6			55.6			43.8	
Turn Bay Length (m)	47.0			23.0								89.0
Base Capacity (vph)	336	553		214	786			1088			904	798
Starvation Cap Reductn	0	0		0	0			0			0	0
Spillback Cap Reductn	0	1		0	0			146			138	0
Storage Cap Reductn	0	0		0	0			0			0	0
Reduced v/c Ratio	0.15	0.93		0.69	0.31			0.95			0.97	0.11
Intersection Summary												
Cycle Length: 100												_
Actuated Cycle Length: 100			art of Cr	oon Moot	ar Intara	otion						
Offset: 0 (0%), Referenced to		WBIL, SI	art of Gr	een, Masi	er interse	ection						
Control Type: Actuated-Coordinated												
Maximum v/c Ratio: 0.93												
Intersection Signal Delay: 39.1Intersection LOS: DIntersection Capacity Utilization 98.3%ICU Level of Service F												
Analysis Period (min) 15												
# 95th percentile volume exceeds capacity, queue may be longer.												
Queue shown is maximu			ouo may	SC IONYCI								
dl Defacto Left Lane. Rec		3	ne as a	eft lane.								

Splits and Phases: 2: Essa Rd./Bradford St. & Tiffin St.



Synchro 9 Report

	<b>→</b>	$\mathbf{\hat{z}}$	4	-	1	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	Ø4
Lane Configurations	<b>†</b>	1	٦	1	7	1	
Traffic Volume (vph)	1178	67	8	849	26	17	
Future Volume (vph)	1178	67	8	849	26	17	
Satd. Flow (prot)	1900	1615	1805	1900	1805	1615	
Flt Permitted			0.064		0.950		
Satd. Flow (perm)	1900	1615	122	1900	1805	1615	
Satd. Flow (RTOR)		26				55	
Lane Group Flow (vph)	1280	73	8	884	84	55	
Turn Type	NA	Perm	pm+pt	NA	Prot	Perm	
Protected Phases	2		1	6	8		4
Permitted Phases		2	6			8	
Total Split (s)	53.0	53.0	11.0	64.0	31.0	31.0	31.0
Total Lost Time (s)	6.0	6.0	4.0	6.0	5.0	5.0	
Act Effct Green (s)	60.1	60.1	63.0	62.2	10.5	10.5	
Actuated g/C Ratio	0.76	0.76	0.79	0.78	0.13	0.13	
v/c Ratio	0.89	0.06	0.03	0.59	0.35	0.21	
Control Delay	21.7	3.8	2.9	7.1	35.9	11.5	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	21.7	3.8	2.9	7.1	35.9	11.5	
LOS	С	А	А	А	D	В	
Approach Delay	20.8			7.1	26.2		
Approach LOS	С			A	С		
Queue Length 50th (m)	131.3	1.6	0.3	55.3	12.3	0.0	
Queue Length 95th (m)	#327.9	8.4	1.2	97.9	8.2	0.0	
Internal Link Dist (m)	144.3	40.0	70.0	137.6	30.3		
Turn Bay Length (m)	1405	40.0	70.0	1407	500	F/F	
Base Capacity (vph)	1435	1226	244	1487	590	565	
Starvation Cap Reductn	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	
Reduced v/c Ratio	0.89	0.06	0.03	0.59	0.14	0.10	
Intersection Summary							
Cycle Length: 95							
Actuated Cycle Length: 79	.5						
Control Type: Semi Act-Un	coord						
Maximum v/c Ratio: 0.89							
Intersection Signal Delay: 7					ntersectior		
Intersection Capacity Utiliz	ation 79.5%			IC	CU Level o	of Service	D
Analysis Period (min) 15							
# 95th percentile volume			leue may	be longe	r.		
Queue shown is maxim	um after two	cycles.					

Queue shown is maximum after two cycles.

Splits and Phases: 3: Go Station East Access & Lakeshore Dr.

Ø1	₩ Ø2	
11 s	53 s	31 s
Ø6		<b>₩</b> Ø8
64 s		31 s

## 4: Essa Rd. & Gowan St.

	4	•	Ť	۲	1	Ļ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Υ		A			- <b>₹</b> †
Traffic Volume (vph)	7	320	456	10	241	438
Future Volume (vph)	7	320	456	10	241	438
Satd. Flow (prot)	1632	0	3497	0	0	3344
Flt Permitted	0.999					0.637
Satd. Flow (perm)	1632	0	3497	0	0	2167
Satd. Flow (RTOR)	390		3			
Lane Group Flow (vph)	399	0	561	0	0	707
Turn Type	Prot	-	NA	-	pm+pt	NA
Protected Phases	8		2		1	6
Permitted Phases	0		2		6	0
Total Split (s)	32.0		87.0		11.0	98.0
Total Lost Time (s)	6.0		6.0		11.0	6.0
Act Effct Green (s)	12.0		92.1			92.1
• •	0.10		92.1 0.79			92.1 0.79
Actuated g/C Ratio						
v/c Ratio	0.77		0.20			0.41
Control Delay	16.2		3.4			4.8
Queue Delay	0.0		0.0			0.7
Total Delay	16.2		3.4			5.5
LOS	В		А			А
Approach Delay	16.2		3.4			5.5
Approach LOS	В		А			А
Queue Length 50th (m)	2.0		12.4			19.6
Queue Length 95th (m)	18.8		23.2			41.5
Internal Link Dist (m)	120.4		80.7			19.9
Turn Bay Length (m)						
Base Capacity (vph)	668		2773			1718
Starvation Cap Reductn	0		0			624
Spillback Cap Reductn	0		0			0
Storage Cap Reductn	0		0			0
Reduced v/c Ratio	0.60		0.20			0.65
Intersection Summary						
Cycle Length: 130						
Actuated Cycle Length: 11	61					
Control Type: Semi Act-Un						
	icooru					
Maximum v/c Ratio: 0.77	7.0			L.,	torocal!-	
Intersection Signal Delay:		/			ntersection	
Intersection Capacity Utiliz	alion 168.5%	0		](	CU Level	of Service
Analysis Period (min) 15						
Splits and Phases: 4: Es	sa Rd. & Go	wan St.				
		mun Ot.				



# 5: Property Access & Essa Rd.

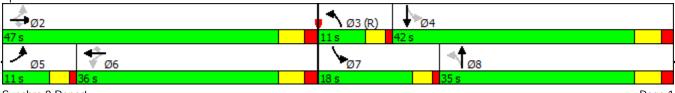
	≯	*	•	Ť	Ļ	<
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			{î†	A	
Traffic Volume (veh/h)	5	26	20	756	654	2
Future Volume (Veh/h)	5	26	20	756	654	2
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.79	0.79	0.84	0.84	0.93	0.93
Hourly flow rate (vph)	6	33	24	900	703	2
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (m)				44	80	
pX, platoon unblocked	0.91	0.89	0.89		00	
vC, conflicting volume	1202	352	705			
vC1, stage 1 conf vol	1202	352	700			
vC2, stage 2 conf vol						
vCu, unblocked vol	838	24	420			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)	0.0	0.7	1.1			
tF (s)	3.5	3.3	2.2			
p0 queue free %	98	96	98			
cM capacity (veh/h)	274	937	1023			
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	39	324	600	469	236	
Volume Left	6	24	0	0	0	
Volume Right	33	0	0	0	2	
cSH	683	1023	1700	1700	1700	
Volume to Capacity	0.06	0.02	0.35	0.28	0.14	
Queue Length 95th (m)	1.5	0.6	0.0	0.0	0.0	
Control Delay (s)	10.6	0.9	0.0	0.0	0.0	
Lane LOS	В	А				
Approach Delay (s)	10.6	0.3		0.0		
Approach LOS	В					
Intersection Summary						
Average Delay			0.4			
Intersection Capacity Utiliza	ation		45.3%	IC	CU Level d	of Service
Analysis Period (min)			15			
			10			

1: Go Station West Access & Tiffin St. & Lakeshore Dr. Future (2024) Total Traffic-Weekend

	٨	-	$\mathbf{F}$	•	+	*	1	1	1	1	ţ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ľ	•	1	ľ	•	1	ľ	4Î		ľ	el el	
Traffic Volume (vph)	177	300	33	0	223	517	33	0	0	539	0	163
Future Volume (vph)	177	300	33	0	223	517	33	0	0	539	0	163
Satd. Flow (prot)	1805	1881	808	950	1881	1615	902	950	0	1787	1615	0
Flt Permitted	0.468						0.644			0.665		
Satd. Flow (perm)	889	1881	808	950	1881	1615	612	950	0	1251	1615	0
Satd. Flow (RTOR)			120			528					590	
Lane Group Flow (vph)	186	316	35	0	228	528	52	0	0	592	179	0
Turn Type	pm+pt	NA	Perm	Perm	NA	Perm	pm+pt			pm+pt	NA	
Protected Phases	5	2			6		3	8		7	4	
Permitted Phases	2		2	6		6	8			4		
Total Split (s)	11.0	47.0	47.0	36.0	36.0	36.0	11.0	35.0		18.0	42.0	
Total Lost Time (s)	4.0	6.0	6.0	6.0	6.0	6.0	4.0	6.0		4.0	6.0	
Act Effct Green (s)	43.0	41.0	41.0		30.0	30.0	38.0			49.0	36.0	
Actuated g/C Ratio	0.43	0.41	0.41		0.30	0.30	0.38			0.49	0.36	
v/c Ratio	0.42	0.41	0.09		0.40	0.62	0.21			0.86	0.19	
Control Delay	32.3	33.6	0.9		30.5	6.0	16.8			35.5	0.4	
Queue Delay	0.0	1.8	0.0		0.0	0.0	0.0			0.0	0.0	
Total Delay	32.3	35.4	0.9		30.5	6.0	16.8			35.5	0.4	
LOS	С	D	А		С	А	В	44.0		D	A	
Approach Delay		32.1			13.4			16.8			27.3	
Approach LOS	07.0	С			В			В		00 <i>(</i>	С	
Queue Length 50th (m)	37.9	68.5	0.0		36.9	0.0	5.5			88.6	0.0	
Queue Length 95th (m)	m52.6	m92.8	m0.0		58.8	25.2	8.7	00.0		#149.7	0.0	
Internal Link Dist (m)	05.0	44.8	05.0		116.0	55.0		88.9		75.0	104.9	
Turn Bay Length (m)	35.0	771	25.0		F ( 4	55.0	252			75.0	050	
Base Capacity (vph)	446	771	402		564	854	252			688	959	
Starvation Cap Reductn	0	299	0		0	0	0 0			0	0	
Spillback Cap Reductn	0	0	0		0	0				0	0	
Storage Cap Reductn Reduced v/c Ratio	0 0.42	0 0.67	0 0.09		0 0.40	0 0.62	0 0.21			0 0.86	0 0.19	
	0.42	0.07	0.09		0.40	0.02	0.21			0.00	0.19	
Intersection Summary												
Cycle Length: 100												
Actuated Cycle Length: 100												
Offset: 26 (26%), Reference		3:NBL, S	start of Gr	een								
Control Type: Actuated-Coc	rainalea											
Maximum v/c Ratio: 0.86	าา			In	toroootion							
Intersection Signal Delay: 2		7/			tersection							
Intersection Capacity Utilization 101.5% ICU Level of Service G												
Analysis Period (min) 15												
<ul> <li>95th percentile volume exceeds capacity, queue may be longer.</li> <li>Output shown is maximum after two evolutions</li> </ul>												
	Queue shown is maximum after two cycles.											

m Volume for 95th percentile queue is metered by upstream signal.

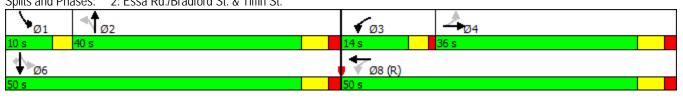
Splits and Phases: 1: Go Station West Access & Tiffin St. & Lakeshore Dr.



Synchro 9 Report

Future (2024) Total Traffic-Weekend

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	٦	ef 🕺		ሻ	<b>↑</b>			4î b				1
Traffic Volume (vph)	29	246	61	175	163	0	69	400	214	61	223	43
Future Volume (vph)	29	246	61	175	163	0	69	400	214	61	223	43
Satd. Flow (prot)	1736	1814	0	1736	1863	0	0	3403	0	0	3462	1615
Flt Permitted	0.643			0.321				0.827			0.553	
Satd. Flow (perm)	1175	1814	0	586	1863	0	0	2829	0	0	1936	1615
Satd. Flow (RTOR)		13						80				76
Lane Group Flow (vph)	32	341	0	194	181	0	0	804	0	0	359	54
Turn Type	Perm	NA		pm+pt	NA		Perm	NA		pm+pt	NA	Perm
Protected Phases		4		3	8			2		1	6	
Permitted Phases	4			8			2			6	6	6
Total Split (s)	36.0	36.0		14.0	50.0		40.0	40.0		10.0	50.0	50.0
Total Lost Time (s)	6.0	6.0		4.0	6.0			6.0			6.0	6.0
Act Effct Green (s)	30.0	30.0		59.0	57.0			31.0			31.0	31.0
Actuated g/C Ratio	0.30	0.30		0.59	0.57			0.31			0.31	0.31
v/c Ratio	0.09	0.62		0.32	0.17			0.86			0.60	0.10
Control Delay	26.2	34.6		6.6	5.9			39.1			33.0	3.0
Queue Delay	0.0	0.0		0.0	0.0			0.6			0.0	0.0
Total Delay	26.2	34.6		6.6	5.9			39.8			33.0	3.0
LOS	С	С		А	А			D			С	A
Approach Delay		33.9			6.3			39.8			29.0	
Approach LOS		С			А			D			С	
Queue Length 50th (m)	4.7	57.2		8.5	8.2			73.4			32.3	0.0
Queue Length 95th (m)	12.1	87.7		15.3	15.0			81.1			36.4	3.0
Internal Link Dist (m)		153.0			41.6			55.6			43.8	
Turn Bay Length (m)	47.0			23.0								89.0
Base Capacity (vph)	352	553		610	1062			1035			851	753
Starvation Cap Reductn	0	0		0	0			54			0	0
Spillback Cap Reductn	0	0		0	0			0			0	0
Storage Cap Reductn	0	0		0	0			0			0	0
Reduced v/c Ratio	0.09	0.62		0.32	0.17			0.82			0.42	0.07
Intersection Summary												
Cycle Length: 100												
Actuated Cycle Length: 100												
Offset: 0 (0%), Referenced		WBTL, St	art of Gr	een, Mast	er Interse	ection						
Control Type: Actuated-Coc	ordinated											
Maximum v/c Ratio: 0.86												
Intersection Signal Delay: 3					tersection		-					
Intersection Capacity Utilization 88.2% ICU Level of Service E												
Analysis Period (min) 15												
Splits and Phases: 2: Ess	Splits and Phases: 2: Essa Rd./Bradford St. & Tiffin St.											



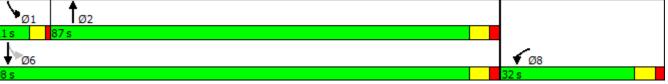
	-	$\mathbf{\hat{z}}$	•	-	1	1				
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	Ø4			
Lane Configurations	<b>†</b>	1	٦	<b>†</b>	٦	1				
Traffic Volume (vph)	815	23	9	720	19	16				
Future Volume (vph)	815	23	9	720	19	16				
Satd. Flow (prot)	1881	1615	1805	1881	1805	1615				
Flt Permitted			0.218		0.950					
Satd. Flow (perm)	1881	1615	414	1881	1805	1615				
Satd. Flow (RTOR)		13				52				
Lane Group Flow (vph)	886	25	9	750	61	52				
Turn Type	NA	Perm	pm+pt	NA	Prot	Perm				
Protected Phases	2		1	6	8		4			
Permitted Phases		2	6			8				
Total Split (s)	53.0	53.0	11.0	64.0	31.0	31.0	31.0			
Total Lost Time (s)	6.0	6.0	4.0	6.0	5.0	5.0				
Act Effct Green (s)	60.1	60.1	63.1	62.3	10.1	10.1				
Actuated g/C Ratio	0.76	0.76	0.80	0.79	0.13	0.13				
v/c Ratio	0.62	0.02	0.02	0.51	0.26	0.21				
Control Delay	9.8	3.5	2.4	5.7	34.4	11.9				
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0				
Total Delay	9.8	3.5	2.4	5.7	34.4	11.9				
LOS	А	А	А	А	С	В				
Approach Delay	9.6			5.7	24.1					
Approach LOS	А			А	С					
Queue Length 50th (m)	56.0	0.4	0.4	41.8	8.9	0.0				
Queue Length 95th (m)	155.5	3.7	1.2	66.7	6.6	0.0				
Internal Link Dist (m)	144.3			137.6	30.3					
Turn Bay Length (m)		40.0	70.0							
Base Capacity (vph)	1426	1228	452	1478	593	565				
Starvation Cap Reductn	0	0	0	0	0	0				
Spillback Cap Reductn	0	0	0	0	0	0				
Storage Cap Reductn	0	0	0	0	0	0				
Reduced v/c Ratio	0.62	0.02	0.02	0.51	0.10	0.09				
Intersection Summary										
Cycle Length: 95										
Actuated Cycle Length: 79.2	2									
Control Type: Semi Act-Unc	coord									
Maximum v/c Ratio: 0.62										
Intersection Signal Delay: 8	.9			In	itersectior	n LOS: A				
Intersection Capacity Utiliza	ntion 60.4%			IC	CU Level o	of Service	В			
Analysis Period (min) 15										

Splits and Phases: 3: Go Station East Access & Lakeshore Dr.

Ø1	₩ Ø2	
11 s	53 s	31 s
₹ø6		<b>₩</b> Ø8
64 s		31 s

## 4: Essa Rd. & Gowan St.

	4	•	Ť	۲	1	Ŧ	
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	Y		<b>∱</b> }			{î†	
Traffic Volume (vph)	11	259	443	8	131	351	
Future Volume (vph)	11	259	443	8	131	351	
Satd. Flow (prot)	1650	0	3530	0	0	3487	
Flt Permitted	0.998					0.680	
Satd. Flow (perm)	1650	0	3530	0	0	2402	
Satd. Flow (RTOR)	316		3				
Lane Group Flow (vph)	329	0	544	0	0	502	
Turn Type	Prot		NA		pm+pt	NA	
Protected Phases	8		2		1	6	
Permitted Phases					6		
Total Split (s)	32.0		87.0		11.0	98.0	
Total Lost Time (s)	6.0		6.0			6.0	
Act Effct Green (s)	11.6		92.0			92.0	
Actuated g/C Ratio	0.10		0.80			0.80	
v/c Ratio	0.73		0.19			0.26	
Control Delay	16.7		3.2			3.6	
Queue Delay	0.0		0.0			0.4	
Total Delay	16.7		3.2			4.0	
LOS	В		А			А	
Approach Delay	16.7		3.2			4.0	
Approach LOS	В		А			А	
Queue Length 50th (m)	2.9		11.9			11.8	
Queue Length 95th (m)	20.0		20.6			23.5	
Internal Link Dist (m)	120.4		80.7			19.9	
Turn Bay Length (m)							
Base Capacity (vph)	616		2811			1912	
Starvation Cap Reductn	0		0			890	
Spillback Cap Reductn	0		0			0	
Storage Cap Reductn	0		0			0	
Reduced v/c Ratio	0.53		0.19			0.49	
Intersection Summary							
Cycle Length: 130							
Actuated Cycle Length: 115	5.6						
Control Type: Semi Act-Unc							
Maximum v/c Ratio: 0.73							
Intersection Signal Delay: 6	.7			Ir	ntersectior	1 LOS: A	
Intersection Capacity Utiliza				IC	CU Level (	of Service	Η
Analysis Period (min) 15							
-							
Splits and Phases: 4: Ess	sa Rd. & Go	wan St.					



# 5: Property Access & Essa Rd.

	٨	*	•	Ť	ţ	4
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Υ			- <b>4</b> ↑	A	
Traffic Volume (veh/h)	10	37	30	672	445	15
Future Volume (Veh/h)	10	37	30	672	445	15
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.79	0.79	0.84	0.84	0.93	0.93
Hourly flow rate (vph)	13	47	36	800	478	16
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)				NOTIC	NOTIC	
Upstream signal (m)				44	80	
pX, platoon unblocked	0.97	0.95	0.95	77	00	
vC, conflicting volume	958	247	494			
vC1, stage 1 conf vol	750	277	777			
vC2, stage 2 conf vol						
vCu, unblocked vol	724	96	357			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)	0.0	0.7	4.1			
tF (s)	3.5	3.3	2.2			
p0 queue free %	3.5 96	3.3 95	2.2 97			
cM capacity (veh/h)	90 341	90 899	1150			
	341	077				
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	60	303	533	319	175	
Volume Left	13	36	0	0	0	
Volume Right	47	0	0	0	16	
cSH	664	1150	1700	1700	1700	
Volume to Capacity	0.09	0.03	0.31	0.19	0.10	
Queue Length 95th (m)	2.4	0.8	0.0	0.0	0.0	
Control Delay (s)	11.0	1.2	0.0	0.0	0.0	
Lane LOS	В	А				
Approach Delay (s)	11.0	0.5		0.0		
Approach LOS	В					
Intersection Summary						
Average Delay			0.7			
Intersection Capacity Utiliza	ation		45.6%	IC	CU Level o	f Service
Analysis Period (min)			15	ic.		
			10			

# **APPENDIX**

# G Intersection Capacity Analysis for Scenario 2

# **Intersection Capacity Analysis for Scenario 2**

Scenario 2 provides dual lanes for the northbound left movement at Lakeshore Drive and Tiffin Street intersection (one dedicated left turn lane plus one shared left/through/right lane). Similar to Scenario 1, signal improvement measures were implemented to facilitate bus movements egressing from the mobility hub:

- Due to dual left turn lanes, the northbound movement at Lakeshore Drive and Tiffin Street intersection was proposed to operate under a split phase, hence so was the southbound movement;
- A protected plus permissive phase was proposed for the westbound left movement at Essa Road/Bradford Street and Tiffin Street intersection; and,
- Actual-coordinated phase setting was applied to Tiffin Street intersections at Essa Road/Bradford Street and at Lakeshore Drive for the above mentioned two movements.

The evaluation elements for intersection capacity analysis include average delays, LOS, v/c ratio, and queue length. As compared with Scenario 1, Scenario 2 assumed different signal timing plans only for Tiffin Street intersections at Lakeshore Drive, and at Essa Road/Bradford Street. The intersection capacity analysis results for these two intersections for Scenario 2 are presented in Table 1. Detailed Synchro reports are presented in Appendix.

 Table 1: Future (2024) Intersection Capacity Analysis Results-Scenario 2

#### Morning Peak Hour

Intersection	Ove Interse			Weekday AM Peak							
Intersection			Movement	Delay (s)	LOS	V/C	Queue <sup>1</sup> (m)				
			EBL	52	D	0.48	46				
			EBT	55	D	0.43	103				
			EBR	8	А	0.11	3				
Lakeshore Dr. &	48	P	WBT	53	D	0.75	116				
Tiffin St. (Signalized)	40	D	WBR	14	В	0.83	74				
			NBL	38	D	0.13	10				
			SBL	103	F	1.07	206				
			SBT	1	А	0.14	0				
			EBL	29	С	0.19	26				
			EBTR	34	С	0.56	99				
			WBL	5	А	0.19	9				
Essa Rd. & Tiffin St. (Signalized)	37	D	WBT	5	А	0.21	19				
(Signalized)			NBLTR	55	D	0.89	86				
			SBLT	45	D	0.66	45				
			SBR	5	А	0.11	4				

#### Afternoon Peak Hour

Overa Intersection				Week	day PM Pea	k	
mersection	Delay	LOS	Movement	Delay (s)	LOS	V/C	Queue¹ (m)
			EBL	49	D	0.72	43
			EBT	106	F	0.88	156
			EBR	3	А	0.11	0
Lakeshore Dr. & Tiffin St.	105	F	WBT	56	E	0.70	111
(Signalized)		E.	WBR	12	В	0.75	54
			NBL	46	D	0.25	16
			SBL	225	F	1.40	373
			SBTR	1	А	0.15	0
			EBL	33	С	0.14	21
			EBTR	63	E	0.89	194
			WBL	46	D	0.53	43
Essa Rd. & Tiffin St. (Signalized)	67	E	WBT	9	А	0.28	26
(Oignalized)			NBLTR	92	F	0.89	122
			SBLT	73	E	1.07	95
			SBR	4	А	0.12	7

#### Weekend Peak Hour

	Ove Interse		Weekend Peak								
Intersection			Movement	Delay (s)	LOS	V/C	Queue¹ (m)				
			EBL	62	E	0.62	75				
			EBT	66	Е	0.53	121				
Lakeshore Dr. & Tiffin St. (Signalized)			EBR	7	А	0.11	1				
	49	D	WBT	49	D	0.53	82				
	49	D	WBR	8	А	0.68	32				
			NBL	46	D	0.26	17				
			SBL	88	F	1.03	243				
			SBTR	1	А	0.19	0				
			EBL	32	С	0.09	14				
			EBTR	41	D	0.59	108				
			WBL	9	А	0.30	24				
Essa Rd. & Tiffin St. (Signalized)	40	D	WBT	8	А	0.16	23				
(			NBLTR	56	Е	0.91	108				
			SBLT	43	D	0.63	47				
			SBR	6	А	0.10	6				

Note: 1. Queue length reflects the 95<sup>th</sup> percentile queue length

The analysis results for Scenario 2 show higher traffic delays for the Tiffin Street intersections, as compared to Scenario 1. The overall intersection delays are expected to increase and LOS will drop from 'C' to 'D' during morning and weekend peak hours, and from 'D/E' to 'E/F' during the afternoon peak hour.

Many turning movements at Tiffin Street intersections are expected to operate at LOS 'E/F'. The higher traffic delays are mainly caused by the longer cycle length (from 100 s to 130 s) required to accommodate the split phases for the northbound and southbound movements at the Lakeshore Drive and Tiffin Street intersection. Therefore, Scenario 1 is preferable over Scenario 2 in terms of traffic performance.

Appendix

1: Go Station West Access & Tiffin St. & Lakeshore Dr.										AM wi	th Dua	l Left
	٦	-	$\mathbf{i}$	4	←	*	1	Ť	1	1	ţ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	۲	<b>†</b>	1	٦	•	1	٦ ۲	\$		۳	et 🗧	
Traffic Volume (vph)	106	267	34	0	344	730	34	0	0	458	0	105
Future Volume (vph)	106	267	34	0	344	730	34	0	0	458	0	105
Satd. Flow (prot)	1787	1900	808	950	1881	1599	857	857	0	1770	1568	0
Flt Permitted	0.230						0.950	0.950		0.950		
Satd. Flow (perm)	433	1900	808	950	1881	1599	857	857	0	1770	1568	0
Satd. Flow (RTOR)			82			669					557	
Lane Group Flow (vph)	112	281	36	0	351	745	26	27	0	503	115	0
Turn Type	pm+pt	NA	Perm	Perm	NA	Perm	Split	NA		Split	NA	
Protected Phases	5	2			6		8	8		4	4	
Permitted Phases	2		2	6		6						
Total Split (s)	11.0	47.0	47.0	36.0	36.0	36.0	35.0	35.0		38.0	38.0	
Total Lost Time (s)	4.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0		6.0	6.0	
Act Effct Green (s)	43.0	41.0	41.0	010	30.0	30.0	29.0	29.0		32.0	32.0	
Actuated g/C Ratio	0.36	0.34	0.34		0.25	0.25	0.24	0.24		0.27	0.27	
v/c Ratio	0.48	0.43	0.11		0.75	0.83	0.13	0.13		1.07	0.14	
Control Delay	52.0	51.8	7.7		52.6	14.4	37.6	37.8		102.5	0.4	
Queue Delay	0.0	2.8	0.0		0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	52.0	54.6	7.7		52.6	14.4	37.6	37.8		102.5	0.4	
LOS	02.0 D	D	A		02.0 D	В	07.0 D	07.0 D		F	A	
Approach Delay	U	50.0	7		26.6	U	U	37.7		•	83.5	
Approach LOS		50.0 D			20.0 C			D			65.5 F	
Queue Length 50th (m)	29.3	74.3	0.6		80.6	14.9	5.3	5.5		~137.7	0.0	
Queue Length 95th (m)	m45.8	103.2	m2.5		116.3	74.1	9.6	9.8		#205.8	0.0	
Internal Link Dist (m)	1110.0	44.8	1112.0		116.0	,	7.0	88.9		#200.0	104.9	
Turn Bay Length (m)	35.0	11.0	25.0		110.0	55.0		00.7		75.0	101.7	
Base Capacity (vph)	234	649	330		470	901	207	207		472	826	
Starvation Cap Reductn	0	258	0		0	0	0	0		0	0	
Spillback Cap Reductn	0	0	0		0	0	0	0		0	0	
Storage Cap Reductn	0	0	0		0	0	0	0		0	0	
Reduced v/c Ratio	0.48	0.72	0.11		0.75	0.83	0.13	0.13		1.07	0.14	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 120	)											
Offset: 66 (55%), Reference	ed to phase	8:NBTL,	Start of C	Green								
Control Type: Actuated-Coo	ordinated											
Maximum v/c Ratio: 1.07												
Intersection Signal Delay: 4	7.5			In	ntersection	n LOS: D						
Intersection Capacity Utilization 97.0% ICU Level of Service F												
Analysis Period (min) 15												
<ul> <li>Volume exceeds capacity, queue is theoretically infinite.</li> </ul>												
Queue shown is maximum after two cycles.												
# 95th percentile volume		2	ieue may	be longe	r.							
Queue shown is maximu				Ű								
	m Volume for 95th percentile queue is metered by upstream signal.											
Splits and Dhasos 1: Co	Station Mr	st Acces	c & Tiffin	Ct & Lak	oshora D	r						

#### Splits and Phases: 1: Go Station West Access & Tiffin St. & Lakeshore Dr.

4 <sub>02</sub>		₩ <sub>Ø4</sub>	🖡 📢 Ø8 (R)
47 s		38 s	35 s
▶ <sub>Ø5</sub> ♦	Ø6		
11 s 36 s			

2024 AM with Dual Left

	۶	-	$\mathbf{F}$	4	+	*	1	1	1	1	ŧ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	4Î		ሻ	<b>↑</b>			ፋት				1
Traffic Volume (vph)	68	245	73	110	226	0	63	363	128	49	229	39
Future Volume (vph)	68	245	73	110	226	0	63	363	128	49	229	39
Satd. Flow (prot)	1770	1781	0	1641	1845	0	0	3388	0	0	3356	1568
Flt Permitted	0.603			0.346				0.784			0.598	
Satd. Flow (perm)	1123	1781	0	598	1845	0	0	2673	0	0	2025	1568
Satd. Flow (RTOR)		14						36				64
Lane Group Flow (vph)	76	353	0	122	251	0	0	652	0	0	352	49
Turn Type	Perm	NA		pm+pt	NA		Perm	NA		pm+pt	NA	Perm
Protected Phases		4		3	8			2		1	6	
Permitted Phases	4			8			2			6	6	6
Total Split (s)	48.0	48.0		15.0	63.0		47.0	47.0		10.0	57.0	57.0
Total Lost Time (s)	6.0	6.0		4.0	6.0			6.0			6.0	6.0
Act Effct Green (s)	42.0	42.0		78.2	76.2			31.8			31.8	31.8
Actuated g/C Ratio	0.35	0.35		0.65	0.64			0.26			0.26	0.26
v/c Ratio	0.19	0.56		0.19	0.21			0.89			0.66	0.11
Control Delay	28.9	34.3		4.9	4.9			54.5			44.8	4.9
Queue Delay	0.0	0.0		0.0	0.4			0.2			0.1	0.0
Total Delay	28.9	34.3		4.9	5.3			54.8			44.8	4.9
LOS	С	С		А	А			D			D	А
Approach Delay		33.3			5.2			54.8			39.9	
Approach LOS		С			А			D			D	
Queue Length 50th (m)	13.1	67.4		5.5	12.0			78.2			41.0	0.0
Queue Length 95th (m)	25.6	98.9		m9.2	m18.6			86.3			45.2	4.1
Internal Link Dist (m)		153.0			41.6			55.6			43.8	
Turn Bay Length (m)	47.0			23.0								89.0
Base Capacity (vph)	393	632		652	1171			936			860	703
Starvation Cap Reductn	0	0		0	521			36			0	0
Spillback Cap Reductn	0	0		0	0			21			43	0
Storage Cap Reductn	0	0		0	0			0			0	0
Reduced v/c Ratio	0.19	0.56		0.19	0.39			0.72			0.43	0.07
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 120												
Offset: 0 (0%), Referenced t		WBTL, St	art of Gr	een, Mas	ter Interse	ection						
Control Type: Actuated-Coo	rdinated											
Maximum v/c Ratio: 0.89												
Intersection Signal Delay: 36					ntersectior							
Intersection Capacity Utilizat	tion 84.3%			10	CU Level o	of Service	E					
Analysis Period (min) 15												
m Volume for 95th percent	tile queue i	is metered	d by upst	ream sigi	nal.							

#### Splits and Phases: 2: Essa Rd./Bradford St. & Tiffin St.

▶ø1 ¶ø2	✓ Ø3 ✓ Ø3
10 s 47 s	15 s 48 s
<b>♦</b> ∞ø6	🛡 🐨 Ø8 (R)
57 s	63 s

1: Go Station West Access & Tiffin St. & Lakeshore Dr.										2024 PM with Dual Left				
	٦	-	$\mathbf{i}$	4	←	*	1	Ť	۲	5	Ļ	~		
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR		
Lane Configurations	۲	•	1	۲	1	1	۲.	eî.		<u>۲</u>	et 🗧			
Traffic Volume (vph)	159	502	32	0	302	574	32	0	0	743	0	122		
Future Volume (vph)	159	502	32	0	302	574	32	0	0	743	0	122		
Satd. Flow (prot)	1805	1900	808	950	1900	1599	902	950	0	1805	1599	0		
Flt Permitted	0.259						0.950			0.950				
Satd. Flow (perm)	492	1900	808	950	1900	1599	902	950	0	1805	1599	0		
Satd. Flow (RTOR)			76			539					562			
Lane Group Flow (vph)	167	528	34	0	308	586	50	0	0	816	134	0		
Turn Type	pm+pt	NA	Perm	Perm	NA	Perm	Split			Split	NA			
Protected Phases	5	2			6		8	8		4	4			
Permitted Phases	2		2	6		6								
Total Split (s)	11.0	47.0	47.0	36.0	36.0	36.0	35.0	35.0		48.0	48.0			
Total Lost Time (s)	4.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0		6.0	6.0			
Act Effct Green (s)	43.0	41.0	41.0		30.0	30.0	29.0			42.0	42.0			
Actuated g/C Ratio	0.33	0.32	0.32		0.23	0.23	0.22			0.32	0.32			
v/c Ratio	0.72	0.88	0.11		0.70	0.75	0.25			1.40	0.15			
Control Delay	48.8	57.2	2.7		55.7	12.2	45.6			224.8	0.4			
Queue Delay	0.0	49.1	0.0		0.0	0.0	0.0			0.0	0.0			
Total Delay	48.8	106.3	2.7		55.7	12.2	45.6			224.8	0.4			
LOS	D	F	А		E	В	D			F	А			
Approach Delay		88.3			27.2			45.6			193.1			
Approach LOS		F			С			D			F			
Queue Length 50th (m)	35.9	117.8	0.0		76.8	10.1	11.2			~293.8	0.0			
Queue Length 95th (m)	m42.5 m	า#155.8	m0.0		110.6	54.1	16.3			#372.8	0.0			
Internal Link Dist (m)		44.8			116.0			88.9			104.9			
Turn Bay Length (m)	35.0		25.0			55.0				75.0				
Base Capacity (vph)	233	599	306		438	783	201			583	897			
Starvation Cap Reductn	0	119	0		0	0	0			0	0			
Spillback Cap Reductn	0	0	0		0	0	0			0	0			
Storage Cap Reductn	0	0	0		0	0	0			0	0			
Reduced v/c Ratio	0.72	1.10	0.11		0.70	0.75	0.25			1.40	0.15			
Intersection Summary														
Cycle Length: 130														
Actuated Cycle Length: 130														
Offset: 95 (73%), Reference		8:NBTL,	Start of C	Green										
Control Type: Actuated-Coo														
Maximum v/c Ratio: 1.40														
Intersection Signal Delay: 1	04.6			In	tersection	n LOS: F								
Intersection Capacity Utiliza		%		IC	CU Level	of Service	еH							
Analysis Period (min) 15														
<ul> <li>Volume exceeds capacity, queue is theoretically infinite.</li> </ul>														
Queue shown is maximum after two cycles.														
# 95th percentile volume e			ieue may	be longe	r.									
Queue shown is maximum after two cycles.														
m Volume for 95th percen			d by upsti	ream sigr	nal.									

#### Splits and Phases: 1: Go Station West Access & Tiffin St. & Lakeshore Dr.

	₩ <sub>Ø4</sub>	🕈 øs (R)
47 s	48 s	35 s
▶ <sub>Ø5</sub> ♥ <sub>Ø6</sub>		
11 s 36 s		

2024 PM with Dual Left

	۶	-	$\mathbf{\hat{v}}$	•	+	•	•	1	1	1	Ŧ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	et		٦	<b>↑</b>			4 b				1
Traffic Volume (vph)	46	374	88	132	217	0	91	491	178	148	437	70
Future Volume (vph)	46	374	88	132	217	0	91	491	178	148	437	70
Satd. Flow (prot)	1752	1816	0	1687	1863	0	0	3428	0	0	3463	1599
Flt Permitted	0.609			0.120				0.645			0.535	
Satd. Flow (perm)	1123	1816	0	213	1863	0	0	2225	0	0	1875	1599
Satd. Flow (RTOR)		10						37				89
Lane Group Flow (vph)	51	514	0	147	241	0	0	894	0	0	740	89
Turn Type	Perm	NA		pm+pt	NA		Perm	NA		pm+pt	NA	Perm
Protected Phases		4		3	8			2		1	6	
Permitted Phases	4			8			2			6	6	6
Total Split (s)	47.0	47.0		15.0	62.0		58.0	58.0		10.0	68.0	68.0
Total Lost Time (s)	6.0	6.0		4.0	6.0			6.0			6.0	6.0
Act Effct Green (s)	41.0	41.0		62.6	60.6			57.4			57.4	57.4
Actuated g/C Ratio	0.32	0.32		0.48	0.47			0.44			0.44	0.44
v/c Ratio	0.14	0.89		0.53	0.28			0.89			1.07dl	0.12
Control Delay	33.4	60.2		46.0	8.7			43.7			47.6	4.0
Queue Delay	0.0	0.3		0.0	0.3			47.9			24.9	0.0
Total Delay	33.4	60.4		46.0	9.0			91.6			72.5	4.0
LOS	С	E		D	А			F			E	А
Approach Delay		58.0			23.0			91.6			65.1	_
Approach LOS	0.0	E		00 (	С			F			E	0.0
Queue Length 50th (m)	9.9	129.5		22.6	17.7			107.0			91.6	0.0
Queue Length 95th (m)	20.7	#194.2		m42.5	25.8			121.8			94.6	6.5
Internal Link Dist (m)	47.0	153.0		22.0	41.6			55.6			43.8	00.0
Turn Bay Length (m)	47.0	570		23.0	0/0			1010			004	89.0
Base Capacity (vph)	354	579		279	868			1019			894	809
Starvation Cap Reductn	0	0		0	251			211			0	0
Spillback Cap Reductn	0	3		0	0			202			178	0
Storage Cap Reductn	0	0		0	0			0			0	0
Reduced v/c Ratio	0.14	0.89		0.53	0.39			1.11			1.03	0.11
Intersection Summary												
Cycle Length: 130												
Actuated Cycle Length: 130												
Offset: 0 (0%), Referenced t	•	:WBTL, St	art of Gr	een, Mast	er Interse	ection						
Control Type: Actuated-Coo	rdinated											
Maximum v/c Ratio: 0.89												
Intersection Signal Delay: 66		,			tersectior		_					
Intersection Capacity Utilizat	tion 98.3%	ò		IC	CU Level o	of Service	e F					_
Analysis Period (min) 15												
# 95th percentile volume e		1 3 1	eue may	be longe	r.							_
Queue shown is maximum			I Inc. 1997. 1									
m Volume for 95th percent					ial.							
dl Defacto Left Lane. Rec	ode with 1	though la	ne as a l	en iane.								

Splits and Phases: 2: Essa Rd./Bradford St. & Tiffin St.

Ø1	Ø2	<b>√</b> Ø3	<u></u> ⊿ <sub>Ø4</sub>
10 s 58 s		15 s	47 s
\$ Ø6		Ø8 (R)	
68 s		62 s	

1: Go Station West	1: Go Station West Access & Tiffin St. & Lakeshore Dr. 2024 Weekend with Dual Le											
	۶	<b>→</b>	$\mathbf{i}$	•	-	*	1	1	1	1	ţ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	<u>م</u>	•		ľ	•	1	۲. ۲	el el		1	et	
Traffic Volume (vph)	177	300	33	0	223	517	33	0	0	539	0	163
Future Volume (vph)	177	300	33	0	223	517	33	0	0	539	0	163
Satd. Flow (prot)	1805	1881	808	950	1881	1615	902	950	0	1787	1615	0
Flt Permitted	0.390						0.950			0.950		
Satd. Flow (perm)	741	1881	808	950	1881	1615	902	950	0	1787	1615	0
Satd. Flow (RTOR)			76			528					610	
Lane Group Flow (vph)	186	316	35	0	228	528	52	0	0	592	179	0
Turn Type	pm+pt	NA	Perm	Perm	NA	Perm	Split			Split	NA	
Protected Phases	5	2			6		. 8	8		. 4	4	
Permitted Phases	2		2	6		6						
Total Split (s)	11.0	47.0	47.0	36.0	36.0	36.0	35.0	35.0		48.0	48.0	
Total Lost Time (s)	4.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0		6.0	6.0	
Act Effct Green (s)	43.0	41.0	41.0		30.0	30.0	29.0			42.0	42.0	
Actuated g/C Ratio	0.33	0.32	0.32		0.23	0.23	0.22			0.32	0.32	
v/c Ratio	0.62	0.53	0.11		0.53	0.68	0.26			1.03	0.19	
Control Delay	61.8	58.4	7.3		48.9	8.3	45.8			87.6	0.5	
Queue Delay	0.0	7.8	0.0		0.0	0.0	0.0			0.0	0.0	
Total Delay	61.8	66.2	7.3		48.9	8.3	45.8			87.6	0.5	
LOS	E	E	А		D	А	D			F	А	
Approach Delay		60.8			20.6			45.8			67.3	
Approach LOS		E			С			D			E	
Queue Length 50th (m)	53.0	90.6	0.3		54.2	0.0	11.6			~169.9	0.0	
Queue Length 95th (m)	m75.3	m121.5	m1.4		81.5	32.4	16.8			#243.3	0.0	
Internal Link Dist (m)		44.8			116.0			88.9			104.9	
Turn Bay Length (m)	35.0		25.0			55.0				75.0		
Base Capacity (vph)	302	593	306		434	778	201			577	934	
Starvation Cap Reductn	0	231	0		0	0	0			0	0	
Spillback Cap Reductn	0	0	0		0	0	0			0	0	
Storage Cap Reductn	0	0	0		0	0	0			0	0	
Reduced v/c Ratio	0.62	0.87	0.11		0.53	0.68	0.26			1.03	0.19	
Intersection Summary												
Cycle Length: 130												
Actuated Cycle Length: 130												
Offset: 79 (61%), Reference		- 8·NBTI	Start of (	Green								
Control Type: Actuated-Coc		o on to re,	otart of c	broom								
Maximum v/c Ratio: 1.03	anatou											
Intersection Signal Delay: 48.5 Intersection LOS: D												
Intersection Signal Delay: 48.5 Intersection LOS: D Intersection Capacity Utilization 101.5% ICU Level of Service G												
Analysis Period (min) 15												
<ul> <li>Volume exceeds capacity, queue is theoretically infinite.</li> </ul>												
<ul> <li>Volume exceeds capacity, queue is medically minine.</li> <li>Queue shown is maximum after two cycles.</li> </ul>												
<ul><li>95th percentile volume exceeds capacity, queue may be longer.</li></ul>												
Queue shown is maximum after two cycles.												
m Volume for 95th percen			d by upst	ream sion	al.							
	0			o								

#### Splits and Phases: 1: Go Station West Access & Tiffin St. & Lakeshore Dr.

	₩ <sub>Ø4</sub>	🕈 øs (R)
47 s	48 s	35 s
▶ <sub>Ø5</sub> ♥ <sub>Ø6</sub>		
11 s 36 s		

2024 Weekend with Dual Left

	٦	-	$\mathbf{i}$	•	-	•	1	Ť	1	1	Ļ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	et 🗧		٦	•			र्स कि				1
Traffic Volume (vph)	29	246	61	175	163	0	69	400	214	61	223	43
Future Volume (vph)	29	246	61	175	163	0	69	400	214	61	223	43
Satd. Flow (prot)	1736	1814	0	1736	1863	0	0	3403	0	0	3462	1615
Flt Permitted	0.643			0.324				0.797			0.530	
Satd. Flow (perm)	1175	1814	0	592	1863	0	0	2726	0	0	1855	1615
Satd. Flow (RTOR)		10						65				59
Lane Group Flow (vph)	32	341	0	194	181	0	0	804	0	0	359	54
Turn Type	Perm	NA		pm+pt	NA		Perm	NA		pm+pt	NA	Perm
Protected Phases		4		3	8			2		1	6	
Permitted Phases	4			8			2			6	6	6
Total Split (s)	47.0	47.0		18.0	65.0		55.0	55.0		10.0	65.0	65.0
Total Lost Time (s)	6.0	6.0		4.0	6.0			6.0			6.0	6.0
Act Effct Green (s)	41.0	41.0		79.8	77.8			40.2			40.2	40.2
Actuated g/C Ratio	0.32	0.32		0.61	0.60			0.31			0.31	0.31
v/c Ratio	0.09	0.59		0.30	0.16			0.91			0.63	0.10
Control Delay	32.3	41.2		8.6	7.6			53.5			42.8	6.3
Queue Delay	0.0	0.0		0.0	0.0			2.8			0.1	0.0
Total Delay	32.3	41.2		8.6	7.6			56.3			43.0	6.3
LOS	С	D		А	A			E			D	А
Approach Delay		40.5			8.1			56.3			38.2	
Approach LOS	/ 1	D		14.0	A			E			D	0.0
Queue Length 50th (m)	6.1	75.0		14.2	13.5			101.7			43.4	0.0
Queue Length 95th (m)	14.4	108.0		24.1	23.1			107.7			46.8	5.8
Internal Link Dist (m)	47.0	153.0		23.0	41.6			55.6			43.8	00.0
Turn Bay Length (m)	47.0 370	578		23.0 651	1114			1067			841	89.0 765
Base Capacity (vph) Starvation Cap Reductn	370	578 0		0	0			161			841 0	705 0
Spillback Cap Reductin	0	0		0	0			71			71	0
Storage Cap Reductin	0	0		0	0			0			0	0
Reduced v/c Ratio	0.09	0.59		0.30	0.16			0.89			0.47	0.07
	0.07	0.57		0.30	0.10			0.07			0.47	0.07
Intersection Summary												
Cycle Length: 130												
Actuated Cycle Length: 130												
Offset: 0 (0%), Referenced		WBIL, St	art of Gr	een, Mast	er Interse	ection						
Control Type: Actuated-Coc	ordinated											
Maximum v/c Ratio: 0.91	0.0			L.								
Intersection Signal Delay: 4					tersectior		-					
Intersection Capacity Utiliza	100 88.2%			IC	U Level (	of Service	:E					_
Analysis Period (min) 15												
Splits and Phases: 2: Ess	sa Rd./Brad	Iford St &	Tiffin St									
			Timi Ji									

▶ø1 <b>1</b> Ø2	Ø3	<u></u> 4
10 s 55 s	18 s	47 s
<b>↓</b> ø <sub>6</sub>	Ø8 (R)	
65 s	65 s	

Synchro 9 Report

# APPENDIX

# H Intersection Capacity Analysis for Sensitivity Scenario

# Intersection Capacity Analysis for Sensitivity Scenario

A sensitivity scenario was conducted for the future total traffic condition, using optimized signal timing plans to reduce overall intersection delays and improve traffic operation. Following signal timing settings were considered at Lakeshore Drive and Tiffin Street intersection and at Essa Road and Tiffin Street intersection:

- A permissive phase was used for the northbound left movement at Lakeshore Drive and Tiffin Street intersection (traffic signal priority exists in the existing condition);
- A protected plus permissive phase was used for the westbound left movement at Essa Road/Bradford Street and Tiffin Street intersection (signal head with green arrow light exists in the existing condition); and,
- Both intersections were designed to operate under actuated phase setting (non-coordination).

The evaluation elements for intersection capacity analysis include average delays, LOS, v/c ratio, and queue length. The intersection capacity analysis results for this sensitivity scenario are presented in Table 1. Detailed Synchro reports are presented in the appendix.

#### Table 1: Future (2024) Intersection Capacity Analysis Results-Sensitivity Scenario

#### Morning Peak Hour

	Ove Interse			Weekday AM Peak								
Intersection	Delay	LOS	Movement	Delay (s)	LOS	V/C	Queue¹ (m)					
			EBL	21	С	0.34	24					
			EBT	22	С	0.36	59					
			EBR	1	А	0.10	1					
Lakeshore Dr. & Tiffin St.	19	В	WBT	36	D	0.62	92					
(Signalized)	19	В	WBR	8	А	0.74	32					
			NBL	33	С	0.28	13					
			SBL	27	С	0.74	103					
			SBT	1	А	0.12	0					
			EBL	17	В	0.17	17					
			EBTR	19	В	0.48	65					
			WBL	9	А	0.24	17					
Essa Rd. & Tiffin St. (Signalized)	22	С	WBT	11	В	0.24	35					
( - 3			NBLTR	30	С	0.78	56					
			SBLT	26	С	0.56	30					
			SBR	2	А	0.10	2					

#### Afternoon Peak Hour

Intersection	Ove Interse		Weekday PM Peak								
mersection	Delay	LOS	Movement	Delay (s)	LOS	V/C	Queue¹ (m)				
			EBL	22	С	0.45	35				
			EBT	38	D	0.68	124				
			EBR	1	А	0.09	1				
Lakeshore Dr. & Tiffin St.	47	D	WBT	34	С	0.54	80				
(Signalized)	77	D	WBR	6	А	0.66	27				
			NBL	34	С	0.31	12				
			SBL	101	F	1.13	276				
			SBTR	1	А	0.14	0				
			EBL	26	С	0.14	17				
			EBTR	48	D	0.87	164				
			WBL	39	D	0.71	42				
Essa Rd. & Tiffin St. (Signalized)	36	D	WBT	21	С	0.30	52				
( <b>0</b> )			NBLTR	35	С	0.84	91				
			SBLT	36	D	0.98	73				
			SBR	4	А	0.12	5				

#### Weekend Peak Hour

	Ove Interse		Weekend Peak							
Intersection	Delay LOS		Movement	Delay (s)	LOS	V/C	Queue¹ (m)			
			EBL	22	С	0.42	39			
			EBT	24	С	0.41	68			
			EBR	1	А	0.09	1			
Lakeshore Dr. & Tiffin St.	21	С	WBT	31	С	0.40	58			
(Signalized)	21	C	WBR	6	А	0.62	25			
			NBL	33	С	0.21	13			
			SBL	36	D	0.86	149			
			SBTR	1	А	0.19	0			
			EBL	18	В	0.07	9			
			EBTR	22	С	0.51	68			
			WBL	12	В	0.40	28			
Essa Rd. & Tiffin St. (Signalized)	22	С	WBT	11	В	0.19	28			
( - 3			NBLTR	27	С	0.81	64			
			SBLT	24	С	0.54	29			
			SBR	2	А	0.09	2			

Appendix

1: Go Station West Access & Tiffin St. & Lakeshore DrFuture (2024) Total Traffic-AM-opt traffic

	≯	-	$\mathbf{F}$	•	+	*	1	1	1	1	Ļ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	<b>↑</b>	1	<u>۲</u>	<b>↑</b>	1	ሻ	eî 👘		- ሻ	eî 👘	
Traffic Volume (vph)	106	267	34	0	344	730	34	0	0	458	0	105
Future Volume (vph)	106	267	34	0	344	730	34	0	0	458	0	105
Satd. Flow (prot)	1787	1900	808	950	1881	1599	902	950	0	1770	1568	0
Flt Permitted	0.308						0.683			0.665		
Satd. Flow (perm)	579	1900	808	950	1881	1599	649	950	0	1239	1568	0
Satd. Flow (RTOR)			76			745					398	
Lane Group Flow (vph)	112	281	36	0	351	745	53	0	0	503	115	0
Turn Type	pm+pt	NA	Perm	Perm	NA	Perm	Perm			pm+pt	NA	
Protected Phases	5	2			6			8		7	4	
Permitted Phases	2		2	6		6	8			4		
Total Split (s)	11.0	47.0	47.0	36.0	36.0	36.0	35.0	35.0		18.0	53.0	
Total Lost Time (s)	4.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0		4.0	6.0	
Act Effct Green (s)	43.0	41.0	41.0		30.0	30.0	29.0			49.0	47.0	
Actuated g/C Ratio	0.43	0.41	0.41		0.30	0.30	0.29			0.49	0.47	
v/c Ratio	0.34	0.36	0.10		0.62	0.74	0.28			0.74	0.12	
Control Delay	20.3	22.1	1.1		35.9	7.5	32.5			26.7	0.3	
Queue Delay	0.0	0.0	0.0		0.0	0.0	0.0			0.0	0.0	
Total Delay	20.3	22.1	1.1		35.9	7.5	32.5			26.7	0.3	
LOS	С	С	А		D	А	С			С	А	
Approach Delay		19.9			16.6			32.5			21.8	
Approach LOS		В			В			С			С	
Queue Length 50th (m)	13.6	38.9	0.0		61.4	0.0	8.3			70.4	0.0	
Queue Length 95th (m)	24.7	59.9	1.2		92.3	32.7	13.2			103.4	0.0	
Internal Link Dist (m)		44.8			116.0			88.9			104.9	
Turn Bay Length (m)	35.0		25.0			55.0				75.0		
Base Capacity (vph)	333	779	376		564	1001	188			681	947	
Starvation Cap Reductn	0	0	0		0	0	0			0	0	
Spillback Cap Reductn	0	0	0		0	0	0			0	0	
Storage Cap Reductn	0	0	0		0	0	0			0	0	
Reduced v/c Ratio	0.34	0.36	0.10		0.62	0.74	0.28			0.74	0.12	
Intersection Summary												
Cycle Length: 100												
Actuated Cycle Length: 100												
Control Type: Semi Act-Unc	coord											
Maximum v/c Ratio: 0.74												
Intersection Signal Delay: 19.1 Intersection LOS: B												
	Intersection Capacity Utilization 97.0% ICU Level of Service F											
Analysis Period (min) 15												
Callia and Diagona 1, Ca	Ctation Ma		0 T'(('									

Splits and Phases: 1: Go Station West Access & Tiffin St. & Lakeshore Dr.

4 <sub>02</sub>		<b>₽</b> Ø4	
47 s		53 s	
	<b>∲</b> Ø6	Ø7	<b>™</b> ø8
11 s	36 s	18 s	35 s

Future (2024) Total Traffic-AM-opt traffic

	≯	<b>→</b>	$\mathbf{F}$	4	←	•	•	Ť	۲	1	ŧ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	٦	4Î		٦	<b>↑</b>			4 î b				1
Traffic Volume (vph)	68	245	73	110	226	0	63	363	128	49	229	39
Future Volume (vph)	68	245	73	110	226	0	63	363	128	49	229	39
Satd. Flow (prot)	1770	1781	0	1641	1845	0	0	3388	0	0	3356	1568
Flt Permitted	0.603			0.410				0.847			0.665	
Satd. Flow (perm)	1123	1781	0	708	1845	0	0	2887	0	0	2252	1568
Satd. Flow (RTOR)		18						45				85
Lane Group Flow (vph)	76	353	0	122	251	0	0	652	0	0	352	49
Turn Type	Perm	NA		pm+pt	NA		Perm	NA		pm+pt	NA	Perm
Protected Phases		4		3	8			2		1	6	
Permitted Phases	4			8			2			6	6	6
Total Split (s)	36.0	36.0		11.0	47.0		33.0	33.0		10.0	43.0	43.0
Total Lost Time (s)	6.0	6.0		4.0	6.0			6.0			6.0	6.0
Act Effct Green (s)	30.1	30.1		43.2	41.2			20.7			20.7	20.7
Actuated g/C Ratio	0.41	0.41		0.58	0.56			0.28			0.28	0.28
v/c Ratio	0.17	0.48		0.24	0.24			0.78			0.56	0.10
Control Delay	16.7	18.9		9.3	10.1			29.5			26.1	2.1
Queue Delay	0.0	0.0		0.0	0.0			0.0			0.0	0.0
Total Delay	16.7	18.9		9.3	10.1			29.5			26.1	2.1
LOS	В	В		А	В			С			С	A
Approach Delay		18.5			9.8			29.5			23.2	
Approach LOS		В			А			С			С	
Queue Length 50th (m)	6.8	34.6		7.2	17.2			42.6			22.9	0.0
Queue Length 95th (m)	17.6	65.9		17.5	35.5			56.4			30.2	1.6
Internal Link Dist (m)		153.0			41.6			55.6			43.8	
Turn Bay Length (m)	47.0			23.0								89.0
Base Capacity (vph)	457	736		502	1027			1087			1131	830
Starvation Cap Reductn	0	0		0	0			0			0	0
Spillback Cap Reductn	0	0		0	0			0			0	0
Storage Cap Reductn	0	0		0	0			0			0	0
Reduced v/c Ratio	0.17	0.48		0.24	0.24			0.60			0.31	0.06
Intersection Summary												
Cycle Length: 90												
Actuated Cycle Length: 73.9	9											
Control Type: Semi Act-Unc	coord											
Maximum v/c Ratio: 0.78												
Intersection Signal Delay: 21.6 Intersection LOS: C												
Intersection Capacity Utiliza	ition 84.3%			IC	CU Level o	of Service	E					
Analysis Period (min) 15												

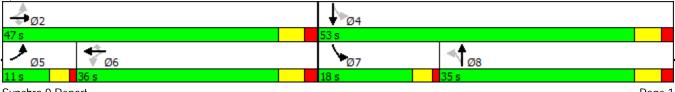
Splits and Phases: 2: Essa Rd./Bradford St. & Tiffin St.

Ø1	<b>√</b> ø <sub>2</sub>	Ø3	<u></u> 04
10 s	33 s	11 s	36 s
\$ Ø6		Ø8	
43 s		47 s	

1: Go Station West Access & Tiffin St. & Lakeshore DrFuture (2024) Total Traffic-PM-opt traffic

	۶	-	$\rightarrow$	*	-	*	1	1	۲	1	ţ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	<b>↑</b>	1	ሻ	<b>↑</b>	1	ሻ	eî 👘		- ሻ	eî 👘	
Traffic Volume (vph)	159	502	32	0	302	574	32	0	0	743	0	122
Future Volume (vph)	159	502	32	0	302	574	32	0	0	743	0	122
Satd. Flow (prot)	1805	1900	808	950	1900	1599	902	950	0	1805	1599	0
Flt Permitted	0.359						0.671			0.646		
Satd. Flow (perm)	682	1900	808	950	1900	1599	637	950	0	1227	1599	0
Satd. Flow (RTOR)			76			586					439	
Lane Group Flow (vph)	167	528	34	0	308	586	50	0	0	816	134	0
Turn Type	pm+pt	NA	Perm	Perm	NA	Perm	Perm			pm+pt	NA	
Protected Phases	5	2			6			8		7	4	
Permitted Phases	2		2	6		6	8			4		
Total Split (s)	11.0	47.0	47.0	36.0	36.0	36.0	35.0	35.0		18.0	53.0	
Total Lost Time (s)	4.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0		4.0	6.0	
Act Effct Green (s)	43.0	41.0	41.0		30.0	30.0	25.2			49.0	47.0	
Actuated g/C Ratio	0.43	0.41	0.41		0.30	0.30	0.25			0.49	0.47	
v/c Ratio	0.45	0.68	0.09		0.54	0.66	0.31			1.13	0.14	
Control Delay	22.3	29.5	0.8		33.5	6.4	33.7			101.4	0.3	
Queue Delay	0.0	8.9	0.0		0.0	0.0	0.0			0.0	0.0	
Total Delay	22.3	38.4	0.8		33.5	6.4	33.7			101.4	0.3	
LOS	С	D	А		С	А	С			F	A	
Approach Delay		32.9			15.7			33.7			87.1	
Approach LOS	00.0	С	0.0		В	0.0	7.0	С		000 7	F	
Queue Length 50th (m)	20.9	86.2	0.0		52.3	0.0	7.8			~202.7	0.0	
Queue Length 95th (m)	35.4	124.7	0.9		79.7	27.2	12.7	00.0		#276.2	0.0	
Internal Link Dist (m)		44.8			116.0			88.9		75.0	104.9	
Turn Bay Length (m)	35.0	770	25.0		570	55.0	104			75.0	004	
Base Capacity (vph)	371	779	376		570	889	184			722	984	
Starvation Cap Reductn	0	215	0		0	0	0			0	0	
Spillback Cap Reductn	0	0	0		0	0	0			0	0	
Storage Cap Reductn	0	0	0		0	0	0			0	0	
Reduced v/c Ratio	0.45	0.94	0.09		0.54	0.66	0.27			1.13	0.14	
Intersection Summary												
Cycle Length: 100												
Actuated Cycle Length: 100												
Control Type: Semi Act-Unc	coord											
Maximum v/c Ratio: 1.13												
<b>v</b>	Intersection Signal Delay: 46.7       Intersection LOS: D         Intersection Capacity Utilization 114.3%       ICU Level of Service H											
Intersection Capacity Utiliza	ition 114.3%	6		IC	U Level	of Service	θH					
Analysis Period (min) 15			II !£!!	1.								
<ul> <li>Volume exceeds capaci</li> </ul>			cally infini	lle.								
Queue shown is maximu				ho longo	r							
# 95th percentile volume e Queue shown is maximu			ieue may	be longe	Ι.							
Queue shown is maximu	ini allei lwo	r cycles.										
Splits and Phases: 1: Go Station West Access & Tiffin St. & Lakeshore Dr.												

Splits and Phases: 1: Go Station West Access & Tiffin St. & Lakeshore Dr.



Synchro 9 Report

Future (2024) Total Traffic-PM-opt traffic

	۶	-	$\mathbf{F}$	4	+	*	•	1	1	1	Ļ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	4Î		٦	<b>↑</b>			4î b				1
Traffic Volume (vph)	46	374	88	132	217	0	91	491	178	148	437	70
Future Volume (vph)	46	374	88	132	217	0	91	491	178	148	437	70
Satd. Flow (prot)	1752	1816	0	1687	1863	0	0	3428	0	0	3463	1599
Flt Permitted	0.609			0.147				0.676			0.548	
Satd. Flow (perm)	1123	1816	0	261	1863	0	0	2332	0	0	1921	1599
Satd. Flow (RTOR)		12						44				89
Lane Group Flow (vph)	51	514	0	147	241	0	0	894	0	0	740	89
Turn Type	Perm	NA		pm+pt	NA		Perm	NA		pm+pt	NA	Perm
Protected Phases		4		3	8		_	2		1	6	
Permitted Phases	4			8			2			6	6	6
Total Split (s)	36.0	36.0		10.0	46.0		40.0	40.0		14.0	54.0	54.0
Total Lost Time (s)	6.0	6.0		4.0	6.0			6.0			6.0	6.0
Act Effct Green (s)	30.1	30.1		42.2	40.2			42.1			42.1	42.1
Actuated g/C Ratio	0.32	0.32		0.45	0.43			0.45			0.45	0.45
v/c Ratio	0.14	0.87		0.71	0.30			0.84			0.98dl	0.12
Control Delay	26.2	48.4		39.1	20.3			30.0			35.1	3.6
Queue Delay	0.0	0.0		0.0	0.0			4.6			0.0	0.0
Total Delay	26.2	48.4		39.1	20.3			34.6			35.1	3.6
LOS	С	D		D	С			С			D	A
Approach Delay		46.4			27.4			34.6			31.8	
Approach LOS		D			С			С			С	
Queue Length 50th (m)	7.2	93.3		17.2	30.8			73.7			64.7	0.0
Queue Length 95th (m)	17.2	#164.5		#42.8	52.7			91.3			73.1	5.8
Internal Link Dist (m)	17.0	153.0		00.0	41.6			55.6			43.8	00.0
Turn Bay Length (m)	47.0	500		23.0	700			10/5			000	89.0
Base Capacity (vph)	358	588		207	793			1065			982	860
Starvation Cap Reductn	0	0		0	0			115			0	0
Spillback Cap Reductn	0	0		0	0			0			0	0
Storage Cap Reductn	0	0		0	0			0			0	0
Reduced v/c Ratio	0.14	0.87		0.71	0.30			0.94			0.75	0.10
Intersection Summary												
Cycle Length: 100	-											
Actuated Cycle Length: 94.3												
Control Type: Semi Act-Unc	coord											
Maximum v/c Ratio: 0.87												
Intersection Signal Delay: 35.2 Intersection LOS: D									_			
Intersection Capacity Utilization 98.3% ICU Level of Service F												
	Analysis Period (min) 15											
# 95th percentile volume exceeds capacity, queue may be longer.												
Queue shown is maximu		,	<b>DO</b>	offlore								
dl Defacto Left Lane. Rec	Lode with 1	inougn la	ine as a l	en iane.								
Splits and Phases 2. Essa Rd /Bradford St & Tiffin St												

Splits and Phases: 2: Essa Rd./Bradford St. & Tiffin St.

Ø1			<b>√</b> Ø3	<u></u> ⊗4			
14 s	40 s		10 s	36 s			
\$ ø6		₩ Ø8					
54 s			46 s				

1: Go Station West Access & Tiffin St. & Lakeshonet (2024) Total Traffic-Weekend-opt traffic

	۶	-	$\rightarrow$	•	+	*	1	1	۲	1	ţ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	۲	<b>↑</b>	1	۲.	•	1	٦	eî		٦	eî 👘	
Traffic Volume (vph)	177	300	33	0	223	517	33	0	0	539	0	163
Future Volume (vph)	177	300	33	0	223	517	33	0	0	539	0	163
Satd. Flow (prot)	1805	1881	808	950	1881	1615	902	950	0	1787	1615	0
Flt Permitted	0.468						0.644			0.665		
Satd. Flow (perm)	889	1881	808	950	1881	1615	612	950	0	1251	1615	0
Satd. Flow (RTOR)			76			528					538	
Lane Group Flow (vph)	186	316	35	0	228	528	52	0	0	592	179	0
Turn Type	pm+pt	NA	Perm	Perm	NA	Perm	Perm			pm+pt	NA	
Protected Phases	5	2			6			8		7	4	
Permitted Phases	2		2	6		6	8			4		
Total Split (s)	11.0	47.0	47.0	36.0	36.0	36.0	35.0	35.0		18.0	53.0	
Total Lost Time (s)	4.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0		4.0	6.0	
Act Effct Green (s)	43.0	41.0	41.0		30.0	30.0	29.0			49.0	47.0	
Actuated g/C Ratio	0.43	0.41	0.41		0.30	0.30	0.29			0.49	0.47	_
v/c Ratio	0.42	0.41	0.09		0.40	0.62	0.29			0.86	0.17	
Control Delay	21.5	23.0	1.0		30.5	6.0	33.2			35.5	0.4	_
Queue Delay	0.0	1.0	0.0		0.0	0.0	0.0			0.0	0.0	
Total Delay LOS	21.5 C	24.0 C	1.0 A		30.5 C	6.0 A	33.2 C			35.5	0.4	
Approach Delay	C	21.6	A		13.4	A	C	33.2		D	A 27.3	
Approach LOS		21.0 C			13.4 B			55.2 C			27.3 C	
Queue Length 50th (m)	23.5	44.8	0.0		36.9	0.0	8.2	C		88.6	0.0	
Queue Length 95th (m)	39.0	68.1	1.0		58.8	25.2	13.1			#149.7	0.0	
Internal Link Dist (m)	57.0	44.8	1.0		116.0	23.2	1J.1	88.9		$\pi$ 147.7	104.9	
Turn Bay Length (m)	35.0	0.77	25.0		110.0	55.0		00.7		75.0	104.7	
Base Capacity (vph)	446	771	376		564	854	177			688	1044	
Starvation Cap Reductn	0	244	0		0	0	0			0	0	
Spillback Cap Reductn	0	0	0		0	0	0			0	0	
Storage Cap Reductn	0	0	0		0	0	0			0	0	
Reduced v/c Ratio	0.42	0.60	0.09		0.40	0.62	0.29			0.86	0.17	
Intersection Summary												
Cycle Length: 100												
Actuated Cycle Length: 100	1											
Control Type: Semi Act-Uncoord												
Maximum v/c Ratio: 0.86												
Intersection Signal Delay: 21.1 Intersection LOS: C												
Intersection Capacity Utiliza	ition 101.5%	6		IC	CU Level	of Service	G					
Analysis Period (min) 15												
# 95th percentile volume			ieue may	be longer	r.							
Queue shown is maximu	im after two	cycles.										

Splits and Phases: 1: Go Station West Access & Tiffin St. & Lakeshore Dr.

	<b>↓</b> <sup>®</sup> Ø4
47 s	53 s
▶ <sub>Ø5</sub> ♥ <sub>Ø6</sub>	▶ø7 <b>1</b> ø8
11 s 36 s	18 s 35 s

2: Essa Rd./Bradford St. & Tiffir	າ St.
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Future (2024) Total Traffic-Weekend-opt traffic

	٦	→	$\mathbf{F}$	4	+	•	•	Ť	1	1	Ļ	∢
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	۲	eî 🗧		۲	•			4î þ				1
Traffic Volume (vph)	29	246	61	175	163	0	69	400	214	61	223	43
Future Volume (vph)	29	246	61	175	163	0	69	400	214	61	223	43
Satd. Flow (prot)	1736	1814	0	1736	1863	0	0	3403	0	0	3462	1615
Flt Permitted	0.643			0.397				0.856			0.589	
Satd. Flow (perm)	1175	1814	0	725	1863	0	0	2928	0	0	2062	1615
Satd. Flow (RTOR)		14						88				85
Lane Group Flow (vph)	32	341	0	194	181	0	0	804	0	0	359	54
Turn Type	Perm	NA		pm+pt	NA		Perm	NA		pm+pt	NA	Perm
Protected Phases		4		3	8			2		1	6	
Permitted Phases	4			8			2			6	6	6
Total Split (s)	33.0	33.0		11.0	44.0		36.0	36.0		10.0	46.0	46.0
Total Lost Time (s)	6.0	6.0		4.0	6.0			6.0			6.0	6.0
Act Effct Green (s)	27.1	27.1		40.2	38.2			23.7			23.7	23.7
Actuated g/C Ratio	0.37	0.37		0.54	0.52			0.32			0.32	0.32
v/c Ratio	0.07	0.51		0.40	0.19			0.81			0.54	0.09
Control Delay	17.8	21.7		12.4	11.3			27.2			23.7	2.2
Queue Delay	0.0	0.0		0.0	0.0			0.0			0.0	0.0
Total Delay	17.8	21.7		12.4	11.3			27.2			23.7	2.2
LOS	В	С		В	В			С			С	A
Approach Delay		21.4			11.9			27.2			20.9	
Approach LOS		С			В			С			С	
Queue Length 50th (m)	3.0	36.8		13.8	13.5			50.0			22.3	0.0
Queue Length 95th (m)	9.6	68.1		28.9	28.2			64.7			29.5	2.2
Internal Link Dist (m)		153.0			41.6			55.6			43.8	
Turn Bay Length (m)	47.0	. = .		23.0								89.0
Base Capacity (vph)	430	674		490	961			1245			1121	916
Starvation Cap Reductn	0	0		0	0			4			0	0
Spillback Cap Reductn	0	0		0	0			0			0	0
Storage Cap Reductn	0	0		0	0			0			0	0
Reduced v/c Ratio	0.07	0.51		0.40	0.19			0.65			0.32	0.06
Intersection Summary												
Cycle Length: 90												
Actuated Cycle Length: 73.9												
Control Type: Semi Act-Uncoord												
Maximum v/c Ratio: 0.81												
<b>č</b> ,	Intersection Signal Delay: 21.9 Intersection LOS: C											
Intersection Capacity Utiliza	ition 88.2%			IC	U Level	of Service	E					
Analysis Period (min) 15												

Splits and Phases: 2: Essa Rd./Bradford St. & Tiffin St.

Ø1	<↑ ø2	<b>√</b> Ø3	<u></u> _Ø4
10 s	36 s	11 s	33 s
\$ ø6		₹ø8	
46 s		44 s	